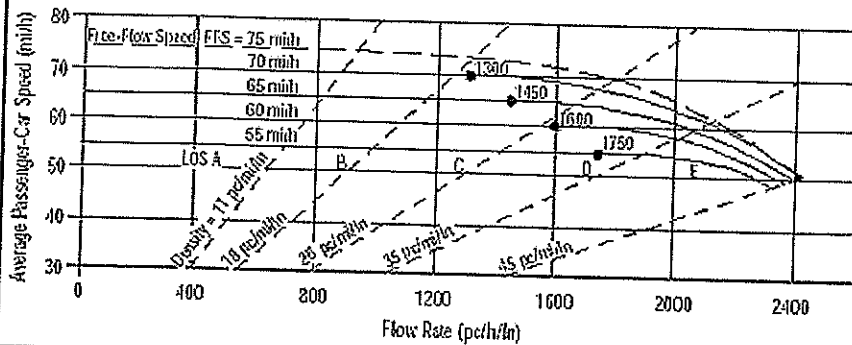


# 2012 Build - Preferred Alternative with Service Road

## BASIC FREEWAY SEGMENTS WORKSHEET



Application	Input	Output
Operational (LOS)	FFS, N, $v_p$	LOS, S, D
Design (N)	FFS, LOS, $v_p$	N, S, D
Design ( $v_p$ )	FFS, LOS, N	$v_p$ , S, D
Planning (LOS)	FFS, N, AADT	LOS, S, D
Planning (N)	FFS, LOS, AADT	N, S, D
Planning ( $v_p$ )	FFS, LOS, N	$v_p$ , S, D

### General Information

Analyst: *KNM*  
 Agency or Company: *HNTB*  
 Date Performed: *3/25/2008*  
 Analysis Time Period: *Peak*  
 Project Description: *Wekiva Parkway PD&E*

### Site Information

Highway/Direction of Travel: *I-4/Eastbound*  
 From/To: *Lake Mary Blvd /CR 46A/SR417*  
 Jurisdiction:  
 Analysis Year: *2012 Build*

Oper. (LOS)

Des. (N)

Planning Data

### Flow Inputs

Volume, V: *5530* veh/h  
 AADT: veh/day  
 Peak-Hr Prop. of AADT, K:  
 Peak-Hr Direction Prop, D:  
 DDHV = AADT x K x D  
 Driver type adjustment: *1.00* veh/h

Peak-Hour Factor, PHF: *0.95*  
 %Trucks and Buses,  $P_T$ : *9*  
 %RVs,  $P_R$ : *0*  
 General Terrain: *Level*  
 Grade % Length *mi*  
 Up/Down %

### Calculate Flow Adjustments

$f_p$ : *1.00*  
 $E_T$ : *1.5*  
 $E_R$ : *1.2*  
 $f_{HV} = 1/[1+P_T(E_T-1) + P_R(E_R-1)]$ : *0.957*

### Speed Inputs

Lane Width: *12.0* ft  
 Rt-Shoulder Lat. Clearance: *6.0* ft  
 Interchange Density: *0.45* l/mi  
 Number of Lanes, N: *3*  
 FFS (measured): mi/h  
 Base free-flow Speed, BFFS: *70.0* mi/h

### Calc Speed Adj and FFS

$f_{LW}$ : *0.0* mi/h  
 $f_{LC}$ : *0.0* mi/h  
 $f_{ID}$ : *0.0* mi/h  
 $f_N$ : *3.0* mi/h  
 FFS: *67.0* mi/h

### LOS and Performance Measures

Operational (LOS)  
 $v_p = (V \text{ or } DDHV) / (PHF \times N \times f_{HV} \times f_p)$  2028 pc/h/ln  
 S: *62.3* mi/h  
 $D = v_p / S$ : *32.5* pc/mi/ln  
 LOS: *D*

### Design (N)

Design (N)  
 Design LOS  
 $v_p = (V \text{ or } DDHV) / (PHF \times N \times f_{HV} \times f_p)$  pc/h  
 S: mi/h  
 $D = v_p / S$  pc/mi/ln  
 Required Number of Lanes, N

### Glossary

N - Number of lanes  
 V - Hourly volume  
 $v_p$  - Flow rate  
 LOS - Level of service  
 DDHV - Directional design hour volume

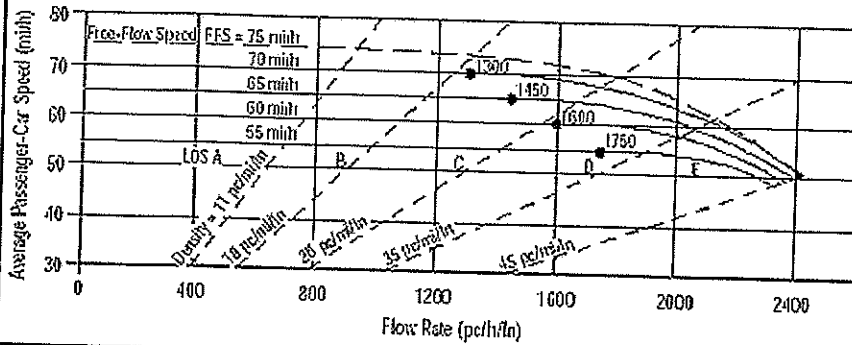
S - Speed  
 D - Density  
 FFS - Free-flow speed  
 BFFS - Base free-flow speed

### Factor Location

$E_R$  - Exhibits 23-8, 23-10  
 $E_T$  - Exhibits 23-8, 23-10, 23-11  
 $f_p$  - Page 23-12  
 LOS, S, FFS,  $v_p$  - Exhibits 23-2, 23-3

$f_{LW}$  - Exhibit 23-4  
 $f_{LC}$  - Exhibit 23-5  
 $f_N$  - Exhibit 23-6  
 $f_{ID}$  - Exhibit 23-7

# BASIC FREEWAY SEGMENTS WORKSHEET



Application	Input	Output
Operational (LOS)	FFS, N, $v_p$	LOS, S, D
Design (N)	FFS, LOS, $v_p$	N, S, D
Design ( $v_p$ )	FFS, LOS, N	$v_p$ , S, D
Planning (LOS)	FFS, N, AADT	LOS, S, D
Planning (N)	FFS, LOS, AADT	N, S, D
Planning ( $v_p$ )	FFS, LOS, N	$v_p$ , S, D

General Information		Site Information	
Analyst	KNM	Highway/Direction of Travel	I-4/Eastbound
Agency or Company	HNTB	From/To	CR 46A/SR 417/SR 46
Date Performed	3/25/2008	Jurisdiction	
Analysis Time Period	Peak	Analysis Year	2012 Build
Project Description: Wekiva Parkway PD&E			

<input checked="" type="checkbox"/> Oper. (LOS)	<input type="checkbox"/> Des. (N)	<input type="checkbox"/> Planning Data
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Flow Inputs			
Volume, V	4220	veh/h	Peak-Hour Factor, PHF
AADT		veh/day	0.95
Peak-Hr Prop. of AADT, K			%Trucks and Buses, $P_T$
Peak-Hr Direction Prop, D			9
DDHV = AADT x K x D		veh/h	%RVs, $P_R$
Driver type adjustment	1.00		0
			General Terrain:
			Level
			Grade % Length
			mi
			Up/Down %

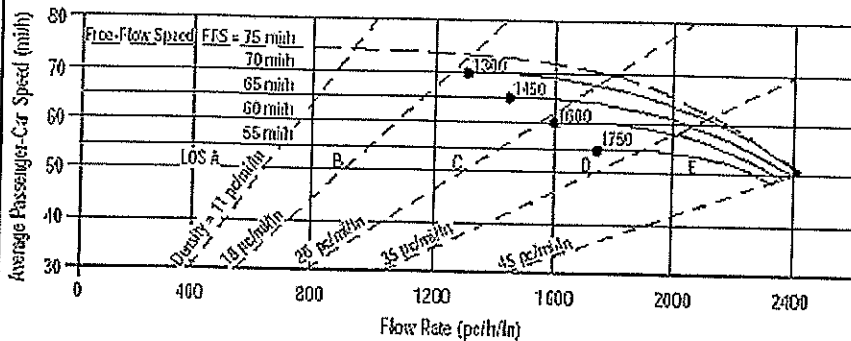
Calculate Flow Adjustments			
$f_p$	1.00	$E_R$	1.2
$E_T$	1.5	$f_{HV} = 1/(1+P_T(E_T-1) + P_R(E_R-1))$	0.957

Speed Inputs		Calc Speed Adj and FFS	
Lane Width	12.0 ft	$f_{LW}$	0.0 mi/h
Rt-Shoulder Lat. Clearance	6.0 ft	$f_{LC}$	0.0 mi/h
Interchange Density	0.54 l/mi	$f_{ID}$	0.2 mi/h
Number of Lanes, N	3	$f_N$	3.0 mi/h
FFS (measured)		FFS	66.8 mi/h
Base free-flow Speed, BFFS	70.0 mi/h		

LOS and Performance Measures		Design (N)	
Operational (LOS)		Design (N)	
$v_p = (V \text{ or } DDHV) / (PHF \times N \times f_{HV} \times f_p)$	1547 pc/h/ln	Design LOS	
S	66.7 mi/h	$v_p = (V \text{ or } DDHV) / (PHF \times N \times f_{HV} \times f_p)$	pc/h
$D = v_p / S$	23.2 pc/mi/ln	S	mi/h
LOS	C	$D = v_p / S$	pc/mi/ln
		Required Number of Lanes, N	

Glossary		Factor Location	
N - Number of lanes	S - Speed	$E_R$ - Exhibits 23-8, 23-10	$f_{LW}$ - Exhibit 23-4
V - Hourly volume	D - Density	$E_T$ - Exhibits 23-8, 23-10, 23-11	$f_{LC}$ - Exhibit 23-5
$v_p$ - Flow rate	FFS - Free-flow speed	$f_p$ - Page 23-12	$f_N$ - Exhibit 23-6
LOS - Level of service	BFFS - Base free-flow speed	LOS, S, FFS, $v_p$ - Exhibits 23-2, 23-3	$f_{ID}$ - Exhibit 23-7
DDHV - Directional design hour volume			

# BASIC FREEWAY SEGMENTS WORKSHEET



Application	Input	Output
Operational (LOS)	FFS, N, $v_p$	LOS, S, D
Design (N)	FFS, LOS, $v_p$	N, S, D
Design ( $v_p$ )	FFS, LOS, N	$v_p$ , S, D
Planning (LOS)	FFS, N, AADT	LOS, S, D
Planning (N)	FFS, LOS, AADT	N, S, D
Planning ( $v_p$ )	FFS, LOS, N	$v_p$ , S, D

## General Information

Analyst: *KNM*  
 Agency or Company: *HNTB*  
 Date Performed: *3/25/2008*  
 Analysis Time Period: *Peak*

## Site Information

Highway/Direction of Travel: *I-4/Eastbound*  
 From/To: *SR 46/US17/92*  
 Jurisdiction:  
 Analysis Year: *2012 Build*

Project Description: *Wekiva Parkway PD&E*

Oper. (LOS)

Des. (N)

Planning Data

## Flow Inputs

Volume, V: *5640* veh/h  
 AADT: veh/day  
 Peak-Hr Prop. of AADT, K:  
 Peak-Hr Direction Prop., D:  
 DDHV = AADT x K x D  
 Driver type adjustment: *1.00* veh/h

Peak-Hour Factor, PHF: *0.95*  
 %Trucks and Buses,  $P_T$ : *9*  
 %RVs,  $P_R$ : *0*  
 General Terrain: *Level*  
 Grade % Length: *mi*  
 Up/Down %

## Calculate Flow Adjustments

$f_p$ : *1.00*  
 $E_T$ : *1.5*  
 $E_R$ : *1.2*  
 $f_{HV} = 1/[1+P_T(E_T - 1) + P_R(E_R - 1)]$ : *0.957*

## Speed Inputs

Lane Width: *12.0* ft  
 Rt-Shoulder Lat. Clearance: *6.0* ft  
 Interchange Density: *0.67* l/mi  
 Number of Lanes, N: *4*  
 FFS (measured): mi/h  
 Base free-flow Speed, BFFS: *70.0* mi/h

## Calc Speed Adj and FFS

$f_{LW}$ : *0.0* mi/h  
 $f_{LC}$ : *0.0* mi/h  
 $f_{ID}$ : *0.9* mi/h  
 $f_N$ : *1.5* mi/h  
 FFS: *67.6* mi/h

## LOS and Performance Measures

Operational (LOS)  
 $v_p = (V \text{ or } DDHV) / (PHF \times N \times f_{HV} \times f_p)$ : *1551* pc/h/ln  
 S: *67.4* mi/h  
 $D = v_p / S$ : *23.0* pc/mi/ln  
 LOS: *C*

## Design (N)

Design (N)  
 Design LOS  
 $v_p = (V \text{ or } DDHV) / (PHF \times N \times f_{HV} \times f_p)$ : pc/h  
 S: mi/h  
 $D = v_p / S$ : pc/mi/ln  
 Required Number of Lanes, N

## Glossary

N - Number of lanes  
 V - Hourly volume  
 $v_p$  - Flow rate  
 LOS - Level of service  
 DDHV - Directional design hour volume

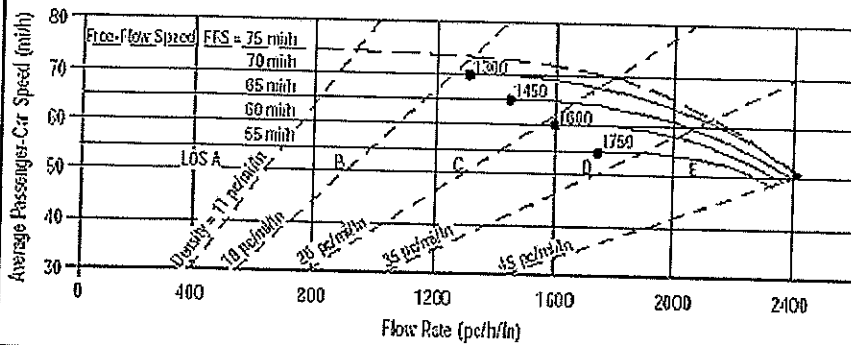
S - Speed  
 D - Density  
 FFS - Free-flow speed  
 BFFS - Base free-flow speed

## Factor Location

$E_R$  - Exhibits 23-8, 23-10  
 $E_T$  - Exhibits 23-8, 23-10, 23-11  
 $f_p$  - Page 23-12  
 LOS, S, FFS,  $v_p$  - Exhibits 23-2, 23-3

$f_{LW}$  - Exhibit 23-4  
 $f_{LC}$  - Exhibit 23-5  
 $f_N$  - Exhibit 23-6  
 $f_{ID}$  - Exhibit 23-7

# BASIC FREEWAY SEGMENTS WORKSHEET



Application	Input	Output
Operational (LOS)	FFS, N, $v_p$	LOS, S, D
Design (N)	FFS, LOS, $v_p$	N, S, D
Design ( $v_p$ )	FFS, LOS, N	$v_p$ , S, D
Planning (LOS)	FFS, N, AADT	LOS, S, D
Planning (N)	FFS, LOS, AADT	N, S, D
Planning ( $v_p$ )	FFS, LOS, N	$v_p$ , S, D

## General Information

Analyst: *KNM*  
 Agency or Company: *HNTB*  
 Date Performed: *3/25/2008*  
 Analysis Time Period: *Peak*

## Site Information

Highway/Direction of Travel: *I-4/Eastbound*  
 From/To: *Us17/92 to Volusia County Line*  
 Jurisdiction:  
 Analysis Year: *2012 Build*

Project Description: *Wekiva Parkway PD&E*

Oper. (LOS)

Des. (N)

Planning Data

## Flow Inputs

Volume, V: *5340* veh/h  
 AADT: veh/day  
 Peak-Hr Prop. of AADT, K: *0.95*  
 Peak-Hr Direction Prop, D: *9*  
 DDHV = AADT x K x D: *0*  
 Driver type adjustment: *1.00* veh/h  
 General Terrain: *Level*  
 Grade % Length: *mi*  
 Up/Down %

## Calculate Flow Adjustments

$f_p$ : *1.00*  
 $E_T$ : *1.5*  
 $E_R$ : *1.2*  
 $f_{HV} = 1 / (1 + P_T(E_T - 1) + P_R(E_R - 1))$ : *0.957*

## Speed Inputs

Lane Width: *12.0* ft  
 Rt-Shoulder Lat. Clearance: *6.0* ft  
 Interchange Density: *2.00* l/mi  
 Number of Lanes, N: *3*  
 FFS (measured): mi/h  
 Base free-flow Speed, BFFS: *70.0* mi/h

## Calc Speed Adj and FFS

$f_{LW}$ : *0.0* mi/h  
 $f_{LC}$ : *0.0* mi/h  
 $f_{ID}$ : *7.5* mi/h  
 $f_N$ : *3.0* mi/h  
 FFS: *59.5* mi/h

## LOS and Performance Measures

Operational (LOS)  
 $v_p = (V \text{ or } DDHV) / (PHF \times N \times f_{HV} \times f_p)$ : *1958* pc/h/ln  
 S: *58.1* mi/h  
 $D = v_p / S$ : *33.7* pc/mi/ln  
 LOS: *D*

## Design (N)

Design (N)  
 Design LOS  
 $v_p = (V \text{ or } DDHV) / (PHF \times N \times f_{HV} \times f_p)$ : pc/h  
 S: mi/h  
 $D = v_p / S$ : pc/mi/ln  
 Required Number of Lanes, N

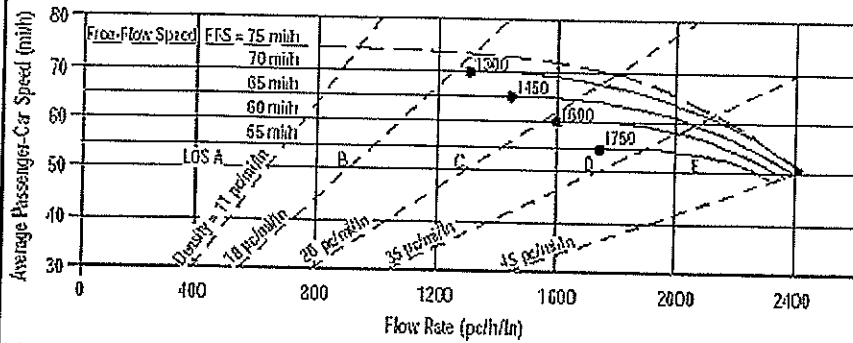
## Glossary

N - Number of lanes  
 V - Hourly volume  
 $v_p$  - Flow rate  
 LOS - Level of service  
 DDHV - Directional design hour volume  
 S - Speed  
 D - Density  
 FFS - Free-flow speed  
 BFFS - Base free-flow speed

## Factor Location

$E_R$  - Exhibits 23-8, 23-10  
 $E_T$  - Exhibits 23-8, 23-10, 23-11  
 $f_p$  - Page 23-12  
 LOS, S, FFS,  $v_p$  - Exhibits 23-2, 23-3  
 $f_{LW}$  - Exhibit 23-4  
 $f_{LC}$  - Exhibit 23-5  
 $f_N$  - Exhibit 23-6  
 $f_{ID}$  - Exhibit 23-7

## BASIC FREEWAY SEGMENTS WORKSHEET



Application	Input	Output
Operational (LOS)	FFS, N, $v_p$	LOS, S, D
Design (N)	FFS, LOS, $v_p$	N, S, D
Design ( $v_p$ )	FFS, LOS, N	$v_p$ , S, D
Planning (LOS)	FFS, N, AADT	LOS, S, D
Planning (N)	FFS, LOS, AADT	N, S, D
Planning ( $v_p$ )	FFS, LOS, N	$v_p$ , S, D

### General Information

Analyst: *KNM*  
 Agency or Company: *HNTB*  
 Date Performed: *3/25/2008*  
 Analysis Time Period: *Peak*  
 Project Description: *Wekiva Parkway PD&E*

### Site Information

Highway/Direction of Travel: *SR 417/Westbound*  
 From/To: *North of Rinehart Rd/Rinehart*  
 Jurisdiction:  
 Analysis Year: *2012 Build*

Oper.(LOS)                     
  Des.(N)                     
  Planning Data

### Flow Inputs

Volume, V: *3370* veh/h                      Peak-Hour Factor, PHF: *0.95*  
 AADT: veh/day                      %Trucks and Buses,  $P_T$ : *10*  
 Peak-Hr Prop. of AADT, K:                      %RVs,  $P_R$ : *0*  
 Peak-Hr Direction Prop, D:                      General Terrain: *Level*  
 DDHV = AADT x K x D: veh/h                      Grade % Length: *mi*  
 Driver type adjustment: *1.00*                      Up/Down %

### Calculate Flow Adjustments

$f_p$ : *1.00*                       $E_R$ : *1.2*  
 $E_T$ : *1.5*                       $f_{HV} = 1/[1+P_T(E_T - 1) + P_R(E_R - 1)]$ : *0.952*

### Speed Inputs

Lane Width: *12.0* ft  
 Rt-Shoulder Lat. Clearance: *6.0* ft  
 Interchange Density: *0.40* l/mi  
 Number of Lanes, N: *3*  
 FFS (measured): mi/h  
 Base free-flow Speed, BFFS: *70.0* mi/h

### Calc Speed Adj and FFS

$f_{LW}$ : *0.0* mi/h  
 $f_{LC}$ : *0.0* mi/h  
 $f_{ID}$ : *0.0* mi/h  
 $f_N$ : *3.0* mi/h  
 FFS: *67.0* mi/h

### LOS and Performance Measures

Operational (LOS)  
 $v_p = (V \text{ or } DDHV) / (PHF \times N \times f_{HV} \times f_p)$ : *1242* pc/h/ln  
 S: *67.0* mi/h  
 $D = v_p / S$ : *18.5* pc/mi/ln  
 LOS: *C*

### Design (N)

Design (N)  
 Design LOS  
 $v_p = (V \text{ or } DDHV) / (PHF \times N \times f_{HV} \times f_p)$ : pc/h  
 S: mi/h  
 $D = v_p / S$ : pc/mi/ln  
 Required Number of Lanes, N

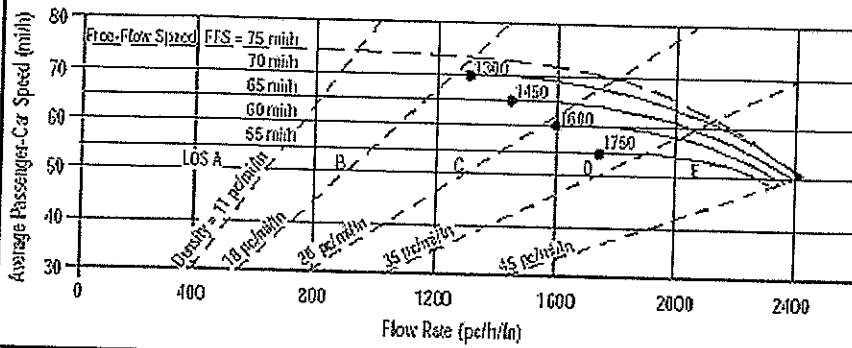
### Glossary

N - Number of lanes                      S - Speed  
 V - Hourly volume                      D - Density  
 $v_p$  - Flow rate                      FFS - Free-flow speed  
 LOS - Level of service                      BFFS - Base free-flow speed  
 DDHV - Directional design hour volume

### Factor Location

$E_R$  - Exhibits 23-8, 23-10                       $f_{LW}$  - Exhibit 23-4  
 $E_T$  - Exhibits 23-8, 23-10, 23-11                       $f_{LC}$  - Exhibit 23-5  
 $f_p$  - Page 23-12                       $f_N$  - Exhibit 23-6  
 LOS, S, FFS,  $v_p$  - Exhibits 23-2, 23-3                       $f_{ID}$  - Exhibit 23-7

## BASIC FREEWAY SEGMENTS WORKSHEET



Application	Input	Output
Operational (LOS)	FFS, N, $v_p$	LOS, S, D
Design (N)	FFS, LOS, $v_p$	N, S, D
Design ( $v_p$ )	FFS, LOS, N	$v_p$ , S, D
Planning (LOS)	FFS, N, AADT	LOS, S, D
Planning (N)	FFS, LOS, AADT	N, S, D
Planning ( $v_p$ )	FFS, LOS, N	$v_p$ , S, D

### General Information

Analyst: *KNM*  
 Agency or Company: *HNTB*  
 Date Performed: *3/25/2008*  
 Analysis Time Period: *Peak*  
 Project Description: *Wekiva Parkway PD&E*

### Site Information

Highway/Direction of Travel: *SR 417/Westbound*  
 From/To: *Rinehart Rd to I-4*  
 Jurisdiction:  
 Analysis Year: *2012 Build*

 Oper.(LOS)

 Des.(N)

 Planning Data

### Flow Inputs

Volume, V	2810	veh/h	Peak-Hour Factor, PHF	0.95
AADT		veh/day	%Trucks and Buses, $P_T$	10
Peak-Hr Prop. of AADT, K			%RVs, $P_R$	0
Peak-Hr Direction Prop, D			General Terrain:	Level
DDHV = AADT x K x D		veh/h	Grade %	Length mi
Driver type adjustment	1.00		Up/Down %	

### Calculate Flow Adjustments

$f_p$	1.00	$E_R$	1.2
$E_T$	1.5	$f_{HV} = 1/[1+P_T(E_T - 1) + P_R(E_R - 1)]$	0.952

### Speed Inputs

Lane Width	12.0	ft
Rt-Shoulder Lat. Clearance	6.0	ft
Interchange Density	2.00	l/mi
Number of Lanes, N	3	
FFS (measured)		mi/h
Base free-flow Speed, BFFS	70.0	mi/h

### Calc Speed Adj and FFS

$f_{LW}$	0.0	mi/h
$f_{LC}$	0.0	mi/h
$f_{ID}$	7.5	mi/h
$f_N$	3.0	mi/h
FFS	59.5	mi/h

### LOS and Performance Measures

Operational (LOS)  
 $v_p = (V \text{ or } DDHV) / (PHF \times N \times f_{HV} \times f_p)$  1035 pc/h/ln  
 S 59.5 mi/h  
 $D = v_p / S$  17.4 pc/mi/ln  
 LOS B

### Design (N)

Design (N)  
 Design LOS  
 $v_p = (V \text{ or } DDHV) / (PHF \times N \times f_{HV} \times f_p)$  pc/h  
 S mi/h  
 $D = v_p / S$  pc/mi/ln  
 Required Number of Lanes, N

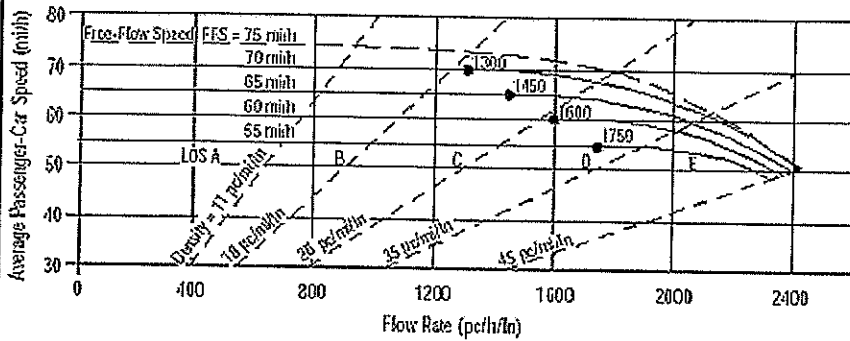
### Glossary

N - Number of lanes  
 V - Hourly volume  
 $v_p$  - Flow rate  
 LOS - Level of service  
 DDHV - Directional design hour volume  
 S - Speed  
 D - Density  
 FFS - Free-flow speed  
 BFFS - Base free-flow speed

### Factor Location

$E_R$  - Exhibits 23-8, 23-10  
 $E_T$  - Exhibits 23-8, 23-10, 23-11  
 $f_p$  - Page 23-12  
 LOS, S, FFS,  $v_p$  - Exhibits 23-2, 23-3  
 $f_{LW}$  - Exhibit 23-4  
 $f_{LC}$  - Exhibit 23-5  
 $f_N$  - Exhibit 23-6  
 $f_{ID}$  - Exhibit 23-7

## BASIC FREEWAY SEGMENTS WORKSHEET



Application	Input	Output
Operational (LOS)	FFS, N, $v_p$	LOS, S, D
Design (N)	FFS, LOS, $v_p$	N, S, D
Design ( $v_p$ )	FFS, LOS, N	$v_p$ , S, D
Planning (LOS)	FFS, N, AADT	LOS, S, D
Planning (N)	FFS, LOS, AADT	N, S, D
Planning ( $v_p$ )	FFS, LOS, N	$v_p$ , S, D

### General Information

Analyst	KNM
Agency or Company	HNTB
Date Performed	3/25/2008
Analysis Time Period	Peak
Project Description	Wekiva Parkway PD&E

### Site Information

Highway/Direction of Travel	SR 417/Westbound
From/To	I-4 to CD Road
Jurisdiction	
Analysis Year	2012 Build

- Oper.(LOS)                     
  Des.(N)                     
  Planning Data

### Flow Inputs

Volume, V	2050	veh/h	Peak-Hour Factor, PHF	0.95
AADT		veh/day	%Trucks and Buses, $P_T$	10
Peak-Hr Prop. of AADT, K			%RVs, $P_R$	0
Peak-Hr Direction Prop, D			General Terrain:	Level
DDHV = AADT x K x D		veh/h	Grade %	Length mi
Driver type adjustment	1.00		Up/Down %	

### Calculate Flow Adjustments

$f_p$	1.00	$E_R$	1.2
$E_T$	1.5	$f_{HV} = 1/[1+P_T(E_T - 1) + P_R(E_R - 1)]$	0.952

### Speed Inputs

Lane Width	12.0	ft
Rt-Shoulder Lat. Clearance	6.0	ft
Interchange Density	2.00	l/mi
Number of Lanes, N	3	
FFS (measured)		mi/h
Base free-flow Speed, BFFS	70.0	mi/h

### Calc Speed Adj and FFS

$f_{LW}$	0.0	mi/h
$f_{LC}$	0.0	mi/h
$f_{ID}$	7.5	mi/h
$f_N$	3.0	mi/h
FFS	59.5	mi/h

### LOS and Performance Measures

Operational (LOS)		
$v_p = (V \text{ or } DDHV) / (PHF \times N \times f_{HV} \times f_p)$	755	pc/h/ln
S	59.5	mi/h
$D = v_p / S$	12.7	pc/mi/ln
LOS	B	

### Design (N)

Design (N)	
Design LOS	
$v_p = (V \text{ or } DDHV) / (PHF \times N \times f_{HV} \times f_p)$	pc/h
S	mi/h
$D = v_p / S$	pc/mi/ln
Required Number of Lanes, N	

### Glossary

N - Number of lanes	S - Speed
V - Hourly volume	D - Density
$v_p$ - Flow rate	FFS - Free-flow speed
LOS - Level of service	BFFS - Base free-flow speed
DDHV - Directional design hour volume	

### Factor Location

$E_R$ - Exhibits 23-8, 23-10	$f_{LW}$ - Exhibit 23-4
$E_T$ - Exhibits 23-8, 23-10, 23-11	$f_{LC}$ - Exhibit 23-5
$f_p$ - Page 23-12	$f_N$ - Exhibit 23-6
LOS, S, FFS, $v_p$ - Exhibits 23-2, 23-3	$f_{ID}$ - Exhibit 23-7



Phone: Fax:  
E-mail:

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Operational Analysis

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Analyst: CTR  
 Agency or Company: HNTB  
 Date Performed: 08/02/10  
 Analysis Time Period: Build Service Road Concept  
 Freeway/Direction: I-4 WB  
 From/To: US 17/92 WB On to Off to SR 46  
 Jurisdiction: Seminole County  
 Analysis Year: 2012  
 Description: Wekiva Parkway PD&E

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Flow Inputs and Adjustments

---

Volume, V	5650	veh/h
Peak-hour factor, PHF	0.92	
Peak 15-min volume, v15	1535	v
Trucks and buses	9	%
Recreational vehicles	0	%
Terrain type:	Level	
Grade	0.00	%
Segment length	0.00	mi
Trucks and buses PCE, ET	1.5	
Recreational vehicle PCE, ER	1.2	
Heavy vehicle adjustment, fHV	0.957	
Driver population factor, fp	1.00	
Flow rate, vp	1604	pc/h/ln

---

Speed Inputs and Adjustments

---

Lane width	12.0	ft
Right-shoulder lateral clearance	6.0	ft
Interchange density	0.67	interchange/mi
Number of lanes, N	4	
Free-flow speed:	Base	
FFS or BFFS	70.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.9	mi/h
Number of lanes adjustment, fN	1.5	mi/h
Free-flow speed, FFS	67.6	mi/h
	Urban Freeway	

---

LOS and Performance Measures

---

Flow rate, vp	1604	pc/h/ln
Free-flow speed, FFS	67.6	mi/h
Average passenger-car speed, S	67.3	mi/h
Number of lanes, N	4	
Density, D	23.8	pc/mi/ln

Level of service, LOS

C

Overall results are not computed when free-flow speed is less than 55 mph.

Phone:  
E-mail:

Fax:

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Operational Analysis

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Analyst: CTR  
 Agency or Company: HNTB  
 Date Performed: 8/02/2010  
 Analysis Time Period: Build Service Road Concept  
 Freeway/Direction: CD Road/EB  
 From/To: SR 417 EB On to Off to I-4 EB  
 Jurisdiction: Seminole County  
 Analysis Year: 2012  
 Description: Wekiva Parkway PD&E

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Flow Inputs and Adjustments

---

Volume, V	1380	veh/h
Peak-hour factor, PHF	0.92	
Peak 15-min volume, v15	375	v
Trucks and buses	9	%
Recreational vehicles	0	%
Terrain type:	Level	
Grade	0.00	%
Segment length	0.00	mi
Trucks and buses PCE, ET	1.5	
Recreational vehicle PCE, ER	1.2	
Heavy vehicle adjustment, fHV	0.957	
Driver population factor, fp	1.00	
Flow rate, vp	522	pc/h/ln

---

Speed Inputs and Adjustments

---

Lane width	12.0	ft
Right-shoulder lateral clearance	6.0	ft
Interchange density	0.54	interchange/mi
Number of lanes, N	3	
Free-flow speed:	Base	
FFS or BFFS	70.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.2	mi/h
Number of lanes adjustment, fN	3.0	mi/h
Free-flow speed, FFS	66.8	mi/h
	Urban Freeway	

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LOS and Performance Measures

---

Flow rate, vp	522	pc/h/ln
Free-flow speed, FFS	66.8	mi/h
Average passenger-car speed, S	66.8	mi/h
Number of lanes, N	3	
Density, D	7.8	pc/mi/ln

Level of service, LOS

A

Overall results are not computed when free-flow speed is less than 55 mph.

Phone: Fax:  
 E-mail:

Operational Analysis

Analyst: CTR  
 Agency or Company: HNTB  
 Date Performed: 8/02/2010  
 Analysis Time Period: Build Service Road Concept  
 Freeway/Direction: CD Road/WB  
 From/To: I-4 WB On to Off to SR 417 WB  
 Jurisdiction: Seminole County  
 Analysis Year: 2012  
 Description: Wekiva Parkway PD&E

Flow Inputs and Adjustments

Volume, V	2760	veh/h
Peak-hour factor, PHF	0.92	
Peak 15-min volume, v15	750	v
Trucks and buses	9	%
Recreational vehicles	0	%
Terrain type:	Level	
Grade	0.00	%
Segment length	0.00	mi
Trucks and buses PCE, ET	1.5	
Recreational vehicle PCE, ER	1.2	
Heavy vehicle adjustment, fHV	0.957	
Driver population factor, fp	1.00	
Flow rate, vp	1045	pc/h/ln

Speed Inputs and Adjustments

Lane width	12.0	ft
Right-shoulder lateral clearance	6.0	ft
Interchange density	0.54	interchange/mi
Number of lanes, N	3	
Free-flow speed:	Base	
FFS or BFFS	70.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.2	mi/h
Number of lanes adjustment, fN	3.0	mi/h
Free-flow speed, FFS	66.8	mi/h
	Urban Freeway	

LOS and Performance Measures

Flow rate, vp	1045	pc/h/ln
Free-flow speed, FFS	66.8	mi/h
Average passenger-car speed, S	66.8	mi/h
Number of lanes, N	3	
Density, D	15.6	pc/mi/ln

Level of service, LOS

B

Overall results are not computed when free-flow speed is less than 55 mph.

Phone: Fax:  
 E-mail:

Operational Analysis

Analyst: CTR  
 Agency or Company: HNTB  
 Date Performed: 8/02/2010  
 Analysis Time Period: Build Service Road Concept  
 Freeway/Direction: I-4 EB  
 From/To: SR 46 On to US 17/92 Off  
 Jurisdiction: Seminole County  
 Analysis Year: 2012  
 Description: Wekiva Parkway PD&E

Flow Inputs and Adjustments

Volume, V	5650	veh/h
Peak-hour factor, PHF	0.92	
Peak 15-min volume, v15	1535	v
Trucks and buses	9	%
Recreational vehicles	0	%
Terrain type:	Level	
Grade	0.00	%
Segment length	0.00	mi
Trucks and buses PCE, ET	1.5	
Recreational vehicle PCE, ER	1.2	
Heavy vehicle adjustment, fHV	0.957	
Driver population factor, fp	1.00	
Flow rate, vp	1604	pc/h/ln

Speed Inputs and Adjustments

Lane width	12.0	ft
Right-shoulder lateral clearance	6.0	ft
Interchange density	0.54	interchange/mi
Number of lanes, N	4	
Free-flow speed:	Base	
FFS or BFFS	70.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	0.2	mi/h
Number of lanes adjustment, fN	1.5	mi/h
Free-flow speed, FFS	68.3	mi/h
	Urban Freeway	

LOS and Performance Measures

Flow rate, vp	1604	pc/h/ln
Free-flow speed, FFS	68.3	mi/h
Average passenger-car speed, S	67.9	mi/h
Number of lanes, N	4	
Density, D	23.6	pc/mi/ln

Level of service, LOS

C

Overall results are not computed when free-flow speed is less than 55 mph.



Phone:  
E-mail:

Fax:

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Operational Analysis

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Analyst: CTR  
 Agency or Company: HNTB  
 Date Performed: 8/03/2010  
 Analysis Time Period: Build Service Road Concept  
 Freeway/Direction: SR 417 WB  
 From/To: On Ramp from I-4 EB & WB  
 Jurisdiction: Seminole County  
 Analysis Year: 2012  
 Description: Wekiva Parkway PD&E

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Flow Inputs and Adjustments

---

Volume, V	1490	veh/h
Peak-hour factor, PHF	0.92	
Peak 15-min volume, v15	405	v
Trucks and buses	11	%
Recreational vehicles	0	%
Terrain type:	Level	
Grade	0.00	%
Segment length	0.00	mi
Trucks and buses PCE, ET	1.5	
Recreational vehicle PCE, ER	1.2	
Heavy vehicle adjustment, fHV	0.948	
Driver population factor, fp	1.00	
Flow rate, vp	427	pc/h/ln

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Speed Inputs and Adjustments

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Lane width	12.0	ft
Right-shoulder lateral clearance	6.0	ft
Interchange density	2.00	interchange/mi
Number of lanes, N	4	
Free-flow speed:	Base	
FFS or BFFS	70.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	7.5	mi/h
Number of lanes adjustment, fN	1.5	mi/h
Free-flow speed, FFS	61.0	mi/h

Urban Freeway

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LOS and Performance Measures

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Flow rate, vp	427	pc/h/ln
Free-flow speed, FFS	61.0	mi/h
Average passenger-car speed, S	61.0	mi/h
Number of lanes, N	4	
Density, D	7.0	pc/mi/ln

Level of service, LOS

A

Overall results are not computed when free-flow speed is less than 55 mph.

I-4 WB On Ramp from CR 46A & SR 46.txt  
HCS+: Basic Freeway Segments Release 5.4

Phone: Fax:  
E-mail:

Operational Analysis

Analyst: CTR  
Agency or Company: HNTB  
Date Performed: 8/10/2010  
Analysis Time Period: Build  
Freeway/Direction: I-4 WB  
From/To: On Ramp from CR 46A & SR 46  
Jurisdiction: Seminole County  
Analysis Year: 2012  
Description: Wekiva Parkway PD&E

Flow Inputs and Adjustments

Volume, V	5530	veh/h
Peak-hour factor, PHF	0.92	
Peak 15-min volume, v15	1503	v
Trucks and buses	9	%
Recreational vehicles	0	%
Terrain type:	Level	
Grade	0.00	%
Segment length	0.00	mi
Trucks and buses PCE, ET	1.5	
Recreational vehicle PCE, ER	1.2	
Heavy vehicle adjustment, fhv	0.957	
Driver population factor, fp	1.00	
Flow rate, vp	1570	pc/h/ln

Speed Inputs and Adjustments

Lane width	12.0	ft
Right-shoulder lateral clearance	6.0	ft
Interchange density	0.90	interchange/mi
Number of lanes, N	4	
Free-flow speed:	Base	
FFS or BFFS	70.0	mi/h
Lane width adjustment, flw	0.0	mi/h
Lateral clearance adjustment, flc	0.0	mi/h
Interchange density adjustment, fid	2.0	mi/h
Number of lanes adjustment, fn	1.5	mi/h
Free-flow speed, FFS	66.5	mi/h
	Urban Freeway	

LOS and Performance Measures

Flow rate, vp	1570	pc/h/ln
Free-flow speed, FFS	66.5	mi/h
Average passenger-car speed, S	66.3	mi/h
Number of lanes, N	4	
Density, D	23.7	pc/mi/ln
Level of service, LOS	C	

I-4 WB On Ramp from CR 46A & SR 46.txt  
Overall results are not computed when free-flow speed is less than 55 mph.

SR 417 WB ON Ramp from Rinehart.txt  
HCS+: Ramps and Ramp Junctions Release 5.4

Phone: Fax:  
E-mail:

Merge Analysis

Analyst: CTR  
Agency/Co.: HNTB  
Date performed: 8/03/2010  
Analysis time period: Build Service Road Concept  
Freeway/Dir of Travel: SR 417 WB  
Junction: On Ramp from Rinehart Rd  
Jurisdiction: Seminole County  
Analysis Year: 2012  
Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	3	
Free-flow speed on freeway	55.0	mph
Volume on freeway	2510	vph

On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	35.0	mph
Volume on ramp	220	vph
Length of first accel/decel lane	1500	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent Ramp	860	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	Off	
Distance to adjacent Ramp	1700	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	2510	220	860	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	682	60	234	v
Trucks and buses	10	10	10	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.952	0.952	0.952	
Driver population factor, fP	1.00	1.00	1.00	

Flow rate, vp      SR 417 WB ON Ramp from Rinehart.txt      982      pcph  
 2865      251

Estimation of V<sub>12</sub> Merge Areas

L = 761.02 (Equation 25-2 or 25-3)  
 EQ  
 P = 0.619 Using Equation 1  
 FM  
 $v_{12} = v_{F} (P_{FM}) = 1775 \text{ pc/h}$

Capacity Checks

	Actual	Maximum	LOS F?
v <sub>FO</sub>	3116	6750	No
v <sub>3 or av34</sub>	1090 pc/h	(Equation 25-4 or 25-5)	
Is v <sub>3 or av34</sub> > 2700 pc/h?		No	
Is v <sub>3 or av34</sub> > 1.5 v <sub>12</sub> / 2		No	
If yes, v <sub>12A</sub> = 1775		(Equation 25-8)	

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v <sub>R12</sub>	1775	4600	No

Level of Service Determination (if not F)

Density,  $D = 5.475 + 0.00734 v_{R} + 0.0078 v_{12} - 0.00627 L_{A}$  = 11.8 pc/mi/ln  
 Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable,      M = 0.246  
 Space mean speed in ramp influence area,      S<sub>S</sub> = 51.8      mph  
 Space mean speed in outer lanes,      S<sub>R</sub> = 52.9      mph  
 Space mean speed for all vehicles,      S<sub>O</sub> = 52.2      mph

Phone: \_\_\_\_\_ Fax: \_\_\_\_\_  
 E-mail: \_\_\_\_\_

Diverge Analysis

Analyst: CTR  
 Agency/Co.: HNTB  
 Date performed: 8/02/2010  
 Analysis time period: Build Service Road Concept  
 Freeway/Dir of Travel: SR 417 WB  
 Junction: Off Ramp to Rinehart Rd  
 Jurisdiction: Seminole County  
 Analysis Year: 2012  
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	3	
Free-flow speed on freeway	55.0	mph
Volume on freeway	3370 ✓	vph

Off Ramp Data

Side of freeway	Right	
Number of lanes in ramp	2	
Free-Flow speed on ramp	35.0 ✓	mph
Volume on ramp	860 ✓	vph
Length of first accel/decel lane	500	ft
Length of second accel/decel lane	0	ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes ✓	
Volume on adjacent ramp	220 ✓	vph
Position of adjacent ramp	Downstream	
Type of adjacent ramp	On	
Distance to adjacent ramp	1833	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	3370 ✓	860	220	vph
Peak-hour factor, PHF	0.92 ✓	0.92	0.92	
Peak 15-min volume, v15	916 ✓	234	60	v
Trucks and buses	10	10	10	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00 %	0.00 %	0.00 %	
Length	0.00 mi	0.00 mi	0.00 mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.952	0.952	0.952	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	3846	982	251	pcph

Estimation of V12 Diverge Areas

$$L = \frac{EQ}{P} \quad (\text{Equation 25-8 or 25-9})$$

$$P = 0.450 \quad \text{Using Equation 0}$$

$$v_{12R} = v_{FR} + (v_{FR} - v_{FD}) P = 2271 \quad \text{pc/h}$$

Capacity Checks

	Actual	Maximum	LOS F?
$v_{Fi} = v_{FR}$	3846	6750	No
$v_{FO} = v_{FR} - v_{R}$	2864	6750	No
$v_{R}$	982	3800	No
$v_{3 \text{ or } av34}$	1575 pc/h	(Equation 25-15 or 25-16)	
Is $v_{3 \text{ or } av34} > 2700$ pc/h?		No	
Is $v_{3 \text{ or } av34} > 1.5 v_{12} / 2$		No	
If yes, $v_{12A} = 2271$		(Equation 25-18)	

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
$v_{12}$	2271	4400	No

Level of Service Determination (if not F)

Density,  $D = 4.252 + 0.0086 v_{12} - 0.009 L_D = 14.8$  pc/mi/ln

Level of service for ramp-freeway junction areas of influence B ✓

Speed Estimation

Intermediate speed variable,	$D = 0.516$	
Space mean speed in ramp influence area,	$S = 48.3$	mph
Space mean speed in outer lanes,	$S = 58.1$	mph
Space mean speed for all vehicles,	$S = 51.9$	mph



Phone:  
E-mail:

Fax:

Merge Analysis

Analyst: CTR  
 Agency/Co.: HNTB  
 Date performed: 8/02/2010  
 Analysis time period: Build Service Road Concept  
 Freeway/Dir of Travel: SR 417 EB  
 Junction: On Ramp from Rinehart Rd  
 Jurisdiction: Seminole County  
 Analysis Year: 2012  
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	3	
Free-flow speed on freeway	55.0 ✓	mph
Volume on freeway	2550 ✓	vph

On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	35.0	mph
Volume on ramp	820 ✓	vph
Length of first accel/decel lane	1000	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes ✓	
Volume on adjacent Ramp	220 ✓	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	Off	
Distance to adjacent Ramp	2402	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	2550 ✓	820	220	vph
Peak-hour factor, PHF	0.92 ✓	0.92	0.92	
Peak 15-min volume, v15	693 ✓	223	60	v
Trucks and buses	10 ✓	10	10	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.952	0.952	0.952	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	2910	936	251	pcph

Estimation of V12 Merge Areas

$$L = 695.24 \quad (\text{Equation 25-2 or 25-3})$$

$$EQ$$

$$P = 0.605 \quad \text{Using Equation 1}$$

$$FM$$

$$v_{12} = v_{F \quad FM} (P) = 1762 \quad \text{pc/h}$$

Capacity Checks

	Actual	Maximum	LOS F?
v <sub>FO</sub>	3846	6750	No
v <sub>3 or av34</sub>	1148 pc/h	(Equation 25-4 or 25-5)	
Is v <sub>3 or av34</sub> > 2700 pc/h?		No	
Is v <sub>3 or av34</sub> > 1.5 v <sub>12</sub> / 2		No	
If yes, v <sub>12A</sub> = 1762		(Equation 25-8)	

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v <sub>R12</sub>	1762	4600	No

Level of Service Determination (if not F)

$$\text{Density, } D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 19.8 \quad \text{pc/mi/ln}$$

Level of service for ramp-freeway junction areas of influence B ✓

Speed Estimation

Intermediate speed variable,	M = 0.309	
Space mean speed in ramp influence area,	S = 51.0	mph
Space mean speed in outer lanes,	S = 52.7	mph
Space mean speed for all vehicles,	S = 51.5	mph

Phone: Fax:  
 E-mail:

Diverge Analysis

Analyst: CTR  
 Agency/Co.: HNTB  
 Date performed: 8/02/2010  
 Analysis time period: Build Service Road Concept  
 Freeway/Dir of Travel: SR 417 EB  
 Junction: Off Ramp to Rinehart Rd  
 Jurisdiction: Seminole County  
 Analysis Year: 2012  
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	4	
Free-flow speed on freeway	55.0	mph
Volume on freeway	2770 ✓	vph

Off Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	35.0	mph
Volume on ramp	220 ✓	vph
Length of first accel/decel lane	0	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes ✓	
Volume on adjacent ramp	1260 ✓	vph
Position of adjacent ramp	Upstream	
Type of adjacent ramp	On	
Distance to adjacent ramp	3000	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	2770	220	1260	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	753	60	342	v
Trucks and buses	10	10	9	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00 %	0.00 %	0.00 %	
Length	0.00 mi	0.00 mi	0.00 mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.952	0.952	0.957	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	3161	251	1431	pcph

Estimation of V12 Diverge Areas

$$L = \text{(Equation 25-8 or 25-9)}$$

$$EQ$$

$$P = 0.436 \text{ Using Equation 8}$$

$$FD$$

$$v_{12} = v_R + (v_F - v_R) P = 1520 \text{ pc/h}$$

Capacity Checks

	Actual	Maximum	LOS F?
$v_{Fi} = v_F$	3161	9000	No
$v_{FO} = v_F - v_R$	2910	9000	No
$v_R$	251	2000	No
$v_{3 \text{ or } av34}$	820 pc/h	(Equation 25-15 or 25-16)	
Is $v_{3 \text{ or } av34} > 2700 \text{ pc/h?}$		No	
Is $v_{3 \text{ or } av34} > 1.5 v_{12} / 2$		No	
If yes, $v_{12A} = 1520$		(Equation 25-18)	

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
$v_{12}$	1520	4400	No

Level of Service Determination (if not F)

Density,  $D = 4.252 + 0.0086 v_{12} - 0.009 L_D = 17.3 \text{ pc/mi/ln}$

Level of service for ramp-freeway junction areas of influence B ✓

Speed Estimation

Intermediate speed variable,	$D = 0.451$	
Space mean speed in ramp influence area,	$S_R = 49.1$	mph
Space mean speed in outer lanes,	$S_0 = 60.3$	mph
Space mean speed for all vehicles,	$S = 54.4$	mph

Phone:  
E-mail:

Fax:

Diverge Analysis

Analyst: CTR  
 Agency/Co.: HNTB  
 Date performed: 8/02/2010  
 Analysis time period: Build Service Road Concept  
 Freeway/Dir of Travel: SR 417 EB  
 Junction: Off Ramp to Rinehart Rd  
 Jurisdiction: Seminole County  
 Analysis Year: 2012  
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	4	
Free-flow speed on freeway	55.0	mph
Volume on freeway	2770 ✓	vph

Off Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	35.0 ✓	mph
Volume on ramp	220 ✓	vph
Length of first accel/decel lane	0	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes ✓	
Volume on adjacent ramp	820 ✓	vph
Position of adjacent ramp	Downstream	
Type of adjacent ramp	On	
Distance to adjacent ramp	2402	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	2770	220	820	vph
Peak-hour factor, PHF	0.92 ✓	0.92	0.92	
Peak 15-min volume, v15	753 ✓	60	223	v
Trucks and buses	10 ✓	10	9	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00 %	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.952	0.952	0.957	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	3161	251	931	pcph

Estimation of V12 Diverge Areas

$$L = \text{EQ} \quad (\text{Equation 25-8 or 25-9})$$

$$P = 0.436 \quad \text{Using Equation 8}$$

$$v_{12R} = v_{FR} + (v_{FR} - v_{FD}) P = 1520 \quad \text{pc/h}$$

Capacity Checks

	Actual	Maximum	LOS F?
$v_{Fi} = v_F$	3161	9000	No
$v_{FO} = v_F - v_R$	2910	9000	No
$v_R$	251	2000	No
$v_{3 \text{ or } 34} = v_{av34}$	820 pc/h	(Equation 25-15 or 25-16)	
Is $v_{3 \text{ or } 34} > 2700$ pc/h?		No	
Is $v_{3 \text{ or } 34} > 1.5 v_{12} / 2$		No	
If yes, $v_{12A} = 1520$		(Equation 25-18)	

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
$v_{12}$	1520	4400	No

Level of Service Determination (if not F)

Density,  $D = 4.252 + 0.0086 v_{12} - 0.009 L_D = 17.3$  pc/mi/ln

Level of service for ramp-freeway junction areas of influence B ✓

Speed Estimation

Intermediate speed variable,	$D = 0.451$	
Space mean speed in ramp influence area,	$S_R = 49.1$	mph
Space mean speed in outer lanes,	$S_0 = 60.3$	mph
Space mean speed for all vehicles,	$S = 54.4$	mph

HCS+: Ramps and Ramp Junctions Release 5.4

Phone: Fax:  
E-mail:

Diverge Analysis

Analyst: CTRR  
 Agency/Co.: HNTB  
 Date performed: 03/07/2010  
 Analysis time period: Build Service Road Concept  
 Freeway/Dir of Travel: SR 417 WB  
 Junction: Off Ramp to I-4 EB  
 Jurisdiction: Seminole County  
 Analysis Year: 2012  
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	3	
Free-flow speed on freeway	55.0	mph
Volume on freeway	2730	vph

Off Ramp Data

Side of freeway	Right	
Number of lanes in ramp	2	
Free-Flow speed on ramp	35.0	mph
Volume on ramp	1210	vph
Length of first accel/decel lane	500	ft
Length of second accel/decel lane	500	ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent ramp	550	vph
Position of adjacent ramp	Downstream	
Type of adjacent ramp	Off	
Distance to adjacent ramp	3765	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
	Ramp			
Volume, V (vph)	2730	1210	550	vph
Peak-hour factor, PHF	0.90	0.90	0.90	

Peak 15-min volume, v15	758	336	153	v
Trucks and buses	0	0	0	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00 %	0.00 %	0.00 %	
Length	0.00 mi	0.00 mi	0.00 mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	1.000	1.000	1.000	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	3033	1344	611	pcph

---

Estimation of V12 Diverge Areas

---

L = (Equation 25-8 or 25-9)

EQ

P = 0.450 Using Equation 0

FD

$v = v + (v - v) P = 2104$  pc/h

12 R F R FD

---

Capacity Checks

---

	Actual	Maximum	LOS F?
$v = v$	3033	6750	No
Fi F			
$v = v - v$	1689	6750	No
FO F R			
v	1344	3800	No
R			
$v = v$	929 pc/h	(Equation 25-15 or 25-16)	
3 or av34			
Is $v = v > 2700$ pc/h?		No	
3 or av34			
Is $v = v > 1.5 v / 2$		No	
3 or av34 12			
If yes, $v = 2104$		(Equation 25-18)	
12A			

---

Flow Entering Diverge Influence Area

---

	Actual	Max Desirable	Violation?
v	2104	4400	No
12			

---

Level of Service Determination (if not F)

---

Density,  $D = 4.252 + 0.0086 v - 0.009 L = 8.8$  pc/mi/ln

R 12 D

Level of service for ramp-freeway junction areas of influence A

---

Speed Estimation

---

Intermediate speed variable,  $D = 0.549$

S

Space mean speed in ramp influence area,  $S = 47.9$  mph



Space mean speed in outer lanes,	$\frac{R}{0}$	S = 60.3	mph
Space mean speed for all vehicles,		S = 51.1	mph

---

Phone: Fax:  
E-mail:

Diverge Analysis

Analyst: CTR  
 Agency/Co.: HNTB  
 Date performed: 8/02/2010  
 Analysis time period: Build Service Road Concept  
 Freeway/Dir of Travel: SR 417 WB  
 Junction: Off Ramp to International Pkwy  
 Jurisdiction: Seminole County  
 Analysis Year: 2012  
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	3	
Free-flow speed on freeway	55.0	mph
Volume on freeway	1520 ✓	vph

Off Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	35.0	mph
Volume on ramp	550 ✓	vph
Length of first accel/decel lane	0	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes ✓	
Volume on adjacent ramp	520 ✓	vph
Position of adjacent ramp	Downstream	
Type of adjacent ramp	On	
Distance to adjacent ramp	2076	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	1520 ✓	550	520	vph
Peak-hour factor, PHF	0.92 ✓	0.92	0.92	
Peak 15-min volume, v15	413 ✓	149	141	v
Trucks and buses	11 ✓	11	9	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00 %	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.948	0.948	0.957	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	1743	631	591	pcph

Estimation of V12 Diverge Areas

$$L = \text{EQ} \quad (\text{Equation 25-8 or 25-9})$$

$$P = 0.687 \quad \text{Using Equation 5}$$

$$v_{12} = v_R + (v_F - v_R) P = 1395 \quad \text{pc/h}$$

Capacity Checks

	Actual	Maximum	LOS F?
$v_{Fi} = v_F$	1743	6750	No
$v_{FO} = v_F - v_R$	1112	6750	No
$v_R$	631	2000	No
$v_{3 \text{ or } 34}$	348 pc/h	(Equation 25-15 or 25-16)	
Is $v_{3 \text{ or } 34} > 2700$ pc/h?		No	
Is $v_{3 \text{ or } 34} > 1.5 v_{12} / 2$		No	
If yes, $v_{12A} = 1395$		(Equation 25-18)	

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
$v_{12}$	1395	4400	No

Level of Service Determination (if not F)

Density,  $D = 4.252 + 0.0086 v_{12} - 0.009 L_D = 16.2$  pc/mi/ln

Level of service for ramp-freeway junction areas of influence B ✓

Speed Estimation

Intermediate speed variable,	$D = 0.485$	
Space mean speed in ramp influence area,	$S = 48.7$	mph
Space mean speed in outer lanes,	$S = 60.3$	mph
Space mean speed for all vehicles,	$S = 50.6$	mph

Phone:  
E-mail:

Fax:

Diverge Analysis

Analyst: CTR  
 Agency/Co.: HNTB  
 Date performed: 08/02/2010  
 Analysis time period: Build Service Road Concept  
 Freeway/Dir of Travel: SR 417 WB  
 Junction: Off Ramp to International Pkwy  
 Jurisdiction: Seminole County  
 Analysis Year: 2022  
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	3	
Free-flow speed on freeway	55.0	mph
Volume on freeway	1520 ✓	vph

Off Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	35.0 ✓	mph
Volume on ramp	550 ✓	vph
Length of first accel/decel lane	0	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes ✓	
Volume on adjacent ramp	1210 ✓	vph
Position of adjacent ramp	Upstream	
Type of adjacent ramp	Off	
Distance to adjacent ramp	3765	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	1520 ✓	550	1210	vph
Peak-hour factor, PHF	0.92 ✓	0.92	0.92	
Peak 15-min volume, v15	413 ✓	149	329	v
Trucks and buses	11 ✓	11	11	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00 %	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.948	0.948	0.948	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	1743	631	1388	pcph

---

Estimation of V12 Diverge Areas

---

$$L = \frac{EQ}{P} \quad (\text{Equation 25-8 or 25-9})$$

$$P = 0.687 \quad \text{Using Equation 5}$$

$$v_{12R} = v_{FR} + (v_{FR} - v_{FD}) P = 1395 \quad \text{pc/h}$$

---

Capacity Checks

---

	Actual	Maximum	LOS F?
$v_{Fi} = v_F$	1743	6750	No
$v_{FO} = v_F - v_R$	1112	6750	No
$v_R$	631	2000	No
$v_{3 \text{ or } av34}$	348 pc/h	(Equation 25-15 or 25-16)	
Is $v_{3 \text{ or } av34} > 2700$ pc/h?		No	
Is $v_{3 \text{ or } av34} > 1.5 v_{12} / 2$		No	
If yes, $v_{12A} = 1395$		(Equation 25-18)	

---

Flow Entering Diverge Influence Area

---

	Actual	Max Desirable	Violation?
$v_{12}$	1395	4400	No

---

Level of Service Determination (if not F)

---

Density,  $D = 4.252 + 0.0086 v_{12} - 0.009 L_D = 16.2$  pc/mi/ln

Level of service for ramp-freeway junction areas of influence B ✓

---

Speed Estimation

---

Intermediate speed variable,	$D = 0.485$	
Space mean speed in ramp influence area,	$S = 48.7$	mph
Space mean speed in outer lanes,	$S = 60.3$	mph
Space mean speed for all vehicles,	$S = 50.6$	mph

---

HCS+: Ramps and Ramp Junctions Release 5.4

Phone: Fax:  
E-mail:

Merge Analysis

Analyst: CTRR  
 Agency/Co.: HNTB  
 Date performed: 03/07/2010  
 Analysis time period: Build Service Road Concept  
 Freeway/Dir of Travel: SR 417 WB  
 Junction: On Ramp from I-4 EB & WB  
 Jurisdiction: Seminole County  
 Analysis Year: 2012  
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Merge		
Number of lanes in freeway	3		
Free-flow speed on freeway	55.0	mph	
Volume on freeway	970	vph	

On Ramp Data

Side of freeway	Right		
Number of lanes in ramp	1		
Free-flow speed on ramp	35.0	mph	
Volume on ramp	520	vph	
Length of first accel/decel lane	1325	ft	
Length of second accel/decel lane		ft	

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes		
Volume on adjacent Ramp	550	vph	
Position of adjacent Ramp	Upstream		
Type of adjacent Ramp	Off		
Distance to adjacent Ramp	2076	ft	

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	970	520	550	vph
Peak-hour factor, PHF	0.90	0.90	0.90	

Peak 15-min volume, v <sub>15</sub>	269	144	153	v
Trucks and buses	0	0	0	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, f <sub>HV</sub>	1.000	1.000	1.000	
Driver population factor, f <sub>P</sub>	1.00	1.00	1.00	
Flow rate, v <sub>p</sub>	1078	578	611	pcph

---

Estimation of V<sub>12</sub> Merge Areas

---

L = 370.88 (Equation 25-2 or 25-3)

EQ

P = 0.615 Using Equation 1

FM

v = v (P ) = 663 pc/h

12 F FM

---

Capacity Checks

---

	Actual	Maximum	LOS F?
v	1656	6750	No
FO			
v v	415 pc/h	(Equation 25-4 or 25-5)	
3 or av <sub>34</sub>			
Is v v > 2700 pc/h?		No	
3 or av <sub>34</sub>			
Is v v > 1.5 v /2		No	
3 or av <sub>34</sub> 12			
If yes, v = 663		(Equation 25-8)	
12A			

---

Flow Entering Merge Influence Area

---

	Actual	Max Desirable	Violation?
v	663	4600	No
R12			

---

Level of Service Determination (if not F)

---

Density, D = 5.475 + 0.00734 v<sub>R</sub> + 0.0078 v<sub>12</sub> - 0.00627 L<sub>A</sub> = 6.6 pc/mi/ln

Level of service for ramp-freeway junction areas of influence A

---

Speed Estimation

---

Intermediate speed variable, M = 0.242

S

Space mean speed in ramp influence area, S = 51.9 mph

R

Space mean speed in outer lanes, S = 55.0 mph

0

Space mean speed for all vehicles, S = 52.6 mph

HCS+: Ramps and Ramp Junctions Release 5.4

Phone: Fax:  
E-mail:

Merge Analysis

Analyst: CTRR  
Agency/Co.: HNTB  
Date performed: 03/07/2010  
Analysis time period: Build Service Road Concept  
Freeway/Dir of Travel: SR 417 WB  
Junction: On Ramp from I-4 EB & WB  
Jurisdiction: Seminole County  
Analysis Year: 2012  
Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	3	
Free-flow speed on freeway	55.0	mph
Volume on freeway	970	vph

On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	35.0	mph
Volume on ramp	520	vph
Length of first accel/decel lane	1325	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent Ramp	320	vph
Position of adjacent Ramp	Downstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1325	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	970	520	320
Peak-hour factor, PHF	0.90	0.90	0.90



Peak 15-min volume, v <sub>15</sub>	269	144	89	v
Trucks and buses	0	0	0	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, f <sub>HV</sub>	1.000	1.000	1.000	
Driver population factor, f <sub>P</sub>	1.00	1.00	1.00	
Flow rate, v <sub>p</sub>	1078	578	356	pcph

Estimation of V12 Merge Areas

---

L = (Equation 25-2 or 25-3)

EQ

P = 0.615 Using Equation 1

FM

v = v (P) = 663 pc/h

12 F FM

Capacity Checks

---

	Actual	Maximum	LOS F?
v	1656	6750	No
FO			
v v	415 pc/h	(Equation 25-4 or 25-5)	
3 or av <sub>34</sub>			
Is v v > 2700 pc/h?		No	
3 or av <sub>34</sub>			
Is v v > 1.5 v /2		No	
3 or av <sub>34</sub> 12			
If yes, v = 663		(Equation 25-8)	
12A			

Flow Entering Merge Influence Area

---

	Actual	Max Desirable	Violation?
v	663	4600	No
R12			

Level of Service Determination (if not F)

---

Density, D = 5.475 + 0.00734 v<sub>R</sub> + 0.0078 v<sub>12</sub> - 0.00627 L<sub>A</sub> = 6.6 pc/mi/ln

Level of service for ramp-freeway junction areas of influence A

Speed Estimation

---

Intermediate speed variable,	M = 0.242
S	
Space mean speed in ramp influence area,	S = 51.9 mph
R	
Space mean speed in outer lanes,	S = 55.0 mph
0	
Space mean speed for all vehicles,	S = 52.6 mph

SR 417 WB Off Ramp to I-4 EB & WB.txt  
HCS+: Ramps and Ramp Junctions Release 5.4

Phone: \_\_\_\_\_ Fax: \_\_\_\_\_  
E-mail: \_\_\_\_\_

\_\_\_\_\_Diverge Analysis\_\_\_\_\_

Analyst: CTR  
Agency/Co.: HNTB  
Date performed: 08/03/2010  
Analysis time period: Build Service Road Concept  
Freeway/Dir of Travel: SR 417 WB  
Junction: Off Ramp to I-4 EB/WB  
Jurisdiction: Seminole County  
Analysis Year: 2012  
Description: Wekiva Parkway Project Development & Environment Study

\_\_\_\_\_Freeway Data\_\_\_\_\_

Type of analysis	Diverge	
Number of lanes in freeway	4	
Free-flow speed on freeway	55.0	mph
Volume on freeway	2730	vph

\_\_\_\_\_Off Ramp Data\_\_\_\_\_

Side of freeway	Right	
Number of lanes in ramp	2	
Free-Flow speed on ramp	35.0	mph
Volume on ramp	1210	vph
Length of first accel/decel lane	0	ft
Length of second accel/decel lane	1500	ft

\_\_\_\_\_Adjacent Ramp Data (if one exists)\_\_\_\_\_

Does adjacent ramp exist?	Yes	
Volume on adjacent ramp	550	vph
Position of adjacent ramp	Downstream	
Type of adjacent ramp	Off	
Distance to adjacent ramp	3765	ft

\_\_\_\_\_Conversion to pc/h Under Base Conditions\_\_\_\_\_

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	2730	1210	550	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	742	329	149	v
Trucks and buses	10	10	10	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00 %	0.00 %	0.00 %	
Length	0.00 mi	0.00 mi	0.00 mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.952	0.952	0.952	
Driver population factor, fP	1.00	1.00	1.00	

Flow rate, vp SR 417 WB Off Ramp to I-4 EB & WB.txt 3116 1381 628 pcph

Estimation of V12 Diverge Areas

$$L = \text{EQ} \quad (\text{Equation 25-8 or 25-9})$$

$$P = 0.260 \quad \text{Using Equation 0}$$

$$V_{12} = V_R + (V_F - V_R) P = 1832 \quad \text{pc/h}$$

Capacity Checks

	Actual	Maximum	LOS F?
$V_{Fi} = V_F$	3116	9000	No
$V_{FO} = V_F - V_R$	1735	9000	No
$V_R$	1381	3800	No
$V_{3 \text{ or } av34}$	642 pc/h	(Equation 25-15 or 25-16)	
Is $V_{3 \text{ or } av34} > 2700$ pc/h?		No	
Is $V_{3 \text{ or } av34} > 1.5 V_{12} / 2$		No	
If yes, $V_{12A} = 1832$		(Equation 25-18)	

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
$V_{12}$	1832	4400	No

Level of Service Determination (if not F)

Density,  $D = 4.252 + 0.0086 V_{12} - 0.009 L_D = 6.5$  pc/mi/ln  
 Level of service for ramp-freeway junction areas of influence A

Speed Estimation

Intermediate speed variable,	$D = 0.552$	
Space mean speed in ramp influence area,	$S_R = 47.8$	mph
Space mean speed in outer lanes,	$S_O = 60.3$	mph
Space mean speed for all vehicles,	$S = 52.3$	mph

Phone: Fax:  
E-mail:

Merge Analysis

Analyst: CTR  
 Agency/Co.: HNTB  
 Date performed: 08/03/2010  
 Analysis time period: Build Service Road Concept  
 Freeway/Dir of Travel: SR 417 WB  
 Junction: On Ramp from International Pwy  
 Jurisdiction: Seminole County  
 Analysis Year: 2012  
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	4	
Free-flow speed on freeway	55.0	mph
Volume on freeway	1490 ✓	vph

On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	35.0	mph
Volume on ramp	320 ✓	vph
Length of first accel/decel lane	900 ✓	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes ✓	
Volume on adjacent Ramp	520 ✓	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1325	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	1490 ✓	320	520	vph
Peak-hour factor, PHF	0.92 ✓	0.92	0.92	
Peak 15-min volume, v15	405 ✓	87	141	v
Trucks and buses	11 ✓	9	9	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	%
Length	mi	mi	mi	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.948	0.957	0.957	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	1709	363	591	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)  
EQ  
P = 0.459 Using Equation 4  
FM  
 $v_{12} = v_F (P_{FM}) = 785 \text{ pc/h}$

Capacity Checks

	Actual	Maximum	LOS F?
v <sub>FO</sub>	2072	9000	No
v <sub>3 or av34</sub>	462 pc/h	(Equation 25-4 or 25-5)	
Is v <sub>3 or av34</sub> > 2700 pc/h?		No	
Is v <sub>3 or av34</sub> > 1.5 v <sub>12</sub> / 2		No	
If yes, v <sub>12A</sub> = 785		(Equation 25-8)	

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v <sub>R12</sub>	785	4600	No

Level of Service Determination (if not F)

Density,  $D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 8.6 \text{ pc/mi/ln}$   
Level of service for ramp-freeway junction areas of influence **A**

Speed Estimation

Intermediate speed variable,	M = 0.270	
Space mean speed in ramp influence area,	S = 51.5	mph
Space mean speed in outer lanes,	S = 55.0	mph
Space mean speed for all vehicles,	S = 53.0	mph

Phone:  
E-mail:

Fax:

Diverge Analysis

Analyst: CTR  
 Agency/Co.: HNTB  
 Date performed: 8/02/2010  
 Analysis time period: Build Service Road Concept  
 Freeway/Dir of Travel: SR 417 EB  
 Junction: Off Ramp to International Pkwy  
 Jurisdiction: Seminole County  
 Analysis Year: 2012  
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	3	
Free-flow speed on freeway	55.0	mph
Volume on freeway	1810 ✓	vph

Off Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	35.0	mph
Volume on ramp	320 ✓	vph
Length of first accel/decel lane	0	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent ramp	520	vph
Position of adjacent ramp	Downstream	
Type of adjacent ramp	Off	
Distance to adjacent ramp	1250	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	1810 ✓	320	520	vph
Peak-hour factor, PHF	0.92 ✓	0.92	0.92	
Peak 15-min volume, v15	492 ✓	87	141	v
Trucks and buses	11 ✓	11	11	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00 %	0.00 %	0.00 %	
Length	0.00 mi	0.00 mi	0.00 mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.948	0.948	0.948	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	2076	367	596	pcph

Estimation of V12 Diverge Areas

$$L = 628.75 \text{ (Equation 25-8 or 25-9)}$$

$$EQ$$

$$P = 0.691 \text{ Using Equation 5}$$

$$FD$$

$$v_{12R} = v_F + (v_F - v_R) P = 1548 \text{ pc/h}$$

Capacity Checks

	Actual	Maximum	LOS F?
$v_{Fi} = v_F$	2076	6750	No
$v_{FO} = v_F - v_R$	1709	6750	No
$v_R$	367	2000	No
$v_{3 \text{ or } av34}$	528 pc/h	(Equation 25-15 or 25-16)	
Is $v_{3 \text{ or } av34} > 2700 \text{ pc/h?}$		No	
Is $v_{3 \text{ or } av34} > 1.5 v_{12} / 2$		No	
If yes, $v_{12A} = 1548$		(Equation 25-18)	

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
$v_{12}$	1548	4400	No

Level of Service Determination (if not F)

Density,  $D = 4.252 + 0.0086 v_{12} - 0.009 L_D = 17.6 \text{ pc/mi/ln}$

Level of service for ramp-freeway junction areas of influence B ✓

Speed Estimation

Intermediate speed variable,	$D = 0.461$	
Space mean speed in ramp influence area,	$S = 49.0$	mph
Space mean speed in outer lanes,	$S = 60.3$	mph
Space mean speed for all vehicles,	$S = 51.5$	mph

Phone:  
E-mail:

Fax:

Diverge Analysis

Analyst: CTR  
 Agency/Co.: HNTB  
 Date performed: 8/02/2010  
 Analysis time period: Build Service Road Concept  
 Freeway/Dir of Travel: SR 417 EB  
 Junction: Off Ramp to I-4 EB & WB  
 Jurisdiction: Seminole County  
 Analysis Year: 2012  
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	3	
Free-flow speed on freeway	55.0	mph
Volume on freeway	1490 ✓	vph

Off Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	35.0	mph
Volume on ramp	520 ✓	vph
Length of first accel/decel lane	0	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes ✓	
Volume on adjacent ramp	320 ✓	vph
Position of adjacent ramp	Upstream	
Type of adjacent ramp	Off	
Distance to adjacent ramp	1250	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	1490 ✓	520	320	vph
Peak-hour factor, PHF	0.92 ✓	0.92	0.92	
Peak 15-min volume, v15	405 ✓	141	87	v
Trucks and buses	11 ✓	11	11	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00 %	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	



Heavy vehicle adjustment, fHV	0.948	0.948	0.948	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	1709	596	367	pcph

Estimation of V12 Diverge Areas

---

$$L = \frac{EQ}{P} \quad (\text{Equation 25-8 or 25-9})$$

$$P = 0.690 \quad \text{Using Equation 5}$$

$$v_{12R} = v_{FR} + (v_{FR} - v_{FD}) P = 1364 \quad \text{pc/h}$$

Capacity Checks

---

	Actual	Maximum	LOS F?
$v_{Fi} = v_F$	1709	6750	No
$v_{FO} = v_F - v_R$	1113	6750	No
$v_R$	596	2000	No
$v_{3 \text{ or } av34}$	345 pc/h	(Equation 25-15 or 25-16)	
Is $v_{3 \text{ or } av34} > 2700$ pc/h?		No	
Is $v_{3 \text{ or } av34} > 1.5 v_{12} / 2$		No	
If yes, $v_{12A} = 1364$		(Equation 25-18)	

Flow Entering Diverge Influence Area

---

	Actual	Max Desirable	Violation?
$v_{12}$	1364	4400	No

Level of Service Determination (if not F)

---

Density,  $D = 4.252 + 0.0086 v_{12} - 0.009 L_D = 16.0$  pc/mi/ln

Level of service for ramp-freeway junction areas of influence B

Speed Estimation

---

Intermediate speed variable,	$D = 0.482$	
Space mean speed in ramp influence area,	$S = 48.7$	mph
Space mean speed in outer lanes,	$S = 60.3$	mph
Space mean speed for all vehicles,	$S = 50.7$	mph

---

Phone:  
E-mail:

Fax:

Diverge Analysis

Analyst: CTR  
 Agency/Co.: HNTB  
 Date performed: 8/02/2010  
 Analysis time period: Build Service Road Concept  
 Freeway/Dir of Travel: SR 417 EB  
 Junction: Off Ramp to I-4 EB & WB  
 Jurisdiction: Seminole County  
 Analysis Year: 2012  
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	3	
Free-flow speed on freeway	55.0	mph
Volume on freeway	1490 ✓	vph

Off Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	35.0 ✓	mph
Volume on ramp	520 ✓	vph
Length of first accel/decel lane	0	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes ✓	
Volume on adjacent ramp	540 ✓	vph
Position of adjacent ramp	Downstream	
Type of adjacent ramp	On	
Distance to adjacent ramp	1964	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	1490 ✓	520	540	vph
Peak-hour factor, PHF	0.92 ✓	0.92	0.92	
Peak 15-min volume, v15	405 ✓	141	147	v
Trucks and buses	11 ✓	11	11	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00 %	0.00 %	0.00 %	
Length	0.00 mi	0.00 mi	0.00 mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.948	0.948	0.948	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	1709	596	619	pcph

Estimation of V12 Diverge Areas

---

$$L = \text{(Equation 25-8 or 25-9)}$$

$$EQ$$

$$P = 0.690 \text{ Using Equation 5}$$

$$FD$$

$$v_{12R} = v_F + (v_F - v_R) P = 1364 \text{ pc/h}$$

Capacity Checks

---

	Actual	Maximum	LOS F?
$v_{Fi} = v_F$	1709	6750	No
$v_{FO} = v_F - v_R$	1113	6750	No
$v_R$	596	2000	No
$v_{3 \text{ or } av34}$	345 pc/h	(Equation 25-15 or 25-16)	
Is $v_{3 \text{ or } av34} > 2700 \text{ pc/h?}$		No	
Is $v_{3 \text{ or } av34} > 1.5 v_{12} / 2$		No	
If yes, $v_{12A} = 1364$		(Equation 25-18)	

Flow Entering Diverge Influence Area

---

	Actual	Max Desirable	Violation?
$v_{12}$	1364	4400	No

Level of Service Determination (if not F)

---

Density,  $D = 4.252 + 0.0086 v_{12} - 0.009 L_D = 16.0 \text{ pc/mi/ln}$

Level of service for ramp-freeway junction areas of influence B ✓

Speed Estimation

---

Intermediate speed variable,	$D = 0.482$	
Space mean speed in ramp influence area,	$S = 48.7$	mph
Space mean speed in outer lanes,	$S = 60.3$	mph
Space mean speed for all vehicles,	$S = 50.7$	mph

---

Phone: Fax:  
E-mail:

----- Merge Analysis -----

Analyst: CTR  
 Agency/Co.: HNTB  
 Date performed: 8/02/2010  
 Analysis time period: Build Service Road Concept  
 Freeway/Dir of Travel: SR 417 EB  
 Junction: On Ramp from International Pky  
 Jurisdiction: Seminole County  
 Analysis Year: 2012  
 Description: Wekiva Parkway Project Development & Environment Project

----- Freeway Data -----

Type of analysis	Merge	
Number of lanes in freeway	3	
Free-flow speed on freeway	55.0	mph
Volume on freeway	970	vph

----- On Ramp Data -----

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	35.0	mph
Volume on ramp	540	vph
Length of first accel/decel lane	1200	ft
Length of second accel/decel lane		ft

----- Adjacent Ramp Data (if one exists) -----

Does adjacent ramp exist?	Yes	
Volume on adjacent Ramp	520	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	Off	
Distance to adjacent Ramp	1964	ft

----- Conversion to pc/h Under Base Conditions -----

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	970	540	520	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	264	147	141	v
Trucks and buses	11	10	10	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade		%	%	%
Length		mi	mi	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.948	0.952	0.952	
Driver population factor, fP	1.00	1.00	1.00	



Phone:  
E-mail:

Fax:

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Merge Analysis

---

Analyst: CTR  
 Agency/Co.: HNTB  
 Date performed: 8/02/2010  
 Analysis time period: Build Service Road Concept  
 Freeway/Dir of Travel: SR 417 EB  
 Junction: On Ramp from International Pky  
 Jurisdiction: Seminole County  
 Analysis Year: 2012  
 Description: Wekiva Parkway Project Development & Environment Project

---

Freeway Data

---

Type of analysis	Merge		
Number of lanes in freeway	3		
Free-flow speed on freeway	55.0	mph	
Volume on freeway	970	vph	

---

On Ramp Data

---

Side of freeway	Right		
Number of lanes in ramp	1		
Free-flow speed on ramp	35.0	mph	
Volume on ramp	540	vph	
Length of first accel/decel lane	1200	ft	
Length of second accel/decel lane		ft	

---

Adjacent Ramp Data (if one exists)

---

Does adjacent ramp exist?	Yes		
Volume on adjacent Ramp	1260	vph	
Position of adjacent Ramp	Downstream		
Type of adjacent Ramp	On		
Distance to adjacent Ramp	3106	ft	

---

Conversion to pc/h Under Base Conditions

---

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	970	540	1260	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	264	147	342	v
Trucks and buses	11	10	9	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.948	0.952	0.957	
Driver population factor, fP	1.00	1.00	1.00	

SR 417 EB ON Ramp From Int'l Pkwy\_Downstream Analysis.txt  
 Flow rate,  $v_p$     1112    616    1431    pcph

Estimation of V12 Merge Areas

---

$$L = \text{(Equation 25-2 or 25-3)}$$

$$P = 0.611 \text{ Using Equation 1}$$

$$v_{12} = v_F (P_{FM}) = 680 \text{ pc/h}$$

Capacity Checks

---

	Actual 1728	Maximum 6750	LOS F? No
$v_{FO}$			
$v_{3 \text{ or } av34}$	432 pc/h	(Equation 25-4 or 25-5)	
Is $v_{3 \text{ or } av34} > 2700$ pc/h?		No	
Is $v_{3 \text{ or } av34} > 1.5 v_{12} / 2$		No	
If yes, $v_{12A} = 680$		(Equation 25-8)	

Flow Entering Merge Influence Area

---

	Actual 680	Max Desirable 4600	Violation? No
$v_{R12}$			

Level of Service Determination (if not F)

---

Density,  $D_R = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 7.8 \text{ pc/mi/ln}$   
 Level of service for ramp-freeway junction areas of influence A

Speed Estimation

---

Intermediate speed variable,	$M = 0.251$
Space mean speed in ramp influence area,	$S_S = 51.7 \text{ mph}$
Space mean speed in outer lanes,	$S_R = 55.0 \text{ mph}$
Space mean speed for all vehicles,	$S_0 = 52.5 \text{ mph}$

---

Phone: Fax:  
E-mail:

Merge Analysis

Analyst: CTR  
 Agency/Co.: HNTB  
 Date performed: 8/02/2010  
 Analysis time period: Build Service Road Concept  
 Freeway/Dir of Travel: SR 417 EB  
 Junction: On Ramp from I-4 EB & WB  
 Jurisdiction: Seminole County  
 Analysis Year: 2012  
 Description: Wekiva Parkway Project Development & Environment Project

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	3	
Free-flow speed on freeway	55.0	mph
Volume on freeway	1510 ✓	vph

On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	2	
Free-flow speed on ramp	35.0 ✓	mph
Volume on ramp	1260 ✓	vph
Length of first accel/decel lane	900	ft
Length of second accel/decel lane	1500	ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes ✓	
Volume on adjacent Ramp	540 ✓	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	3106	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	1510 ✓	1260	540	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	410 ✓	342	147	v
Trucks and buses	11 ✓	9	11	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	



Heavy vehicle adjustment, fHV	0.948	0.957	0.948	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	1732	1431	619	pcph

---

Estimation of V12 Merge Areas

---

L = (Equation 25-2 or 25-3)  
EQ  
P = 0.555 Using Equation 0  
FM  
 $v_{12} = v_F (P_{FM}) = 961 \text{ pc/h}$

---

Capacity Checks

---

	v	Actual	Maximum	LOS F?
	FO	3163	6750	No
	v <sub>3</sub> or v <sub>av34</sub>	771 pc/h	(Equation 25-4 or 25-5)	
Is	v <sub>3</sub> or v <sub>av34</sub>	> 2700 pc/h?	No	
Is	v <sub>3</sub> or v <sub>av34</sub>	> 1.5 v <sub>12</sub> / 2	Yes	
If yes,	v <sub>12A</sub>	= 989	(Equation 25-8)	

---

Flow Entering Merge Influence Area

---

	v	Actual	Max Desirable	Violation?
	12A	989	4600	No

---

Level of Service Determination (if not F)

---

Density,  $D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 3.0 \text{ pc/mi/ln}$   
Level of service for ramp-freeway junction areas of influence A ✓

---

Speed Estimation

---

Intermediate speed variable,	M = 0.134	
Space mean speed in ramp influence area,	S = 53.3	mph
Space mean speed in outer lanes,	S = 54.1	mph
Space mean speed for all vehicles,	S = 53.5	mph

---

Phone: Fax:  
 E-mail:

Diverge Analysis

Analyst: CTR  
 Agency/Co.: HNTB  
 Date performed: 8/2/2010  
 Analysis time period: Build Service Road Concept  
 Freeway/Dir of Travel: I-4 EB  
 Junction: Off Ramp to CR 46A  
 Jurisdiction: Seminole County  
 Analysis Year: 2012  
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	4 ✓	
Free-flow speed on freeway	55.0	mph
Volume on freeway	5530 ✓	vph

Off Ramp Data

Side of freeway	Right	
Number of lanes in ramp	2 ✓	
Free-Flow speed on ramp	35.0	mph
Volume on ramp	1190 ✓	vph
Length of first accel/decel lane	0	ft
Length of second accel/decel lane	1500	ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent ramp	1390	vph
Position of adjacent ramp	Downstream	
Type of adjacent ramp	Off	
Distance to adjacent ramp	1906	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	5530 ✓	1190	1390	vph
Peak-hour factor, PHF	0.92 ✓	0.92	0.92	
Peak 15-min volume, v15	1503 ✓	323	378	v
Trucks and buses	9 ✓	9	9	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00 %	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5*	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.957	0.957	0.957	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	6281	1352	1579	pcph

Estimation of V12 Diverge Areas

$$L = \frac{EQ}{P} \quad \text{(Equation 25-8 or 25-9)}$$

$$P = 0.260 \quad \text{Using Equation 0}$$

$$v_{12R} = v_{12F} + (v_{12R} - v_{12F}) P_{FD} = 2634 \quad \text{pc/h}$$

Capacity Checks

	Actual	Maximum	LOS F?
$v_{12F}$	6281	9000	No
$v_{12R}$	4929	9000	No
$v_{12R}$	1352	3800	No
$v_{12R}$	1823 pc/h	(Equation 25-15 or 25-16)	
Is $v_{12R} > 2700$ pc/h?		No	
Is $v_{12R} > 1.5 v_{12F} / 2$		No	
If yes, $v_{12A} = 2634$		(Equation 25-18)	

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
$v_{12}$	2634	4400	No

Level of Service Determination (if not F)

Density,  $D = 4.252 + 0.0086 v_{12R} - 0.009 L_D = 13.4 \quad \text{pc/mi/ln}$

Level of service for ramp-freeway junction areas of influence B ✓

Speed Estimation

Intermediate speed variable,	$D = 0.550$	
Space mean speed in ramp influence area,	$S_R = 47.9$	mph
Space mean speed in outer lanes,	$S_O = 57.1$	mph
Space mean speed for all vehicles,	$S = 52.8$	mph

HCS+: Ramps and Ramp Junctions Release 5.4

Phone: Fax:  
E-mail:

Diverge Analysis

Analyst: CTRR  
 Agency/Co.: HNTB  
 Date performed: 3/07/2010  
 Analysis time period: Build Service Road Concept  
 Freeway/Dir of Travel: I-4 EB  
 Junction: Off Ramp to SR 417 & SR 46  
 Jurisdiction: Seminole County  
 Analysis Year: 2012  
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	3	
Free-flow speed on freeway	55.0	mph
Volume on freeway	4340	vph

Off Ramp Data

Side of freeway	Right	
Number of lanes in ramp	2	
Free-Flow speed on ramp	35.0	mph
Volume on ramp	1390	vph
Length of first accel/decel lane	406	ft
Length of second accel/decel lane	1500	ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent ramp	1190	vph
Position of adjacent ramp	Upstream	
Type of adjacent ramp	Off	
Distance to adjacent ramp	1906	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	4340	1390	1190	vph
Peak-hour factor, PHF	0.90	0.90	0.90	

Peak 15-min volume, v <sub>15</sub>	1206	386	331	v
Trucks and buses	0	0	0	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00 %	0.00 %	0.00 %	
Length	0.00 mi	0.00 mi	0.00 mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, f <sub>HV</sub>	1.000	1.000	1.000	
Driver population factor, f <sub>P</sub>	1.00	1.00	1.00	
Flow rate, v <sub>p</sub>	4822	1544	1322	pcph

Estimation of V12 Diverge Areas

---

L = (Equation 25-8 or 25-9)

EQ

P = 0.450 Using Equation 0

FD

v = v + (v - v) P = 3019 pc/h

12 R F R FD

Capacity Checks

---

	Actual	Maximum	LOS F?
v = v	4822	6750	No
F <sub>i</sub> F			
v = v - v	3278	6750	No
F <sub>O</sub> F R			
v	1544	3800	No
R			
v v	1803 pc/h	(Equation 25-15 or 25-16)	
3 or av <sub>34</sub>			
Is v v > 2700 pc/h?		No	
3 or av <sub>34</sub>			
Is v v > 1.5 v /2		No	
3 or av <sub>34</sub> 12			
If yes, v = 3019		(Equation 25-18)	
12A			

Flow Entering Diverge Influence Area

---

	Actual	Max Desirable	Violation?
v	3019	4400	No
12			

Level of Service Determination (if not F)

---

Density, D = 4.252 + 0.0086 v - 0.009 L = 9.4 pc/mi/ln  
R 12 D

Level of service for ramp-freeway junction areas of influence A

Speed Estimation

---

Intermediate speed variable, D = 0.567  
S

Space mean speed in ramp influence area, S = 47.6 mph

Space mean speed in outer lanes,	R	S = 57.2	mph
	0		
Space mean speed for all vehicles,		S = 50.8	mph

---

HCS+: Ramps and Ramp Junctions Release 5.4

Phone: Fax:  
E-mail:

Diverge Analysis

Analyst: CTRR  
 Agency/Co.: HNTB  
 Date performed: 3/07/2010  
 Analysis time period: Build Service Road Concept  
 Freeway/Dir of Travel: I-4 EB  
 Junction: Off Ramp to SR 417 & SR 46  
 Jurisdiction: Seminole County  
 Analysis Year: 2012  
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Diverge		
Number of lanes in freeway	3		
Free-flow speed on freeway	55.0	mph	
Volume on freeway	4340	vph	

Off Ramp Data

Side of freeway	Right		
Number of lanes in ramp	2		
Free-Flow speed on ramp	35.0	mph	
Volume on ramp	1390	vph	
Length of first accel/decel lane	406	ft	
Length of second accel/decel lane	1500	ft	

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes		
Volume on adjacent ramp	410	vph	
Position of adjacent ramp	Downstream		
Type of adjacent ramp	On		
Distance to adjacent ramp	1410	ft	

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
		Ramp		
Volume, V (vph)	4340	1390	410	vph
Peak-hour factor, PHF	0.90	0.90	0.90	

Peak 15-min volume, v <sub>15</sub>	1206	386	114	v
Trucks and buses	0	0	0	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00 %	0.00 %	0.00 %	
Length	0.00 mi	0.00 mi	0.00 mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, f <sub>HV</sub>	1.000	1.000	1.000	
Driver population factor, f <sub>P</sub>	1.00	1.00	1.00	
Flow rate, v <sub>p</sub>	4822	1544	456	pcph

---

Estimation of V12 Diverge Areas

---

L = (Equation 25-8 or 25-9)

EQ

P = 0.450 Using Equation 0

FD

$v = v + (v - v) P = 3019$  pc/h

12 R F R FD

---

Capacity Checks

---

	Actual	Maximum	LOS F?
v = v	4822	6750	No
F <sub>i</sub> F			
v = v - v	3278	6750	No
F <sub>O</sub> F R			
v	1544	3800	No
R			
v v	1803 pc/h	(Equation 25-15 or 25-16)	
3 or av <sub>34</sub>			
Is v v > 2700 pc/h?		No	
3 or av <sub>34</sub>			
Is v v > 1.5 v / 2		No	
3 or av <sub>34</sub> 12			
If yes, v = 3019		(Equation 25-18)	
12A			

---

Flow Entering Diverge Influence Area

---

	Actual	Max Desirable	Violation?
v	3019	4400	No
12			

---

Level of Service Determination (if not F)

---

Density,  $D = 4.252 + 0.0086 v - 0.009 L = 9.4$  pc/mi/ln

R 12 D

Level of service for ramp-freeway junction areas of influence A

---

Speed Estimation

---

Intermediate speed variable,  $D = 0.567$

S

Space mean speed in ramp influence area,  $S = 47.6$  mph



Space mean speed in outer lanes,	$\frac{R}{0}$	S = 57.2	mph
Space mean speed for all vehicles,		S = 50.8	mph

---

Phone: Fax:  
E-mail:

Merge Analysis

Analyst: CTR  
 Agency/Co.: HNTB  
 Date performed: 8/2/2010  
 Analysis time period: Build Service Road Concept  
 Freeway/Dir of Travel: I-4 EB  
 Junction: On Ramp from CR 46A  
 Jurisdiction: Seminole County  
 Analysis Year: 2012  
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	3	
Free-flow speed on freeway	55.0	mph
Volume on freeway	2950 ✓	vph

On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	35.0	mph
Volume on ramp	410 ✓	vph
Length of first accel/decel lane	700 ✓	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent Ramp	860 ✓	vph
Position of adjacent Ramp	Downstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	4826	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	2950 ✓	410	860	vph
Peak-hour factor, PHF	0.92 ✓	0.92	0.92	
Peak 15-min volume, v15	802 ✓	111	234	v
Trucks and buses	9 ✓	9	10	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade		%	%	%
Length		mi	mi	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.957	0.957	0.952	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	3351	466	982	pcph

---

Estimation of V12 Merge Areas

---

L = (Equation 25-2 or 25-3)  
EQ  
P = 0.597 Using Equation 1  
FM  
 $v_{12} = v_F (P_{FM}) = 2001 \text{ pc/h}$

---

Capacity Checks

---

	Actual	Maximum	LOS F?
v <sub>FO</sub>	3817	6750	No
v <sub>3 or av34</sub>	1350 pc/h	(Equation 25-4 or 25-5)	
Is v <sub>3 or av34</sub> > 2700 pc/h?		No	
Is v <sub>3 or av34</sub> > 1.5 v <sub>12</sub> / 2		No	
If yes, v <sub>12A</sub> = 2001		(Equation 25-8)	

---

Flow Entering Merge Influence Area

---

	Actual	Max Desirable	Violation?
v <sub>R12</sub>	2001	4600	No

---

Level of Service Determination (if not F)

---

Density,  $D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 20.1 \text{ pc/mi/ln}$   
Level of service for ramp-freeway junction areas of influence C ✓

---

Speed Estimation

---

Intermediate speed variable,	M = 0.318	
Space mean speed in ramp influence area,	S <sub>R</sub> = 50.9	mph
Space mean speed in outer lanes,	S <sub>0</sub> = 51.9	mph
Space mean speed for all vehicles,	S = 51.2	mph

---

Phone: Fax:  
E-mail:

Merge Analysis

Analyst: CTR  
 Agency/Co.: HNTB  
 Date performed: 8/2/2010  
 Analysis time period: Build Service Road Concept  
 Freeway/Dir of Travel: I-4 EB  
 Junction: On Ramp from CR 46A  
 Jurisdiction: Seminole County  
 Analysis Year: 2012  
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	3	
Free-flow speed on freeway	55.0	mph
Volume on freeway	2950 ✓	vph

On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	35.0 ✓	mph
Volume on ramp	410 ✓	vph
Length of first accel/decel lane	700	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent Ramp	1390 ✓	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	Off	
Distance to adjacent Ramp	1410	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	2950 ✓	410	1390	vph
Peak-hour factor, PHF	0.92 ✓	0.92	0.92	
Peak 15-min volume, v15	802 ✓	111	378	v
Trucks and buses	9 ✓	9	9	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.957	0.957	0.957	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	3351	466	1579	pcph

Estimation of V12 Merge Areas

L = 555.84 (Equation 25-2 or 25-3)  
EQ  
P = 0.597 Using Equation 1  
FM  
 $v_{12} = v_F (P_{FM}) = 2001 \text{ pc/h}$

Capacity Checks

v <sub>FO</sub>	Actual	Maximum	LOS F?
	3817	6750	No
v <sub>3 or av34</sub>	1350 pc/h	(Equation 25-4 or 25-5)	
Is v <sub>3 or av34</sub> > 2700 pc/h?		No	
Is v <sub>3 or av34</sub> > 1.5 v <sub>12</sub> / 2		No	
If yes, v <sub>12A</sub> = 2001		(Equation 25-8)	

Flow Entering Merge Influence Area

v <sub>R12</sub>	Actual	Max Desirable	Violation?
	2001	4600	No

Level of Service Determination (if not F)

Density,  $D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 20.1 \text{ pc/mi/ln}$   
Level of service for ramp-freeway junction areas of influence C ✓

Speed Estimation

Intermediate speed variable,	M = 0.318	
Space mean speed in ramp influence area,	S <sub>R</sub> = 50.9	mph
Space mean speed in outer lanes,	S <sub>O</sub> = 51.9	mph
Space mean speed for all vehicles,	S = 51.2	mph

Phone: \_\_\_\_\_ Fax: \_\_\_\_\_  
 E-mail: \_\_\_\_\_

Merge Analysis

Analyst: CTR  
 Agency/Co.: HNTB  
 Date performed: 8/2/2010  
 Analysis time period: Build Service Road Concept  
 Freeway/Dir of Travel: I-4 EB  
 Junction: On Ramp from SR 417 WB  
 Jurisdiction: Seminole County  
 Analysis Year: 2012  
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	3	
Free-flow speed on freeway	55.0	mph
Volume on freeway	3360 ✓	vph

On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	2	
Free-flow speed on ramp	35.0 ✓	mph
Volume on ramp	860 ✓	vph
Length of first accel/decel lane	900	ft
Length of second accel/decel lane	1500	ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes ✓	
Volume on adjacent Ramp	410 ✓	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	4826	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	3360 ✓	860	410	vph
Peak-hour factor, PHF	0.92 ✓	0.92	0.92	
Peak 15-min volume, v15	913 ✓	234	111	v
Trucks and buses	9 ✓	10	9	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	%
Length	mi	mi	mi	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.957	0.952	0.957	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	3817	982	466	pcph

Estimation of V12 Merge Areas

$$L = \text{(Equation 25-2 or 25-3)}$$

$$EQ$$

$$P = 0.555 \text{ Using Equation 0}$$

$$FM$$

$$v_{12} = v_F (P_{FM}) = 2118 \text{ pc/h}$$

Capacity Checks

	Actual	Maximum	LOS F?
v <sub>FO</sub>	4799	6750	No
v <sub>3 or av34</sub>	1699 pc/h	(Equation 25-4 or 25-5)	
Is v <sub>3 or av34</sub> > 2700 pc/h?		No	
Is v <sub>3 or av34</sub> > 1.5 v <sub>12</sub> / 2		Yes	
If yes, v <sub>12A</sub> = 2181		(Equation 25-8)	

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v <sub>12A</sub>	2181	4600	No

Level of Service Determination (if not F)

$$\text{Density, } D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 9.0 \text{ pc/mi/ln}$$

Level of service for ramp-freeway junction areas of influence A ✓

Speed Estimation

Intermediate speed variable,	M = 0.182	
Space mean speed in ramp influence area,	S <sub>R</sub> = 52.6	mph
Space mean speed in outer lanes,	S <sub>0</sub> = 50.9	mph
Space mean speed for all vehicles,	S = 52.0	mph

Phone: Fax:  
E-mail:

Merge Analysis

Analyst: CTR  
 Agency/Co.: HNTB  
 Date performed: 8/2/2010  
 Analysis time period: Build Service Road Concept  
 Freeway/Dir of Travel: I-4 EB  
 Junction: On Ramp from SR 417 WB  
 Jurisdiction: Seminole County  
 Analysis Year: 2012  
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	3	
Free-flow speed on freeway	55.0	mph
Volume on freeway	3360 ✓	vph

On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	2	
Free-flow speed on ramp	35.0 ✓	mph
Volume on ramp	860 ✓	vph
Length of first accel/decel lane	900	ft
Length of second accel/decel lane	1500	ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent Ramp	430 ✓	vph
Position of adjacent Ramp	Downstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1976	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	3360 ✓	860	430	vph
Peak-hour factor, PHF	0.92 ✓	0.92	0.92	
Peak 15-min volume, v15	913	234	117	v
Trucks and buses	9 ✓	10	9	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	%
Length	mi	mi	mi	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	



Heavy vehicle adjustment, fHV	0.957	0.952	0.957	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	3817	982	488	pcph

Estimation of V12 Merge Areas

$$L = \text{(Equation 25-2 or 25-3)}$$

$$EQ$$

$$P = 0.555 \text{ Using Equation 0}$$

$$FM$$

$$v_{12} = v_F (P_{FM}) = 2118 \text{ pc/h}$$

Capacity Checks

	Actual	Maximum	LOS F?
v <sub>FO</sub>	4799	6750	No
v <sub>3 or av34</sub>	1699 pc/h	(Equation 25-4 or 25-5)	
Is v <sub>3 or av34</sub> > 2700 pc/h?		No	
Is v <sub>3 or av34</sub> > 1.5 v <sub>12</sub> / 2		Yes	
If yes, v <sub>12A</sub> = 2181		(Equation 25-8)	

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v <sub>12A</sub>	2181	4600	No

Level of Service Determination (if not F)

$$\text{Density, } D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 9.0 \text{ pc/mi/ln}$$

Level of service for ramp-freeway junction areas of influence A ✓

Speed Estimation

Intermediate speed variable,	M = 0.182	
Space mean speed in ramp influence area,	S <sub>R</sub> = 52.6	mph
Space mean speed in outer lanes,	S <sub>0</sub> = 50.9	mph
Space mean speed for all vehicles,	S = 52.0	mph

Phone: \_\_\_\_\_ Fax: \_\_\_\_\_  
 E-mail: \_\_\_\_\_

Merge Analysis

Analyst: CTR  
 Agency/Co.: HNTB  
 Date performed: 8/2/2010  
 Analysis time period: Build Service Road Concept  
 Freeway/Dir of Travel: I-4 EB  
 Junction: On Ramp from SR 417 EB  
 Jurisdiction: Seminole County  
 Analysis Year: 2012  
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	3	
Free-flow speed on freeway	55.0	mph
Volume on freeway	4220 ✓	vph

On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	35.0	mph
Volume on ramp	430 ✓	vph
Length of first accel/decel lane	900	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes ✓	
Volume on adjacent Ramp	1000 ✓	vph
Position of adjacent Ramp	Downstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	3172	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	4220 ✓	430	1000	vph
Peak-hour factor, PHF	0.92 ✓	0.92	0.92	
Peak 15-min volume, v15	1147 ✓	117	272	v
Trucks and buses	9 ✓	9	9	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.957	0.957	0.957	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	4793	488	1136	pcph

Estimation of V12 Merge Areas

---

L = (Equation 25-2 or 25-3)  
EQ  
P = 0.603 Using Equation 1  
FM  
 $v_{12} = v_F (P_{FM}) = 2889 \text{ pc/h}$

Capacity Checks

---

v <sub>FO</sub>	Actual	Maximum	LOS F?
	5281	6750	No
v <sub>3 or av34</sub>	1904 pc/h	(Equation 25-4 or 25-5)	
Is v <sub>3 or av34</sub> > 2700 pc/h?		No	
Is v <sub>3 or av34</sub> > 1.5 v <sub>12</sub> / 2		No	
If yes, v <sub>12A</sub> = 2889		(Equation 25-8)	

Flow Entering Merge Influence Area

---

v <sub>R12</sub>	Actual	Max Desirable	Violation?
	2889	4600	No

Level of Service Determination (if not F)

---

Density,  $D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 25.9 \text{ pc/mi/ln}$   
Level of service for ramp-freeway junction areas of influence C ✓

Speed Estimation

---

Intermediate speed variable,	M = 0.372	
Space mean speed in ramp influence area,	S <sub>R</sub> = 50.2	mph
Space mean speed in outer lanes,	S <sub>0</sub> = 49.9	mph
Space mean speed for all vehicles,	S = 50.1	mph

---

Phone: \_\_\_\_\_ Fax: \_\_\_\_\_  
 E-mail: \_\_\_\_\_

Merge Analysis

Analyst: CTR  
 Agency/Co.: HNTB  
 Date performed: 8/2/2010  
 Analysis time period: Build Service Road Concept  
 Freeway/Dir of Travel: I-4 EB  
 Junction: On Ramp from SR 417 EB  
 Jurisdiction: Seminole County  
 Analysis Year: 2012  
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	3	
Free-flow speed on freeway	55.0	mph
Volume on freeway	4220 ✓	vph

On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	35.0 ✓	mph
Volume on ramp	430 ✓	vph
Length of first accel/decel lane	900	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent Ramp	860 ✓	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1976	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	4220 ✓	430	860	vph
Peak-hour factor, PHF	0.92 ✓	0.92	0.92	
Peak 15-min volume, v15	1147 ✓	117	234	v
Trucks and buses	9 ✓	9	9	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.957	0.957	0.957	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	4793	488	977	pcph

Estimation of V12 Merge Areas

$$L = \text{(Equation 25-2 or 25-3)}$$

$$EQ$$

$$P = 0.603 \text{ Using Equation 1}$$

$$FM$$

$$v_{12} = v_{F, FM} = 2889 \text{ pc/h}$$

Capacity Checks

	Actual	Maximum	LOS F?
v <sub>FO</sub>	5281	6750	No
v <sub>3 or av34</sub>	1904 pc/h	(Equation 25-4 or 25-5)	
Is v <sub>3 or av34</sub> > 2700 pc/h?		No	
Is v <sub>3 or av34</sub> > 1.5 v <sub>12</sub> / 2		No	
If yes, v <sub>12A</sub> = 2889		(Equation 25-8)	

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v <sub>R12</sub>	2889	4600	No

Level of Service Determination (if not F)

$$\text{Density, } D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 25.9 \text{ pc/mi/ln}$$

Level of service for ramp-freeway junction areas of influence C ✓

Speed Estimation

Intermediate speed variable,	M = 0.372	
Space mean speed in ramp influence area,	S <sub>R</sub> = 50.2	mph
Space mean speed in outer lanes,	S <sub>0</sub> = 49.9	mph
Space mean speed for all vehicles,	S = 50.1	mph

Phone: Fax:  
E-mail:

Merge Analysis

Analyst: CTR  
 Agency/Co.: HNTB  
 Date performed: 08/02/2010  
 Analysis time period: Build  
 Freeway/Dir of Travel: I-4 EB  
 Junction: On Ramp from US 1792  
 Jurisdiction: Seminole County  
 Analysis Year: 2012  
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	3	
Free-flow speed on freeway	70.0	mph
Volume on freeway	4990 ✓	vph

On Ramp Data

Side of freeway	Right ✓	
Number of lanes in ramp	1 ✓	
Free-flow speed on ramp	35.0	mph
Volume on ramp	360 ✓	vph
Length of first accel/decel lane	900	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent Ramp	660 ✓	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	Off	
Distance to adjacent Ramp	1948	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	4990 ✓	360	660	vph
Peak-hour factor, PHF	0.92 ✓	0.92	0.92	
Peak 15-min volume, v15	1356 ✓	98	179	v
Trucks and buses	9 ✓	9	9	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	%
Length	mi	mi	mi	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.957	0.957	0.957	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	5668	409	750	pcph

Estimation of V12 Merge Areas

$$L = 1128.28 \text{ (Equation 25-2 or 25-3)}$$

$$P = 0.603 \text{ Using Equation 1}$$

$$v_{12} = v_{FM} (P_{FM}) = 3416 \text{ pc/h}$$

Capacity Checks

	Actual	Maximum	LOS F?
v <sub>FO</sub>	6077	7200	No
v <sub>3 or av34</sub>	2252 pc/h	(Equation 25-4 or 25-5)	
Is v <sub>3 or av34</sub> > 2700 pc/h?		No	
Is v <sub>3 or av34</sub> > 1.5 v <sub>12</sub> / 2		No	
If yes, v <sub>12A</sub> = 3416		(Equation 25-8)	

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v <sub>R12</sub>	3416	4600	No

Level of Service Determination (if not F)

$$\text{Density, } D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 29.5 \text{ pc/mi/ln}$$

Level of service for ramp-freeway junction areas of influence D ✓

Speed Estimation

Intermediate speed variable,	M = 0.437	
Space mean speed in ramp influence area,	S = 57.8	mph
Space mean speed in outer lanes,	S = 63.7	mph
Space mean speed for all vehicles,	S = 59.8	mph

Phone: Fax:  
E-mail:

Diverge Analysis

Analyst: CTR  
 Agency/Co.: HNTB  
 Date performed: 8/02/2010  
 Analysis time period: Build  
 Freeway/Dir of Travel: I-4 WB  
 Junction: Off Ramp to US 1792  
 Jurisdiction: Seminole County  
 Analysis Year: 2012  
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	3	
Free-flow speed on freeway	70.0	mph
Volume on freeway	5350	vph

Off Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	35.0	mph
Volume on ramp	360	vph
Length of first accel/decel lane	500	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent ramp	660	vph
Position of adjacent ramp	Downstream	
Type of adjacent ramp	On	
Distance to adjacent ramp	1948	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	5350	360	660	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	1454	98	179	v
Trucks and buses	9	9	9	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00 %	0.00 %	0.00 %	
Length	0.00 mi	0.00 mi	0.00 mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	



Heavy vehicle adjustment, fHV	0.957	0.957	0.957	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	6077	409	750	pcph

Estimation of V12 Diverge Areas

---

$$L = \frac{EQ}{P} \quad (\text{Equation 25-8 or 25-9})$$

$$P = 0.589 \quad \text{Using Equation 5}$$

$$v_{12} = v_R + (v_F - v_R) P = 3749 \quad \text{pc/h}$$

Capacity Checks

---

	Actual	Maximum	LOS F?
$v_{Fi} = v_F$	6077	7200	No
$v_{FO} = v_F - v_R$	5668	7200	No
$v_R$	409	2000	No
$v_{3 \text{ or } 34}$	2328 pc/h	(Equation 25-15 or 25-16)	
Is $v_{3 \text{ or } 34} > 2700$ pc/h?		No	
Is $v_{3 \text{ or } 34} > 1.5 v_{12} / 2$		No	
If yes, $v_{12A} = 3749$		(Equation 25-18)	

Flow Entering Diverge Influence Area

---

	Actual	Max Desirable	Violation?
$v_{12}$	3749	4400	No

Level of Service Determination (if not F)

---

Density,  $D = 4.252 + 0.0086 v_{12} - 0.009 L_D = 32.0$  pc/mi/ln

Level of service for ramp-freeway junction areas of influence D ✓

Speed Estimation

---

Intermediate speed variable,	$D = 0.465$	
Space mean speed in ramp influence area,	$S = 57.0$	mph
Space mean speed in outer lanes,	$S = 71.6$	mph
Space mean speed for all vehicles,	$S = 61.8$	mph

---

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Phone: Fax:  
E-mail:

Merge Analysis

Analyst: CTRR  
 Agency/Co.: HNTB  
 Date performed: 3/07/2010  
 Analysis time period: Build Service Road Concept  
 Freeway/Dir of Travel: I-4 EB  
 Junction: On Ramp from SR 46  
 Jurisdiction: Seminole County  
 Analysis Year: 2012  
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	4	
Free-flow speed on freeway	55.0	mph
Volume on freeway	4650	vph

On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	2	
Free-flow speed on ramp	35.0	mph
Volume on ramp	1000	vph
Length of first accel/decel lane	500	ft
Length of second accel/decel lane	500	ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent Ramp	430	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	3172	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	4650	1000	430	vph
Peak-hour factor, PHF	0.90	0.90	0.90	

Peak 15-min volume, v <sub>15</sub>	1292	278	119	v
Trucks and buses	0	0	0	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, f <sub>HV</sub>	1.000	1.000	1.000	
Driver population factor, f <sub>P</sub>	1.00	1.00	1.00	
Flow rate, v <sub>p</sub>	5167	1111	478	pcph

---

Estimation of V12 Merge Areas

---

L = (Equation 25-2 or 25-3)

EQ

P = 0.209 Using Equation 0

FM

v = v (P) = 1080 pc/h

12 F FM

---

Capacity Checks

---

	Actual	Maximum	LOS F?
v	6278	9000	No
FO			
v v	2043 pc/h	(Equation 25-4 or 25-5)	
3 or av <sub>34</sub>			
Is v v > 2700 pc/h?		No	
3 or av <sub>34</sub>			
Is v v > 1.5 v /2		Yes	
3 or av <sub>34</sub> 12			
If yes, v = 2066		(Equation 25-8)	
12A			

---

Flow Entering Merge Influence Area

---

	Actual	Max Desirable	Violation?
v	2066	4600	No
12A			

---

Level of Service Determination (if not F)

---

Density, D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 20.3 pc/mi/ln

R            R            12            A

Level of service for ramp-freeway junction areas of influence C

---

Speed Estimation

---

Intermediate speed variable, M = 0.310

S

Space mean speed in ramp influence area, S = 51.0 mph

R

Space mean speed in outer lanes, S = 51.2 mph

0

Space mean speed for all vehicles, S = 51.1 mph

Phone:  
E-mail:

Fax:

Diverge Analysis

Analyst: CTR  
 Agency/Co.: HNTB  
 Date performed: 8/2/2010  
 Analysis time period: Build Service Road Concept  
 Freeway/Dir of Travel: I-4 WB  
 Junction: Off Ramp to SR 46  
 Jurisdiction: Seminole County  
 Analysis Year: 2012  
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	4	
Free-flow speed on freeway	55.0 ✓	mph
Volume on freeway	5650 ✓	vph

Off Ramp Data

Side of freeway	Right	
Number of lanes in ramp	2	
Free-Flow speed on ramp	35.0 ✓	mph
Volume on ramp	1000 ✓	vph
Length of first accel/decel lane	0	ft
Length of second accel/decel lane	500	ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent ramp	1700 ✓	vph
Position of adjacent ramp	Downstream	
Type of adjacent ramp	Off	
Distance to adjacent ramp	6098	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	5650 ✓	1000	1700	vph
Peak-hour factor, PHF	0.92 ✓	0.92	0.92	
Peak 15-min volume, v15	1535 ✓	272	462	v
Trucks and buses	9 ✓	9	9	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00 %	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.957	0.957	0.957	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	6418	1136	1931	pcph

Estimation of V12 Diverge Areas

---

$$L = \text{EQ} \quad (\text{Equation 25-8 or 25-9})$$

$$P = 0.260 \quad \text{Using Equation 0}$$

$$v_{12R} = v_{FR} + (v_{FR} - v_{FD}) P = 2509 \quad \text{pc/h}$$

Capacity Checks

---

	Actual	Maximum	LOS F?
$v_{Fi} = v_F$	6418	9000	No
$v_{FO} = v_F - v_R$	5282	9000	No
$v_R$	1136	3800	No
$v_{3 \text{ or } av34}$	1954 pc/h	(Equation 25-15 or 25-16)	
Is $v_{3 \text{ or } av34} > 2700$ pc/h?		No	
Is $v_{3 \text{ or } av34} > 1.5 v_{12} / 2$		Yes	
If yes, $v_{12A} = 2567$		(Equation 25-18)	

Flow Entering Diverge Influence Area

---

	Actual	Max Desirable	Violation?
$v_{12A}$	2567	4400	No

Level of Service Determination (if not F)

---

Density,  $D = 4.252 + 0.0086 v_{12R} - 0.009 L_D = 21.8 \quad \text{pc/mi/ln}$

Level of service for ramp-freeway junction areas of influence C ✓

Speed Estimation

---

Intermediate speed variable,	$D = 0.530$	
Space mean speed in ramp influence area,	$S = 48.1$	mph
Space mean speed in outer lanes,	$S = 56.7$	mph
Space mean speed for all vehicles,	$S = 52.9$	mph

---

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Phone: Fax:  
E-mail:

Diverge Analysis

Analyst: CTRR  
 Agency/Co.: HNTB  
 Date performed: 3/07/2010  
 Analysis time period: Build Service Road Concept  
 Freeway/Dir of Travel: I-4 WB  
 Junction: Off Ramp to SR 417 & CR 46A  
 Jurisdiction: Seminole County  
 Analysis Year: 2012  
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	4	
Free-flow speed on freeway	55.0	mph
Volume on freeway	4220	vph

Off Ramp Data

Side of freeway	Right	
Number of lanes in ramp	2	
Free-Flow speed on ramp	35.0	mph
Volume on ramp	1690	vph
Length of first accel/decel lane	500	ft
Length of second accel/decel lane	500	ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent ramp	1000	vph
Position of adjacent ramp	Upstream	
Type of adjacent ramp	Off	
Distance to adjacent ramp	6098	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
		Ramp		
Volume, V (vph)	4220	1690	1000	vph
Peak-hour factor, PHF	0.90	0.90	0.90	

Peak 15-min volume, v <sub>15</sub>	1172	469	278	v
Trucks and buses	0	0	0	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00 %	0.00 %	0.00 %	
Length	0.00 mi	0.00 mi	0.00 mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, f <sub>HV</sub>	1.000	1.000	1.000	
Driver population factor, f <sub>P</sub>	1.00	1.00	1.00	
Flow rate, v <sub>p</sub>	4689	1878	1111	pcph

---

Estimation of V12 Diverge Areas

---

L = (Equation 25-8 or 25-9)

EQ

P = 0.260 Using Equation 0

FD

$v = v + (v - v) P = 2609$  pc/h

12 R F R FD

---

Capacity Checks

---

	Actual	Maximum	LOS F?
$v = v$	4689	9000	No
F <sub>i</sub> F			
$v = v - v$	2811	9000	No
F <sub>O</sub> F R			
v	1878	3800	No
R			
$v v$	1040 pc/h	(Equation 25-15 or 25-16)	
3 or av34			
Is $v v > 2700$ pc/h?		No	
3 or av34			
Is $v v > 1.5 v / 2$		No	
3 or av34 12			
If yes, $v = 2609$		(Equation 25-18)	
12A			

---

Flow Entering Diverge Influence Area

---

	Actual	Max Desirable	Violation?
v	2609	4400	No
12			

---

Level of Service Determination (if not F)

---

Density,  $D = 4.252 + 0.0086 v - 0.009 L = 13.2$  pc/mi/ln

R 12 D

Level of service for ramp-freeway junction areas of influence B

---

Speed Estimation

---

Intermediate speed variable,  $D = 0.597$

S

Space mean speed in ramp influence area,  $S = 47.2$  mph

	R	
Space mean speed in outer lanes,		S = 60.2 mph
	0	
Space mean speed for all vehicles,		S = 52.2 mph

---



HCS+: Ramps and Ramp Junctions Release 5.4

Phone: Fax:  
E-mail:

Diverge Analysis

Analyst: CTRR  
Agency/Co.: HNTB  
Date performed: 3/07/2010  
Analysis time period: Build Service Road Concept  
Freeway/Dir of Travel: I-4 WB  
Junction: Off Ramp to SR 417 & CR 46A  
Jurisdiction: Seminole County  
Analysis Year: 2012  
Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	4	
Free-flow speed on freeway	55.0	mph
Volume on freeway	4220	vph

Off Ramp Data

Side of freeway	Right	
Number of lanes in ramp	2	
Free-Flow speed on ramp	35.0	mph
Volume on ramp	1690	vph
Length of first accel/decel lane	500	ft
Length of second accel/decel lane	500	ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent ramp	440	vph
Position of adjacent ramp	Downstream	
Type of adjacent ramp	On	
Distance to adjacent ramp	3490	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	4220	1690	440
Peak-hour factor, PHF	0.90	0.90	0.90

Peak 15-min volume, v <sub>15</sub>	1172	469	122	v
Trucks and buses	0	0	0	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00 %	0.00 %	0.00 %	
Length	0.00 mi	0.00 mi	0.00 mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, f <sub>HV</sub>	1.000	1.000	1.000	
Driver population factor, f <sub>P</sub>	1.00	1.00	1.00	
Flow rate, v <sub>p</sub>	4689	1878	489	pcph

Estimation of V12 Diverge Areas

---

L = (Equation 25-8 or 25-9)

EQ

P = 0.260 Using Equation 0

FD

$v = v + (v - v) P = 2609$  pc/h

12 R F R FD

Capacity Checks

---

	Actual	Maximum	LOS F?
v = v	4689	9000	No
F <sub>i</sub> F			
v = v - v	2811	9000	No
F <sub>O</sub> F R			
v	1878	3800	No
R			
v v	1040 pc/h	(Equation 25-15 or 25-16)	
3 or av <sub>34</sub>			
Is v v > 2700 pc/h?		No	
3 or av <sub>34</sub>			
Is v v > 1.5 v /2		No	
3 or av <sub>34</sub> 12			
If yes, v = 2609		(Equation 25-18)	
12A			

Flow Entering Diverge Influence Area

---

	Actual	Max Desirable	Violation?
v	2609	4400	No
12			

Level of Service Determination (if not F)

---

Density,  $D = 4.252 + 0.0086 v - 0.009 L = 13.2$  pc/mi/ln

R 12 D

Level of service for ramp-freeway junction areas of influence B

Speed Estimation

---

Intermediate speed variable,  $D = 0.597$

S

Space mean speed in ramp influence area,  $S = 47.2$  mph

Space mean speed in outer lanes,	$\frac{R}{0}$	S = 60.2	mph
Space mean speed for all vehicles,		S = 52.2	mph

---

Phone: Fax:  
E-mail:

Merge Analysis

Analyst: CTR  
 Agency/Co.: HNTB  
 Date performed: 8/2/2010  
 Analysis time period: Build Service Road Concept  
 Freeway/Dir of Travel: I-4 WB  
 Junction: On Ramp from SR 417  
 Jurisdiction: Seminole County  
 Analysis Year: 2012  
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	3	
Free-flow speed on freeway	55.0	mph
Volume on freeway	2950 ✓	vph

On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	2	
Free-flow speed on ramp	35.0	mph
Volume on ramp	440 ↓	vph
Length of first accel/decel lane	600 ↓	ft
Length of second accel/decel lane	1500 ↓	ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes ↓	
Volume on adjacent Ramp	2140 ↓	vph
Position of adjacent Ramp	Downstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	3654	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	2950 ↓	440	2140	vph
Peak-hour factor, PHF	0.92 ↓	0.92	0.92	
Peak 15-min volume, v15	802 ↓	120	582	v
Trucks and buses	9 ↓	9	9	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.957	0.957	0.957	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	3351	500	2431	pcph

Estimation of V12 Merge Areas

$$L = \text{(Equation 25-2 or 25-3)}$$

$$EQ$$

$$P = 0.555 \text{ Using Equation 0}$$

$$FM$$

$$v_{12} = v_F (P_{FM}) = 1860 \text{ pc/h}$$

Capacity Checks

	Actual	Maximum	LOS F?
v <sub>FO</sub>	3851	6750	No
v <sub>3 or av34</sub>	1491 pc/h	(Equation 25-4 or 25-5)	
Is v <sub>3 or av34</sub> > 2700 pc/h?		No	
Is v <sub>3 or av34</sub> > 1.5 v <sub>12</sub> / 2		Yes	
If yes, v <sub>12A</sub> = 1914		(Equation 25-8)	

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v <sub>12A</sub>	1914	4600	No

Level of Service Determination (if not F)

$$\text{Density, } D_R = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 7.1 \text{ pc/mi/ln}$$

Level of service for ramp-freeway junction areas of influence A ✓

Speed Estimation

Intermediate speed variable,	M = 0.176	
Space mean speed in ramp influence area,	S <sub>R</sub> = 52.7	mph
Space mean speed in outer lanes,	S <sub>0</sub> = 51.6	mph
Space mean speed for all vehicles,	S = 52.3	mph

Phone: Fax:  
 E-mail:

Merge Analysis

Analyst: CTR  
 Agency/Co.: HNTB  
 Date performed: 8/2/2010  
 Analysis time period: Build Service Road Concept  
 Freeway/Dir of Travel: I-4 WB  
 Junction: On Ramp from SR 417  
 Jurisdiction: Seminole County  
 Analysis Year: 2012  
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	3	
Free-flow speed on freeway	55.0	mph
Volume on freeway	2950 ✓	vph

On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	2	
Free-flow speed on ramp	35.0 ✓	mph
Volume on ramp	440 ✓	vph
Length of first accel/decel lane	600 ✓	ft
Length of second accel/decel lane	1500 ✓	ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes ✓	
Volume on adjacent Ramp	1700 ✓	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	Off	
Distance to adjacent Ramp	3490	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	2950	440	1700	vph
Peak-hour factor, PHF	0.92 ✓	0.92	0.92	
Peak 15-min volume, v15	802 ✓	120	462	v
Trucks and buses	9 ✓	9	9	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade		%	%	%
Length		mi	mi	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.957	0.957	0.957	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	3351	500	1931	pcph

Estimation of V12 Merge Areas

$$L = \text{EQ} \quad (\text{Equation 25-2 or 25-3})$$

$$P = 0.555 \quad \text{Using Equation 0}$$

$$v_{12} = v_{F \text{ FM}} (P) = 1860 \quad \text{pc/h}$$

Capacity Checks

	Actual	Maximum	LOS F?
v <sub>FO</sub>	3851	6750	No
v <sub>3 or av34</sub>	1491 pc/h	(Equation 25-4 or 25-5)	
Is v <sub>3 or av34</sub> > 2700 pc/h?		No	
Is v <sub>3 or av34</sub> > 1.5 v <sub>12</sub> / 2		Yes	
If yes, v <sub>12A</sub> = 1914		(Equation 25-8)	

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v <sub>12A</sub>	1914	4600	No

Level of Service Determination (if not F)

$$\text{Density, } D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 7.1 \quad \text{pc/mi/ln}$$

Level of service for ramp-freeway junction areas of influence A ✓

Speed Estimation

Intermediate speed variable,	M = 0.176	
Space mean speed in ramp influence area,	S = 52.7	mph
Space mean speed in outer lanes,	S = 51.6	mph
Space mean speed for all vehicles,	S = 52.3	mph

HCS+: Ramps and Ramp Junctions Release 5.4

Phone: Fax:  
E-mail:

Merge Analysis

Analyst: CTRR  
 Agency/Co.: HNTB  
 Date performed: 3/07/2010  
 Analysis time period: Build Service Road Concept  
 Freeway/Dir of Travel: I-4 WB  
 Junction: On Ramp from SR 46 & CR 46A  
 Jurisdiction: Seminole County  
 Analysis Year: 2012  
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	3	
Free-flow speed on freeway	55.0	mph
Volume on freeway	2970	vph

On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	2	
Free-flow speed on ramp	35.0	mph
Volume on ramp	2130	vph
Length of first accel/decel lane	900	ft
Length of second accel/decel lane	900	ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent Ramp	440	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	3654	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	2970	2130	440	vph
Peak-hour factor, PHF	0.90	0.90	0.90	



Peak 15-min volume, v <sub>15</sub>	825	592	122	v
Trucks and buses	0	0	0	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, f <sub>HV</sub>	1.000	1.000	1.000	
Driver population factor, f <sub>P</sub>	1.00	1.00	1.00	
Flow rate, v <sub>p</sub>	3300	2367	489	pcph

---

Estimation of V12 Merge Areas

---

L = (Equation 25-2 or 25-3)

EQ

P = 0.555 Using Equation 0

FM

v = v (P ) = 1832 pc/h

12 F FM

---

Capacity Checks

---

	Actual	Maximum	LOS F?
v	5667	6750	No
FO			
v v	1468 pc/h	(Equation 25-4 or 25-5)	
3 or av <sub>34</sub>			
Is v v > 2700 pc/h?		No	
3 or av <sub>34</sub>			
Is v v > 1.5 v /2		Yes	
3 or av <sub>34</sub> 12			
If yes, v = 1885		(Equation 25-8)	
12A			

---

Flow Entering Merge Influence Area

---

	Actual	Max Desirable	Violation?
v	1885	4600	No
12A			

---

Level of Service Determination (if not F)

---

Density, D = 5.475 + 0.00734 v<sub>R</sub> + 0.0078 v<sub>12</sub> - 0.00627 L<sub>A</sub> = 20.6 pc/mi/ln

Level of service for ramp-freeway junction areas of influence C

---

Speed Estimation

---

Intermediate speed variable, M = 0.406

S

Space mean speed in ramp influence area, S = 49.7 mph

R

Space mean speed in outer lanes, S = 51.7 mph

0

Space mean speed for all vehicles, S = 50.2 mph

Phone:  
E-mail:

Fax:

Merge Analysis

Analyst: CTR  
 Agency/Co.: HNTB  
 Date performed: 8/2/2010  
 Analysis time period: Build Service Road Concept  
 Freeway/Dir of Travel: CD Rd (West of I-4) WB  
 Junction: On Ramp from CR 46A to CD Rd  
 Jurisdiction: Seminole County  
 Analysis Year: 2012  
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	2	
Free-flow speed on freeway	55.0	mph
Volume on freeway	950 ✓	vph

On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	35.0	mph
Volume on ramp	1190 ✓	vph
Length of first accel/decel lane	600	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent Ramp	470 ✓	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	Off	
Distance to adjacent Ramp	1320	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	950 ✓	1190	470	vph
Peak-hour factor, PHF	0.92 ✓	0.92	0.92	
Peak 15-min volume, v15	258	323	128	v
Trucks and buses	9 ✓	9	9	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	%
Length	mi	mi	mi	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.957	0.957	0.957	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	1079	1352	534	pcph

---

Estimation of V12 Merge Areas

---

L = (Equation 25-2 or 25-3)  
EQ  
P = 1.000 Using Equation 0  
FM  
 $v_{12} = v_F (P_{FM}) = 1079 \text{ pc/h}$

---

Capacity Checks

---

	Actual	Maximum	LOS F?
v <sub>FO</sub>	2431	4500	No
v <sub>3 or av34</sub>	0 pc/h	(Equation 25-4 or 25-5)	
Is v <sub>3 or av34</sub> > 2700 pc/h?		No	
Is v <sub>3 or av34</sub> > 1.5 v <sub>12</sub> / 2		No	
If yes, v <sub>12A</sub> = 1079		(Equation 25-8)	

---

Flow Entering Merge Influence Area

---

	Actual	Max Desirable	Violation?
v <sub>R12</sub>	1079	4600	No

---

Level of Service Determination (if not F)

---

Density,  $D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 20.1 \text{ pc/mi/ln}$   
Level of service for ramp-freeway junction areas of influence C ✓

---

Speed Estimation

---

Intermediate speed variable,	M = 0.323	
Space mean speed in ramp influence area,	S = 50.8	mph
Space mean speed in outer lanes,	S = N/A	mph
Space mean speed for all vehicles,	S = 50.8	mph

---

Phone: Fax:  
 E-mail:

Diverge Analysis

Analyst: CTR  
 Agency/Co.: HNTB  
 Date performed: 8/02/2010  
 Analysis time period: Build Service Road Concept  
 Freeway/Dir of Travel: CD Rd (East of I-4) EB  
 Junction: Off Ramp to SR 417 EB  
 Jurisdiction: Seminole County  
 Analysis Year: 2012  
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	2	
Free-flow speed on freeway	55.0	mph
Volume on freeway	1390 ✓	vph

Off Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	35.0	mph
Volume on ramp	350 ✓	vph
Length of first accel/decel lane	0	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes ✓	
Volume on adjacent ramp	90 ✓	vph
Position of adjacent ramp	Downstream	
Type of adjacent ramp	Off	
Distance to adjacent ramp	1584	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	1390 ↓	350	90	vph
Peak-hour factor, PHF	0.92 ↓	0.92	0.92	
Peak 15-min volume, v15	378 ↓	95	24	v
Trucks and buses	9 ✓	9	9	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00 %	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.957	0.957	0.957	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	1579	398	102	pcph

---

Estimation of V12 Diverge Areas

---

$$L = \frac{EQ}{P} \quad (\text{Equation 25-8 or 25-9})$$

$$P = 1.000 \quad \text{Using Equation 0}$$

$$v_{12R} = v_{FR} + (v_{FR} - v_{FD}) P = 1579 \quad \text{pc/h}$$

---

Capacity Checks

---

	Actual	Maximum	LOS F?
$v_{Fi} = v_F$	1579	4500	No
$v_{FO} = v_F - v_R$	1181	4500	No
$v_R$	398	2000	No
$v_{3 \text{ or } 34}$	0 pc/h	(Equation 25-15 or 25-16)	
Is $v_{3 \text{ or } 34} > 2700$ pc/h?		No	
Is $v_{3 \text{ or } 34} > 1.5 v_{12} / 2$		No	
If yes, $v_{12A} = 1579$		(Equation 25-18)	

---

Flow Entering Diverge Influence Area

---

	Actual	Max Desirable	Violation?
$v_{12}$	1579	4400	No

---

Level of Service Determination (if not F)

---

Density,  $D = 4.252 + 0.0086 v_{12} - 0.009 L_D = 17.8$  pc/mi/ln

Level of service for ramp-freeway junction areas of influence B ✓

---

Speed Estimation

---

Intermediate speed variable,	$D = 0.464$	
Space mean speed in ramp influence area,	$S = 49.0$	mph
Space mean speed in outer lanes,	$S = \text{N/A}$	mph
Space mean speed for all vehicles,	$S = 49.0$	mph

---

Phone: Fax:  
 E-mail:

Diverge Analysis

Analyst: CTR  
 Agency/Co.: HNTB  
 Date performed: 8/02/2010  
 Analysis time period: Build Service Road Concept  
 Freeway/Dir of Travel: CD Rd (East of I-4) EB  
 Junction: Off Ramp to SR 417 WB  
 Jurisdiction: Seminole County  
 Analysis Year: 2012  
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	2	
Free-flow speed on freeway	55.0	mph
Volume on freeway	1040 ✓	vph

Off Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	35.0 ✓	mph
Volume on ramp	90 ✓	vph
Length of first accel/decel lane	0	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent ramp	350 ✓	vph
Position of adjacent ramp	Upstream	
Type of adjacent ramp	Off	
Distance to adjacent ramp	1584	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	1040 ✓	90	350	vph
Peak-hour factor, PHF	0.92 ✓	0.92	0.92	
Peak 15-min volume, v15	283 ✓	24	95	v
Trucks and buses	9 ✓	9	9	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00 %	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.957	0.957	0.957	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	1181	102	398	pcph

Estimation of V12 Diverge Areas

$$L = \frac{EQ}{P} \quad (\text{Equation 25-8 or 25-9})$$

$$P = 1.000 \quad \text{Using Equation 0}$$

$$v_{12} = v_R + (v_F - v_R) P = 1181 \quad \text{pc/h}$$

Capacity Checks

	Actual	Maximum	LOS F?
$v_{Fi} = v_F$	1181	4500	No
$v_{FO} = v_F - v_R$	1079	4500	No
$v_R$	102	2000	No
$v_{3 \text{ or } av34}$	0 pc/h	(Equation 25-15 or 25-16)	
Is $v_{3 \text{ or } av34} > 2700$ pc/h?		No	
Is $v_{3 \text{ or } av34} > 1.5 v_{12} / 2$		No	
If yes, $v_{12A} = 1181$		(Equation 25-18)	

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
$v_{12}$	1181	4400	No

Level of Service Determination (if not F)

Density,  $D = 4.252 + 0.0086 v_{12} - 0.009 L_D = 14.4$  pc/mi/ln

Level of service for ramp-freeway junction areas of influence B ✓

Speed Estimation

Intermediate speed variable,	$D = 0.437$	
Space mean speed in ramp influence area,	$S = 49.3$	mph
Space mean speed in outer lanes,	$S = N/A$	mph
Space mean speed for all vehicles,	$S = 49.3$	mph

Phone: \_\_\_\_\_ Fax: \_\_\_\_\_  
 E-mail: \_\_\_\_\_

Diverge Analysis

Analyst: CTR  
 Agency/Co.: HNTB  
 Date performed: 8/02/2010  
 Analysis time period: Build Service Road Concept  
 Freeway/Dir of Travel: CD Rd (East of I-4) EB  
 Junction: Off Ramp to SR 417 WB  
 Jurisdiction: Seminole County  
 Analysis Year: 2012  
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	2	
Free-flow speed on freeway	55.0	mph
Volume on freeway	1040 ✓	vph

Off Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	35.0 ✓	mph
Volume on ramp	90 ✓	vph
Length of first accel/decel lane	0	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent ramp	430 ✓	vph
Position of adjacent ramp	Downstream ✓	
Type of adjacent ramp	On	
Distance to adjacent ramp	1478	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	1040 ✓	90	430	vph
Peak-hour factor, PHF	0.92 ✓	0.92	0.92	
Peak 15-min volume, v15	283 ✓	24	117	v
Trucks and buses	9 ✓	9	9	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00 %	0.00 %	0.00 %	
Length	0.00 mi	0.00 mi	0.00 mi	
Trucks and buses PCE, ET	1.5*	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	



Heavy vehicle adjustment, fHV	0.957	0.957	0.957	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	1181	102	488	pcph

Estimation of V12 Diverge Areas

---

$$L = \text{(Equation 25-8 or 25-9)}$$

$$EQ$$

$$P = 1.000 \text{ Using Equation } 0$$

$$FD$$

$$v_{12R} = v_F + (v_F - v_R) P = 1181 \text{ pc/h}$$

Capacity Checks

---

	Actual	Maximum	LOS F?
$v_{Fi} = v_F$	1181	4500	No
$v_{FO} = v_F - v_R$	1079	4500	No
$v_R$	102	2000	No
$v_3 \text{ or } v_{av34}$	0 pc/h	(Equation 25-15 or 25-16)	
Is $v_3 \text{ or } v_{av34} > 2700 \text{ pc/h?}$		No	
Is $v_3 \text{ or } v_{av34} > 1.5 v_{12} / 2$		No	
If yes, $v_{12A} = 1181$		(Equation 25-18)	

Flow Entering Diverge Influence Area

---

	Actual	Max Desirable	Violation?
$v_{12}$	1181	4400	No

Level of Service Determination (if not F)

---

Density,  $D = 4.252 + 0.0086 v_{12} - 0.009 L_D = 14.4 \text{ pc/mi/ln}$

Level of service for ramp-freeway junction areas of influence B ✓

Speed Estimation

---

Intermediate speed variable,	$D = 0.437$	
Space mean speed in ramp influence area,	$S_R = 49.3$	mph
Space mean speed in outer lanes,	$S_0 = N/A$	mph
Space mean speed for all vehicles,	$S = 49.3$	mph

---

HCS+: Ramps and Ramp Junctions Release 5.4

Phone: Fax:  
E-mail:

Merge Analysis

Analyst: CTRR  
Agency/Co.: HNTB  
Date performed: 3/07/2010  
Analysis time period: Build Service Road Concept  
Freeway/Dir of Travel: CD Rd (East of I-4) EB  
Junction: On Ramp from SR 417 EB  
Jurisdiction: Seminole County  
Analysis Year: 2012  
Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Merge		
Number of lanes in freeway	2		
Free-flow speed on freeway	55.0	mph	
Volume on freeway	950	vph	

On Ramp Data

Side of freeway	Left		
Number of lanes in ramp	2		
Free-flow speed on ramp	35.0	mph	
Volume on ramp	430	vph	
Length of first accel/decel lane	500	ft	
Length of second accel/decel lane	500	ft	

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes		
Volume on adjacent Ramp	90	vph	
Position of adjacent Ramp	Upstream		
Type of adjacent Ramp	Off		
Distance to adjacent Ramp	1478	ft	

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
Volume, V (vph)	950	430	90	vph
Peak-hour factor, PHF	0.90	0.90	0.90	

Peak 15-min volume, v <sub>15</sub>	264	119	25	v
Trucks and buses	0	0	0	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, f <sub>HV</sub>	1.000	1.000	1.000	
Driver population factor, f <sub>P</sub>	1.00	1.00	1.00	
Flow rate, v <sub>p</sub>	1056	478	100	pcph

Estimation of V12 Merge Areas

---

L = (Equation 25-2 or 25-3)

EQ

P = 1.000 Using Equation 0

FM

v = v (P ) = 1056 pc/h

12 F FM

Capacity Checks

---

	Actual	Maximum	LOS F?
v	1534	4500	No
FO			
v v	0 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		No	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34	12		
If yes, v = 1056		(Equation 25-8)	
12A			

Flow Entering Merge Influence Area

---

	Actual	Max Desirable	Violation?
v	1056	4600	No
R12			

Level of Service Determination (if not F)

---

Density, D = 5.475 + 0.00734 v<sub>R</sub> + 0.0078 v<sub>12</sub> - 0.00627 L<sub>A</sub> = 7.8 pc/mi/ln

Level of service for ramp-freeway junction areas of influence A

Speed Estimation

---

Intermediate speed variable, M = 0.237

S

Space mean speed in ramp influence area, S = 51.9 mph

R

Space mean speed in outer lanes, S = N/A mph

0

Space mean speed for all vehicles, S = 51.9 mph

HCS+: Ramps and Ramp Junctions Release 5.4

Phone: Fax:  
E-mail:

Merge Analysis

Analyst: CTRR  
 Agency/Co.: HNTB  
 Date performed: 3/07/2010  
 Analysis time period: Build Service Road Concept  
 Freeway/Dir of Travel: CD Rd (East of I-4) EB  
 Junction: On Ramp from SR 417 EB  
 Jurisdiction: Seminole County  
 Analysis Year: 2012  
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Merge		
Number of lanes in freeway	2		
Free-flow speed on freeway	55.0	mph	
Volume on freeway	950	vph	

On Ramp Data

Side of freeway	Left		
Number of lanes in ramp	2		
Free-flow speed on ramp	35.0	mph	
Volume on ramp	430	vph	
Length of first accel/decel lane	500	ft	
Length of second accel/decel lane	500	ft	

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes		
Volume on adjacent Ramp	430	vph	
Position of adjacent Ramp	Downstream		
Type of adjacent Ramp	Off		
Distance to adjacent Ramp	3010	ft	

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
	Ramp			
Volume, V (vph)	950	430	430	vph
Peak-hour factor, PHF	0.90	0.90	0.90	

Peak 15-min volume, v <sub>15</sub>	264	119	119	v
Trucks and buses	0	0	0	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, f <sub>HV</sub>	1.000	1.000	1.000	
Driver population factor, f <sub>P</sub>	1.00	1.00	1.00	
Flow rate, v <sub>p</sub>	1056	478	478	pcph

---

Estimation of V12 Merge Areas

---

L = (Equation 25-2 or 25-3)

EQ

P = 1.000 Using Equation 0

FM

v = v (P) = 1056 pc/h

12 F FM

---

Capacity Checks

---

	Actual	Maximum	LOS F?
v	1534	4500	No
FO			
v v	0 pc/h	(Equation 25-4 or 25-5)	
3 or av <sub>34</sub>			
Is v v > 2700 pc/h?		No	
3 or av <sub>34</sub>			
Is v v > 1.5 v /2		No	
3 or av <sub>34</sub>	12		
If yes, v = 1056		(Equation 25-8)	
12A			

---

Flow Entering Merge Influence Area

---

	Actual	Max Desirable	Violation?
v	1056	4600	No
R12			

---

Level of Service Determination (if not F)

---

Density, D = 5.475 + 0.00734 v<sub>R</sub> + 0.0078 v<sub>A</sub> - 0.00627 L = 7.8 pc/mi/ln

Level of service for ramp-freeway junction areas of influence A

---

Speed Estimation

---

Intermediate speed variable, M = 0.237

S

Space mean speed in ramp influence area, S = 51.9 mph

R

Space mean speed in outer lanes, S = N/A mph

0

Space mean speed for all vehicles, S = 51.9 mph

HCS+: Ramps and Ramp Junctions Release 5.4

Phone: Fax:  
E-mail:

Diverge Analysis

Analyst: CTRR  
 Agency/Co.: HNTB  
 Date performed: 3/07/2010  
 Analysis time period: Build Service Road Concept  
 Freeway/Dir of Travel: CD Rd (East of I-4) EB  
 Junction: Off Ramp to I-4 EB  
 Jurisdiction: Seminole County  
 Analysis Year: 2012  
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	3	
Free-flow speed on freeway	55.0	mph
Volume on freeway	1380	vph

Off Ramp Data

Side of freeway	Left	
Number of lanes in ramp	2	
Free-Flow speed on ramp	35.0	mph
Volume on ramp	430	vph
Length of first accel/decel lane	500	ft
Length of second accel/decel lane	500	ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent ramp	430	vph
Position of adjacent ramp	Upstream	
Type of adjacent ramp	On	
Distance to adjacent ramp	3010	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	1380	430	430	vph
Peak-hour factor, PHF	0.90	0.90	0.90	

Peak 15-min volume, v <sub>15</sub>	383	119	119	v
Trucks and buses	0	0	0	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00 %	0.00 %	0.00 %	
Length	0.00 mi	0.00 mi	0.00 mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, f <sub>HV</sub>	1.000	1.000	1.000	
Driver population factor, f <sub>P</sub>	1.00	1.00	1.00	
Flow rate, v <sub>p</sub>	1533	478	478	pcph

Estimation of V12 Diverge Areas

---

L = (Equation 25-8 or 25-9)

EQ

P = 0.450 Using Equation 0

FD

v = v + (v - v) P = 953 pc/h

12 R F R FD

Capacity Checks

---

	Actual	Maximum	LOS F?
v = v	1533	6750	No
F <sub>i</sub> F			
v = v - v	1055	6750	No
F <sub>O</sub> F R			
v	478	3800	No
R			
v v	580 pc/h	(Equation 25-15 or 25-16)	
3 or av <sub>34</sub>			
Is v v > 2700 pc/h?		No	
3 or av <sub>34</sub>			
Is v v > 1.5 v / 2		No	
3 or av <sub>34</sub> 12			
If yes, v = 953		(Equation 25-18)	
12A			

Flow Entering Diverge Influence Area

---

	Actual	Max Desirable	Violation?
v	953	4400	No
12			

Level of Service Determination (if not F)

---

Density, D = 4.252 + 0.0086 v - 0.009 L = -0.6 pc/mi/ln

R 12 D

Level of service for ramp-freeway junction areas of influence A

Speed Estimation

---

Intermediate speed variable, D = 0.471

S

Space mean speed in ramp influence area, S = 48.9 mph

Space mean speed in outer lanes,	$S = 60.3$	mph
Space mean speed for all vehicles,	$S = 52.3$	mph

---



Phone: Fax:  
 E-mail:

Merge Analysis

Analyst: CTR  
 Agency/Co.: HNTB  
 Date performed: 8/02/2010  
 Analysis time period: Build Service Road Concept  
 Freeway/Dir of Travel: CD Rd (West of I-4) WB  
 Junction: On Ramp from SR 46 EB  
 Jurisdiction: Seminole County  
 Analysis Year: 2012  
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	2	
Free-flow speed on freeway	55.0 ✓	mph
Volume on freeway	720 ✓	vph

On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	35.0 ✓	mph
Volume on ramp	340 ✓	vph
Length of first accel/decel lane	700	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent Ramp	1700 ✓	vph
Position of adjacent Ramp	Downstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1426	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	720 ✓	340	1700	vph
Peak-hour factor, PHF	0.92 ✓	0.92	0.92	
Peak 15-min volume, v15	196 ✓	92	462	v
Trucks and buses	9 ✓	9	9	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade		%	%	%
Length		mi	mi	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.957	0.957	0.957	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	818	386	1931	pcph

---

Estimation of V12 Merge Areas

---

L = (Equation 25-2 or 25-3)  
EQ  
P = 1.000 Using Equation 0  
FM  
 $v_{12} = v_F (P_{FM}) = 818 \text{ pc/h}$

---

Capacity Checks

---

	v	Actual	Maximum	LOS F?
	FO	1204	4500	No
	v	0 pc/h	(Equation 25-4 or 25-5)	
	3 or av34			
Is	v	> 2700 pc/h?	No	
	3 or av34			
Is	v	> 1.5 v /2	No	
	3 or av34	12		
If yes,	v	= 818	(Equation 25-8)	
	12A			

---

Flow Entering Merge Influence Area

---

	v	Actual	Max Desirable	Violation?
	R12	818	4600	No

---

Level of Service Determination (if not F)

---

Density,  $D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 10.3 \text{ pc/mi/ln}$   
Level of service for ramp-freeway junction areas of influence B ✓

---

Speed Estimation

---

Intermediate speed variable,	M = 0.285	
	S	
Space mean speed in ramp influence area,	S = 51.3	mph
	R	
Space mean speed in outer lanes,	S = N/A	mph
	0	
Space mean speed for all vehicles,	S = 51.3	mph

---

HCS+: Ramps and Ramp Junctions Release 5.4

Phone: Fax:  
E-mail:

Merge Analysis

Analyst: CTRR  
 Agency/Co.: HNTB  
 Date performed: 3/07/2010  
 Analysis time period: Build Service Road Concept  
 Freeway/Dir of Travel: CD Rd (West of I-4) WB  
 Junction: On Ramp from I-4 WB  
 Jurisdiction: Seminole County  
 Analysis Year: 2012  
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Merge		
Number of lanes in freeway	4		
Free-flow speed on freeway	55.0	mph	
Volume on freeway	1060	vph	

On Ramp Data

Side of freeway	Left		
Number of lanes in ramp	2		
Free-flow speed on ramp	35.0	mph	
Volume on ramp	1690	vph	
Length of first accel/decel lane	530	ft	
Length of second accel/decel lane	530	ft	

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes		
Volume on adjacent Ramp	340	vph	
Position of adjacent Ramp	Upstream		
Type of adjacent Ramp	On		
Distance to adjacent Ramp	1426	ft	

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	1060	1690	340	vph
Peak-hour factor, PHF	0.90	0.90	0.90	

Peak 15-min volume, v <sub>15</sub>	294	469	94	v
Trucks and buses	0	0	0	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, f <sub>HV</sub>	1.000	1.000	1.000	
Driver population factor, f <sub>P</sub>	1.00	1.00	1.00	
Flow rate, v <sub>p</sub>	1178	1878	378	pcph

---

Estimation of V12 Merge Areas

---

L = (Equation 25-2 or 25-3)

EQ

P = 0.209 Using Equation 0

FM

v = v (P ) = 246 pc/h

12 F FM

---

Capacity Checks

---

	Actual	Maximum	LOS F?
v	3056	9000	No
FO			
v v	466 pc/h	(Equation 25-4 or 25-5)	
3 or av <sub>34</sub>			
Is v v > 2700 pc/h?		No	
3 or av <sub>34</sub>			
Is v v > 1.5 v /2		Yes	
3 or av <sub>34</sub> 12			
If yes, v = 471		(Equation 25-8)	
12A			

---

Flow Entering Merge Influence Area

---

	Actual	Max Desirable	Violation?
v	471	4600	No
12A			

---

Level of Service Determination (if not F)

---

Density, D = 5.475 + 0.00734 v<sub>R</sub> + 0.0078 v<sub>12</sub> - 0.00627 L<sub>A</sub> = 13.7 pc/mi/ln

Level of service for ramp-freeway junction areas of influence B

---

Speed Estimation

---

Intermediate speed variable, M = 0.253

S

Space mean speed in ramp influence area, S = 51.7 mph

R

Space mean speed in outer lanes, S = 55.0 mph

0

Space mean speed for all vehicles, S = 52.3 mph

HCS+: Ramps and Ramp Junctions Release 5.4

Phone: Fax:  
E-mail:

Merge Analysis

Analyst: CTRR  
 Agency/Co.: HNTB  
 Date performed: 3/07/2010  
 Analysis time period: Build Service Road Concept  
 Freeway/Dir of Travel: CD Rd (West of I-4) WB  
 Junction: On Ramp from I-4 WB  
 Jurisdiction: Seminole County  
 Analysis Year: 2012  
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	4	
Free-flow speed on freeway	55.0	mph
Volume on freeway	1060	vph

On Ramp Data

Side of freeway	Left	
Number of lanes in ramp	2	
Free-flow speed on ramp	35.0	mph
Volume on ramp	1690	vph
Length of first accel/decel lane	530	ft
Length of second accel/decel lane	530	ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent Ramp	430	vph
Position of adjacent Ramp	Downstream	
Type of adjacent Ramp	Off	
Distance to adjacent Ramp	2851	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
		Ramp		
Volume, V (vph)	1060	1690	430	vph
Peak-hour factor, PHF	0.90	0.90	0.90	

Peak 15-min volume, v <sub>15</sub>	294	469	119	v
Trucks and buses	0	0	0	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, f <sub>HV</sub>	1.000	1.000	1.000	
Driver population factor, f <sub>P</sub>	1.00	1.00	1.00	
Flow rate, v <sub>p</sub>	1178	1878	478	pcph

Estimation of V12 Merge Areas

---

L = (Equation 25-2 or 25-3)

EQ

P = 0.209 Using Equation 0

FM

v = v (P ) = 246 pc/h

12 F FM

Capacity Checks

---

	Actual	Maximum	LOS F?
v	3056	9000	No
FO			
v v	466 pc/h	(Equation 25-4 or 25-5)	
3 or av <sub>34</sub>			
Is v v > 2700 pc/h?		No	
3 or av <sub>34</sub>			
Is v v > 1.5 v /2		Yes	
3 or av <sub>34</sub> 12			
If yes, v = 471		(Equation 25-8)	
12A			

Flow Entering Merge Influence Area

---

	Actual	Max Desirable	Violation?
v	471	4600	No
12A			

Level of Service Determination (if not F)

---

Density, D = 5.475 + 0.00734 v<sub>R</sub> + 0.0078 v<sub>12</sub> - 0.00627 L<sub>A</sub> = 13.7 pc/mi/ln

Level of service for ramp-freeway junction areas of influence B

Speed Estimation

---

Intermediate speed variable, M = 0.253

S

Space mean speed in ramp influence area, S = 51.7 mph

R

Space mean speed in outer lanes, S = 55.0 mph

0

Space mean speed for all vehicles, S = 52.3 mph

Phone: Fax:  
E-mail:

Diverge Analysis

Analyst: CTR  
 Agency/Co.: HNTB  
 Date performed: 8/2/2010  
 Analysis time period: Build Service Road Concept  
 Freeway/Dir of Travel: CD Rd (West of I-4) WB  
 Junction: Off Ramp to SR 417 WB  
 Jurisdiction: Seminole County  
 Analysis Year: 2012  
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	3	
Free-flow speed on freeway	55.0	mph
Volume on freeway	2760	vph

Off Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	35.0	mph
Volume on ramp	430	vph
Length of first accel/decel lane	500	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent ramp	910	vph
Position of adjacent ramp	Downstream	
Type of adjacent ramp	Off	
Distance to adjacent ramp	1531	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	2760	430	910	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	750	117	247	v
Trucks and buses	9	9	9	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00	%	0.00	%
Length	0.00	mi	0.00	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.957	0.957	0.957	
Driver population factor, fP	1.00	1.00	1.00	





HCS+: Ramps and Ramp Junctions Release 5.4

Phone: Fax:  
E-mail:

Diverge Analysis

Analyst: CTRR  
Agency/Co.: HNTB  
Date performed: 3/07/2010  
Analysis time period: Build Service Road Concept  
Freeway/Dir of Travel: CD Rd (West of I-4) WB  
Junction: Off Ramp to SR 417 WB  
Jurisdiction: Seminole County  
Analysis Year: 2012  
Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	3	
Free-flow speed on freeway	55.0	mph
Volume on freeway	2750	vph

Off Ramp Data

Side of freeway	Right	
Number of lanes in ramp	2	
Free-Flow speed on ramp	35.0	mph
Volume on ramp	430	vph
Length of first accel/decel lane	730	ft
Length of second accel/decel lane	730	ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent ramp	910	vph
Position of adjacent ramp	Downstream	
Type of adjacent ramp	Off	
Distance to adjacent ramp	1531	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
Volume, V (vph)	2750	430	910	vph
Peak-hour factor, PHF	0.90	0.90	0.90	

Peak 15-min volume, v15	764	119	253	v
Trucks and buses	0	0	0	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00 %	0.00 %	0.00 %	
Length	0.00 mi	0.00 mi	0.00 mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	1.000	1.000	1.000	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	3056	478	1011	pcph

Estimation of V12 Diverge Areas

---

L = (Equation 25-8 or 25-9)  
EQ  
P = 0.450 Using Equation 0  
FD  
 $v = v + (v - v) P = 1638$  pc/h  
12 R F R FD

Capacity Checks

---

	Actual	Maximum	LOS F?
v = v	3056	6750	No
Fi F			
v = v - v	2578	6750	No
FO F R			
v	478	3800	No
R			
v v	1418 pc/h	(Equation 25-15 or 25-16)	
3 or av34			
Is v v > 2700 pc/h?		No	
3 or av34			
Is v v > 1.5 v /2		Yes	
3 or av34 12			
If yes, v = 1746		(Equation 25-18)	
12A			

Flow Entering Diverge Influence Area

---

	Actual	Max Desirable	Violation?
v	1746	4400	No
12A			

Level of Service Determination (if not F)

---

Density,  $D = 4.252 + 0.0086 v - 0.009 L = -0.4$  pc/mi/ln  
R 12 D  
Level of service for ramp-freeway junction areas of influence A

Speed Estimation

---

Intermediate speed variable,  $D = 0.471$   
S  
Space mean speed in ramp influence area,  $S = 48.9$  mph

Space mean speed in outer lanes,	$S = 59.1$ mph
Space mean speed for all vehicles,	$S = 52.8$ mph

---

HCS+: Ramps and Ramp Junctions Release 5.4

Phone: Fax:  
E-mail:

Diverge Analysis

Analyst: CTRR  
 Agency/Co.: HNTB  
 Date performed: 3/07/2010  
 Analysis time period: Build Service Road Concept  
 Freeway/Dir of Travel: CD Rd (West of I-4) WB  
 Junction: Off Ramp to SR 417 EB  
 Jurisdiction: Seminole County  
 Analysis Year: 2012  
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	2	
Free-flow speed on freeway	55.0	mph
Volume on freeway	2320	vph

Off Ramp Data

Side of freeway	Right	
Number of lanes in ramp	2	
Free-Flow speed on ramp	35.0	mph
Volume on ramp	910	vph
Length of first accel/decel lane	500	ft
Length of second accel/decel lane	500	ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent ramp	430	vph
Position of adjacent ramp	Upstream	
Type of adjacent ramp	Off	
Distance to adjacent ramp	1531	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
		Ramp		
Volume, V (vph)	2320	910	430	vph
Peak-hour factor, PHF	0.90	0.90	0.90	

Peak 15-min volume, v <sub>15</sub>	644	253	119	v
Trucks and buses	0	0	0	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00 %	0.00 %	0.00 %	
Length	0.00 mi	0.00 mi	0.00 mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, f <sub>HV</sub>	1.000	1.000	1.000	
Driver population factor, f <sub>P</sub>	1.00	1.00	1.00	
Flow rate, v <sub>p</sub>	2578	1011	478	pcph

Estimation of V12 Diverge Areas

---

L = (Equation 25-8 or 25-9)

EQ

P = 1.000 Using Equation 0

FD

$v = v + (v - v) P = 2578$  pc/h

12 R F R FD

Capacity Checks

---

	Actual	Maximum	LOS F?
v = v	2578	4500	No
F <sub>i</sub> F			
v = v - v	1567	4500	No
F <sub>O</sub> F R			
v	1011	3800	No
R			
v v	0 pc/h	(Equation 25-15 or 25-16)	
3 or av34			
Is v v > 2700 pc/h?		No	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34 12			
If yes, v = 2578		(Equation 25-18)	
12A			

Flow Entering Diverge Influence Area

---

	Actual	Max Desirable	Violation?
v	2578	4400	No
12			

Level of Service Determination (if not F)

---

Density,  $D = 4.252 + 0.0086 v - 0.009 L = 12.9$  pc/mi/ln

R 12 D

Level of service for ramp-freeway junction areas of influence B

Speed Estimation

---

Intermediate speed variable,  $D = 0.519$

S

Space mean speed in ramp influence area,  $S = 48.3$  mph

Space mean speed in outer lanes,	$\frac{R}{0}$	S = N/A	mph
Space mean speed for all vehicles,	0	S = 48.3	mph

---

HCS+: Ramps and Ramp Junctions Release 5.4

Phone: Fax:  
E-mail:

Diverge Analysis

Analyst: CTRR  
 Agency/Co.: HNTB  
 Date performed: 3/07/2010  
 Analysis time period: Build Service Road Concept  
 Freeway/Dir of Travel: CD Rd (West of I-4) WB  
 Junction: Off Ramp to SR 417 EB  
 Jurisdiction: Seminole County  
 Analysis Year: 2012  
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Diverge		
Number of lanes in freeway	2		
Free-flow speed on freeway	55.0	mph	
Volume on freeway	2320	vph	

Off Ramp Data

Side of freeway	Right		
Number of lanes in ramp	2		
Free-Flow speed on ramp	35.0	mph	
Volume on ramp	910	vph	
Length of first accel/decel lane	500	ft	
Length of second accel/decel lane	500	ft	

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes		
Volume on adjacent ramp	470	vph	
Position of adjacent ramp	Downstream		
Type of adjacent ramp	Off		
Distance to adjacent ramp	4594	ft	

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
		Ramp		
Volume, V (vph)	2320	910	470	vph
Peak-hour factor, PHF	0.90	0.90	0.90	

Peak 15-min volume, v <sub>15</sub>	644	253	131	v
Trucks and buses	0	0	0	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00 %	0.00 %	0.00 %	
Length	0.00 mi	0.00 mi	0.00 mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, f <sub>HV</sub>	1.000	1.000	1.000	
Driver population factor, f <sub>P</sub>	1.00	1.00	1.00	
Flow rate, v <sub>p</sub>	2578	1011	522	pcph

---

Estimation of V12 Diverge Areas

---

L = (Equation 25-8 or 25-9)

EQ

P = 1.000 Using Equation 0

FD

$$v = v + (v - v) P = 2578 \text{ pc/h}$$

12 R F R FD

---

Capacity Checks

---

	Actual	Maximum	LOS F?
v = v	2578	4500	No
F <sub>i</sub> F			
v = v - v	1567	4500	No
F <sub>O</sub> F R			
v	1011	3800	No
R			
v v	0 pc/h	(Equation 25-15 or 25-16)	
3 or av <sub>34</sub>			
Is v v > 2700 pc/h?		No	
3 or av <sub>34</sub>			
Is v v > 1.5 v /2		No	
3 or av <sub>34</sub> 12			
If yes, v = 2578		(Equation 25-18)	
12A			

---

Flow Entering Diverge Influence Area

---

	Actual	Max Desirable	Violation?
v	2578	4400	No
12			

---

Level of Service Determination (if not F)

---

Density,  $D = 4.252 + 0.0086 v - 0.009 L = 12.9 \text{ pc/mi/ln}$

R 12 D

Level of service for ramp-freeway junction areas of influence B

---

Speed Estimation

---

Intermediate speed variable,  $D = 0.519$

S

Space mean speed in ramp influence area,  $S = 48.3 \text{ mph}$



Space mean speed in outer lanes,	R	S = N/A	mph
Space mean speed for all vehicles,	0	S = 48.3	mph

---

Phone: Fax:  
E-mail:

Diverge Analysis

Analyst: CTR  
 Agency/Co.: HNTB  
 Date performed: 8/2/2010  
 Analysis time period: Build Service Road Concept  
 Freeway/Dir of Travel: CD Rd (West of I-4) WB  
 Junction: Off Ramp to CR 46A  
 Jurisdiction: Seminole County  
 Analysis Year: 2012  
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	2	
Free-flow speed on freeway	55.0 ✓	mph
Volume on freeway	1420 ✓	vph

Off Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	35.0 ✓	mph
Volume on ramp	470 ✓	vph
Length of first accel/decel lane	0	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes ✓	
Volume on adjacent ramp	1190 ✓	vph
Position of adjacent ramp	Downstream	
Type of adjacent ramp	On	
Distance to adjacent ramp	1320	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	1420 ✓	470	1190	vph
Peak-hour factor, PHF	0.92 ✓	0.92	0.92	
Peak 15-min volume, v15	386 ✓	128	323	v
Trucks and buses	9 ✓	9	9	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00 %	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.957	0.957	0.957	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	1613	534	1352	pcph

Estimation of V12 Diverge Areas

$$L = \text{EQ} \quad (\text{Equation 25-8 or 25-9})$$

$$P = 1.000 \quad \text{Using Equation 0}$$

$$v_{12} = v_R + (v_F - v_R) P = 1613 \quad \text{pc/h}$$

Capacity Checks

	Actual	Maximum	LOS F?
$v_{Fi} = v_F$	1613	4500	No
$v_{FO} = v_F - v_R$	1079	4500	No
$v_R$	534	2000	No
$v_{3 \text{ or } av34}$	0 pc/h	(Equation 25-15 or 25-16)	
Is $v_{3 \text{ or } av34} > 2700$ pc/h?		No	
Is $v_{3 \text{ or } av34} > 1.5 v_{12} / 2$		No	
If yes, $v_{12A} = 1613$		(Equation 25-18)	

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
$v_{12}$	1613	4400	No

Level of Service Determination (if not F)

Density,  $D = 4.252 + 0.0086 v_{12} - 0.009 L_D = 18.1$  pc/mi/ln

Level of service for ramp-freeway junction areas of influence B ✓

Speed Estimation

Intermediate speed variable,	$D = 0.476$	
Space mean speed in ramp influence area,	$S_R = 48.8$	mph
Space mean speed in outer lanes,	$S_0 = \text{N/A}$	mph
Space mean speed for all vehicles,	$S = 48.8$	mph

Phone: Fax:  
E-mail:

Diverge Analysis

Analyst: CTR  
 Agency/Co.: HNTB  
 Date performed: 8/2/2010  
 Analysis time period: Build Service Road Concept  
 Freeway/Dir of Travel: CD Rd (West of I-4) WB  
 Junction: Off Ramp to CR 46A  
 Jurisdiction: Seminole County  
 Analysis Year: 2012  
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Diverge		
Number of lanes in freeway	2		
Free-flow speed on freeway	55.0 ✓	mph	
Volume on freeway	1420 ✓	vph	

Off Ramp Data

Side of freeway	Right		
Number of lanes in ramp	1		
Free-Flow speed on ramp	35.0 ✓	mph	
Volume on ramp	470 ✓	vph	
Length of first accel/decel lane	0	ft	
Length of second accel/decel lane		ft	

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes		
Volume on adjacent ramp	910 ✓	vph	
Position of adjacent ramp	Upstream		
Type of adjacent ramp	Off		
Distance to adjacent ramp	4594	ft	

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	1420 ✓	470	910	vph
Peak-hour factor, PHF	0.92 ✓	0.92	0.92	
Peak 15-min volume, v15	386	128	247	v
Trucks and buses	9 ✓	9	9	%
Recreational vehicles	0 ✓	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00 %	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.957	0.957	0.957	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	1613	534	1034	pcph

Estimation of V12 Diverge Areas

---

L = (Equation 25-8 or 25-9)  
EQ  
P = 1.000 Using Equation 0  
FD  
 $v_{12} = v_R + (v_F - v_R) P = 1613 \text{ pc/h}$

Capacity Checks

---

	Actual	Maximum	LOS F?
$v_{Fi} = v_F$	1613	4500	No
$v_{FO} = v_F - v_R$	1079	4500	No
$v_R$	534	2000	No
$v_{3 \text{ or } 34}$	0 pc/h	(Equation 25-15 or 25-16)	
Is $v_{3 \text{ or } 34} > 2700 \text{ pc/h?}$		No	
Is $v_{3 \text{ or } 34} > 1.5 v_{12} / 2$		No	
If yes, $v_{12A} = 1613$		(Equation 25-18)	

Flow Entering Diverge Influence Area

---

	Actual	Max Desirable	Violation?
$v_{12}$	1613	4400	No

Level of Service Determination (if not F)

---

Density,  $D = 4.252 + 0.0086 v_{12} - 0.009 L_D = 18.1 \text{ pc/mi/ln}$   
Level of service for ramp-freeway junction areas of influence B ✓

Speed Estimation

---

Intermediate speed variable,	D = 0.476	
Space mean speed in ramp influence area,	S = 48.8	mph
Space mean speed in outer lanes,	S = N/A	mph
Space mean speed for all vehicles,	S = 48.8	mph

---

HCS+: Ramps and Ramp Junctions Release 5.4

Phone: Fax:  
E-mail:

Merge Analysis

Analyst: CTRR  
 Agency/Co.: HNTB  
 Date performed: 3/07/2010  
 Analysis time period: Build Service Road Concept  
 Freeway/Dir of Travel: CD Rd (West of I-4) WB  
 Junction: On Ramp from SR 46 EB  
 Jurisdiction: Seminole County  
 Analysis Year: 2012  
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	2	
Free-flow speed on freeway	55.0	mph
Volume on freeway	940	vph

On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	35.0	mph
Volume on ramp	1190	vph
Length of first accel/decel lane	500	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent Ramp	470	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	Off	
Distance to adjacent Ramp	1320	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	940	1190	470	vph
Peak-hour factor, PHF	0.90	0.90	0.90	

Peak 15-min volume, v <sub>15</sub>	261	331	131	v
Trucks and buses	0	0	0	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, f <sub>HV</sub>	1.000	1.000	1.000	
Driver population factor, f <sub>P</sub>	1.00	1.00	1.00	
Flow rate, v <sub>p</sub>	1044	1322	522	pcph

Estimation of V12 Merge Areas

---

L = (Equation 25-2 or 25-3)

EQ

P = 1.000 Using Equation 0

FM

v = v (P) = 1044 pc/h

12 F FM

Capacity Checks

---

	Actual	Maximum	LOS F?
v	2366	4500	No
FO			
v v	0 pc/h	(Equation 25-4 or 25-5)	
3 or av <sub>34</sub>			
Is v v > 2700 pc/h?		No	
3 or av <sub>34</sub>			
Is v v > 1.5 v /2		No	
3 or av <sub>34</sub> 12			
If yes, v = 1044		(Equation 25-8)	
12A			

Flow Entering Merge Influence Area

---

	Actual	Max Desirable	Violation?
v	1044	4600	No
R12			

Level of Service Determination (if not F)

---

Density, D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 20.2 pc/mi/ln

Level of service for ramp-freeway junction areas of influence C

Speed Estimation

---

Intermediate speed variable, M = 0.328

S

Space mean speed in ramp influence area, S = 50.7 mph

R

Space mean speed in outer lanes, S = N/A mph

0

Space mean speed for all vehicles, S = 50.7 mph

HCS+: Ramps and Ramp Junctions Release 5.4

Phone: Fax:  
E-mail:

Merge Analysis

Analyst: CTRR  
 Agency/Co.: HNTB  
 Date performed: 3/07/2010  
 Analysis time period: Build Service Road Concept  
 Freeway/Dir of Travel: Wekiva Pkwy. WB  
 Junction: On Ramp from SR 46  
 Jurisdiction: Seminole County  
 Analysis Year: 2012  
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	3	
Free-flow speed on freeway	55.0	mph
Volume on freeway	1810	vph

On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	2	
Free-flow speed on ramp	35.0	mph
Volume on ramp	360	vph
Length of first accel/decel lane	800	ft
Length of second accel/decel lane	640	ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent Ramp	320	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	6336	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
	Ramp		
Volume, V (vph)	1810	360	320 vph
Peak-hour factor, PHF	0.90	0.90	0.90



Peak 15-min volume, v <sub>15</sub>	503	100	89	v
Trucks and buses	0	0	0	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, f <sub>HV</sub>	1.000	1.000	1.000	
Driver population factor, f <sub>P</sub>	1.00	1.00	1.00	
Flow rate, v <sub>p</sub>	2011	400	356	pcph

---

Estimation of V12 Merge Areas

---

L = (Equation 25-2 or 25-3)

EQ

P = 0.555 Using Equation 0

FM

v = v (P ) = 1116 pc/h

12 F FM

---

Capacity Checks

---

	Actual	Maximum	LOS F?
v	2411	6750	No
FO			
v v	895 pc/h	(Equation 25-4 or 25-5)	
3 or av <sub>34</sub>			
Is v v > 2700 pc/h?		No	
3 or av <sub>34</sub>			
Is v v > 1.5 v /2		Yes	
3 or av <sub>34</sub> 12			
If yes, v = 1149		(Equation 25-8)	
12A			

---

Flow Entering Merge Influence Area

---

	Actual	Max Desirable	Violation?
v	1149	4600	No
12A			

---

Level of Service Determination (if not F)

---

Density, D = 5.475 + 0.00734 v<sub>R</sub> + 0.0078 v<sub>A</sub> - 0.00627 L = 3.3 pc/mi/ln

Level of service for ramp-freeway junction areas of influence A

---

Speed Estimation

---

Intermediate speed variable, M = 0.183

S

Space mean speed in ramp influence area, S = 52.6 mph

R

Space mean speed in outer lanes, S = 53.7 mph

0

Space mean speed for all vehicles, S = 53.0 mph

HCS+: Ramps and Ramp Junctions Release 5.4

Phone: Fax:  
E-mail:

Merge Analysis

Analyst: CTRR  
 Agency/Co.: HNTB  
 Date performed: 3/07/2010  
 Analysis time period: Build Service Road Concept  
 Freeway/Dir of Travel: Wekiva Pkwy. WB  
 Junction: On Ramp from SR 46  
 Jurisdiction: Seminole County  
 Analysis Year: 2012  
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Merge		
Number of lanes in freeway	3		
Free-flow speed on freeway	55.0	mph	
Volume on freeway	1810	vph	

On Ramp Data

Side of freeway	Right		
Number of lanes in ramp	2		
Free-flow speed on ramp	35.0	mph	
Volume on ramp	360	vph	
Length of first accel/decel lane	800	ft	
Length of second accel/decel lane	640	ft	

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes		
Volume on adjacent Ramp	160	vph	
Position of adjacent Ramp	Downstream		
Type of adjacent Ramp	Off		
Distance to adjacent Ramp	6684	ft	

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
		Ramp		
Volume, V (vph)	1810	360	160	vph
Peak-hour factor, PHF	0.90	0.90	0.90	

Peak 15-min volume, v <sub>15</sub>	503	100	44	v
Trucks and buses	0	0	0	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, f <sub>HV</sub>	1.000	1.000	1.000	
Driver population factor, f <sub>P</sub>	1.00	1.00	1.00	
Flow rate, v <sub>p</sub>	2011	400	178	pcph

Estimation of V12 Merge Areas

---

L = (Equation 25-2 or 25-3)

EQ

P = 0.555 Using Equation 0

FM

v = v (P ) = 1116 pc/h

12 F FM

Capacity Checks

---

	Actual	Maximum	LOS F?
v	2411	6750	No
FO			
v v	895 pc/h	(Equation 25-4 or 25-5)	
3 or av <sub>34</sub>			
Is v v > 2700 pc/h?		No	
3 or av <sub>34</sub>			
Is v v > 1.5 v /2		Yes	
3 or av <sub>34</sub> 12			
If yes, v = 1149		(Equation 25-8)	
12A			

Flow Entering Merge Influence Area

---

	Actual	Max Desirable	Violation?
v	1149	4600	No
12A			

Level of Service Determination (if not F)

---

Density, D = 5.475 + 0.00734 v<sub>R</sub> + 0.0078 v<sub>12</sub> - 0.00627 L<sub>A</sub> = 3.3 pc/mi/ln

Level of service for ramp-freeway junction areas of influence A

Speed Estimation

---

Intermediate speed variable, M = 0.183

S

Space mean speed in ramp influence area, S = 52.6 mph

R

Space mean speed in outer lanes, S = 53.7 mph

0

Space mean speed for all vehicles, S = 53.0 mph

HCS+: Ramps and Ramp Junctions Release 5.4

Phone: Fax:  
E-mail:

Diverge Analysis

Analyst: CTRR  
 Agency/Co.: HNTB  
 Date performed: 3/07/2010  
 Analysis time period: Build Service Road Concept  
 Freeway/Dir of Travel: Wekiva Pkwy. WB  
 Junction: Off Ramp to WB CD  
 Jurisdiction: Seminole County  
 Analysis Year: 2012  
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Diverge		
Number of lanes in freeway	3		
Free-flow speed on freeway	55.0	mph	
Volume on freeway	2170	vph	

Off Ramp Data

Side of freeway	Right		
Number of lanes in ramp	1		
Free-Flow speed on ramp	35.0	mph	
Volume on ramp	160	vph	
Length of first accel/decel lane	1340	ft	
Length of second accel/decel lane		ft	

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes		
Volume on adjacent ramp	360	vph	
Position of adjacent ramp	Upstream		
Type of adjacent ramp	On		
Distance to adjacent ramp	6684	ft	

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
	Ramp			
Volume, V (vph)	2170	160	360	vph
Peak-hour factor, PHF	0.90	0.90	0.90	

Peak 15-min volume, v <sub>15</sub>	603	44	100	v
Trucks and buses	0	0	0	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00 %	0.00 %	0.00 %	
Length	0.00 mi	0.00 mi	0.00 mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, f <sub>HV</sub>	1.000	1.000	1.000	
Driver population factor, f <sub>P</sub>	1.00	1.00	1.00	
Flow rate, v <sub>p</sub>	2411	178	400	pcph

Estimation of V12 Diverge Areas

---

L = 3542.17 (Equation 25-8 or 25-9)

EQ

P = 0.692 Using Equation 5

FD

$v = v + (v - v) P = 1722$  pc/h

12 R F R FD

Capacity Checks

---

	Actual	Maximum	LOS F?
v = v	2411	6750	No
F <sub>i</sub> F			
v = v - v	2233	6750	No
F <sub>O</sub> F R			
v	178	2000	No
R			
v v	689 pc/h	(Equation 25-15 or 25-16)	
3 or av <sub>34</sub>			
Is v v > 2700 pc/h?		No	
3 or av <sub>34</sub>			
Is v v > 1.5 v /2		No	
3 or av <sub>34</sub> 12			
If yes, v = 1722		(Equation 25-18)	
12A			

Flow Entering Diverge Influence Area

---

	Actual	Max Desirable	Violation?
v	1722	4400	No
12			

Level of Service Determination (if not F)

---

Density,  $D = 4.252 + 0.0086 v - 0.009 L = 7.0$  pc/mi/ln

R 12 D

Level of service for ramp-freeway junction areas of influence A

Speed Estimation

---

Intermediate speed variable,  $D = 0.444$

S

Space mean speed in ramp influence area,  $S = 49.2$  mph

Space mean speed in outer lanes,	$\frac{R}{0}$	S = 60.3	mph
Space mean speed for all vehicles,		S = 52.0	mph

---

HCS+: Ramps and Ramp Junctions Release 5.4

Phone: Fax:  
E-mail:

Diverge Analysis

Analyst: CTRR  
Agency/Co.: HNTB  
Date performed: 03/07/2010  
Analysis time period: Build Service Road Concept  
Freeway/Dir of Travel: Wekiva Pkwy. WB  
Junction: Off Ramp to WB CD  
Jurisdiction: Seminole County  
Analysis Year: 2012  
Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Diverge		
Number of lanes in freeway	3		
Free-flow speed on freeway	55.0	mph	
Volume on freeway	2170	vph	

Off Ramp Data

Side of freeway	Right		
Number of lanes in ramp	1		
Free-Flow speed on ramp	35.0	mph	
Volume on ramp	160	vph	
Length of first accel/decel lane	1340	ft	
Length of second accel/decel lane		ft	

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes		
Volume on adjacent ramp	20	vph	
Position of adjacent ramp	Downstream		
Type of adjacent ramp	On		
Distance to adjacent ramp	6336	ft	

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
		Ramp		
Volume, V (vph)	2170	160	20	vph
Peak-hour factor, PHF	0.90	0.90	0.90	

Peak 15-min volume, v <sub>15</sub>	603	44	6	v
Trucks and buses	0	0	0	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00 %	0.00 %	0.00 %	
Length	0.00 mi	0.00 mi	0.00 mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, f <sub>HV</sub>	1.000	1.000	1.000	
Driver population factor, f <sub>P</sub>	1.00	1.00	1.00	
Flow rate, v <sub>p</sub>	2411	178	22	pcph

---

Estimation of V<sub>12</sub> Diverge Areas

---

L = (Equation 25-8 or 25-9)

EQ

P = 0.692 Using Equation 5

FD

$v = v + (v - v) P = 1722$  pc/h

12 R F R FD

---

Capacity Checks

---

	Actual	Maximum	LOS F?
v = v	2411	6750	No
F <sub>i</sub> F			
v = v - v	2233	6750	No
F <sub>O</sub> F R			
v	178	2000	No
R			
v v	689 pc/h	(Equation 25-15 or 25-16)	
3 or av <sup>34</sup>			
Is v v > 2700 pc/h?		No	
3 or av <sup>34</sup>			
Is v v > 1.5 v /2		No	
3 or av <sup>34</sup> 12			
If yes, v = 1722		(Equation 25-18)	
12A			

---

Flow Entering Diverge Influence Area

---

	Actual	Max Desirable	Violation?
v	1722	4400	No
12			

---

Level of Service Determination (if not F)

---

Density,  $D = 4.252 + 0.0086 v - 0.009 L = 7.0$  pc/mi/ln  
R 12 D

Level of service for ramp-freeway junction areas of influence A

---

Speed Estimation

---

Intermediate speed variable,  $D = 0.444$   
S

Space mean speed in ramp influence area,  $S = 49.2$  mph



	R	
Space mean speed in outer lanes,		S = 60.3 mph
	0	
Space mean speed for all vehicles,		S = 52.0 mph

---

Phone: Fax:  
 E-mail:

Merge Analysis

Analyst: CTR  
 Agency/Co.: HNTB  
 Date performed: 08/02/2010  
 Analysis time period: Build  
 Freeway/Dir of Travel: I-4 EB  
 Junction: On Ramp from US 1792  
 Jurisdiction: Seminole County  
 Analysis Year: 2012  
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	3	
Free-flow speed on freeway	70.0	mph
Volume on freeway	4990 ✓	vph

On Ramp Data

Side of freeway	Right ✓	
Number of lanes in ramp	1 ✓	
Free-flow speed on ramp	35.0	mph
Volume on ramp	360 ✓	vph
Length of first accel/decel lane	900	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent Ramp	660 ✓	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	Off	
Distance to adjacent Ramp	1948	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	4990 ✓	360	660	vph
Peak-hour factor, PHF	0.92 ✓	0.92	0.92	
Peak 15-min volume, v15	1356 ✓	98	179	v
Trucks and buses	9 ✓	9	9	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	%
Length	mi	mi	mi	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.957	0.957	0.957	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	5668	409	750	pcph

Estimation of V12 Merge Areas

$$L = 1128.28 \text{ (Equation 25-2 or 25-3)}$$

$$EQ$$

$$P = 0.603 \text{ Using Equation 1}$$

$$FM$$

$$v_{12} = v_F (P_{FM}) = 3416 \text{ pc/h}$$

Capacity Checks

	Actual	Maximum	LOS F?
v <sub>FO</sub>	6077	7200	No
v <sub>3 or av34</sub>	2252 pc/h	(Equation 25-4 or 25-5)	
Is v <sub>3 or av34</sub> > 2700 pc/h?		No	
Is v <sub>3 or av34</sub> > 1.5 v <sub>12</sub> / 2		No	
If yes, v <sub>12A</sub> = 3416		(Equation 25-8)	

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v <sub>R12</sub>	3416	4600	No

Level of Service Determination (if not F)

$$\text{Density, } D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 29.5 \text{ pc/mi/ln}$$

Level of service for ramp-freeway junction areas of influence D ✓

Speed Estimation

Intermediate speed variable,	M = 0.437	
Space mean speed in ramp influence area,	S = 57.8	mph
Space mean speed in outer lanes,	S = 63.7	mph
Space mean speed for all vehicles,	S = 59.8	mph

Phone: Fax:  
E-mail:

Diverge Analysis

Analyst: CTR  
 Agency/Co.: HNTB  
 Date performed: 8/02/2010  
 Analysis time period: Build  
 Freeway/Dir of Travel: I-4 WB  
 Junction: Off Ramp to US 1792  
 Jurisdiction: Seminole County  
 Analysis Year: 2012  
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	3	
Free-flow speed on freeway	70.0	mph
Volume on freeway	5350	vph

Off Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	35.0	mph
Volume on ramp	360	vph
Length of first accel/decel lane	500	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent ramp	660	vph
Position of adjacent ramp	Downstream	
Type of adjacent ramp	On	
Distance to adjacent ramp	1948	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	5350	360	660	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	1454	98	179	v
Trucks and buses	9	9	9	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00 %	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.957	0.957	0.957	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	6077	409	750	pcph

Estimation of V12 Diverge Areas

---

$$L = \frac{EQ}{P} \quad (\text{Equation 25-8 or 25-9})$$

$$P = 0.589 \quad \text{Using Equation 5}$$

$$v_{12} = v_R + (v_F - v_R) P = 3749 \quad \text{pc/h}$$

Capacity Checks

---

	Actual	Maximum	LOS F?
$v_{Fi} = v_F$	6077	7200	No
$v_{FO} = v_F - v_R$	5668	7200	No
$v_R$	409	2000	No
$v_{3 \text{ or } 34}$	2328 pc/h	(Equation 25-15 or 25-16)	
Is $v_{3 \text{ or } 34} > 2700$ pc/h?		No	
Is $v_{3 \text{ or } 34} > 1.5 v_{12} / 2$		No	
If yes, $v_{12A} = 3749$		(Equation 25-18)	

Flow Entering Diverge Influence Area

---

	Actual	Max Desirable	Violation?
$v_{12}$	3749	4400	No

Level of Service Determination (if not F)

---

Density,  $D = 4.252 + 0.0086 v_{12} - 0.009 L_D = 32.0 \quad \text{pc/mi/ln}$

Level of service for ramp-freeway junction areas of influence D ✓

Speed Estimation

---

Intermediate speed variable,	$D = 0.465$	
Space mean speed in ramp influence area,	$S = 57.0$	mph
Space mean speed in outer lanes,	$S = 71.6$	mph
Space mean speed for all vehicles,	$S = 61.8$	mph

---

HCS+: Ramps and Ramp Junctions Release 5.4

Phone: Fax:  
E-mail:

Merge Analysis

Analyst: CTRR  
 Agency/Co.: HNTB  
 Date performed: 3/07/2010  
 Analysis time period: Build Service Road Concept  
 Freeway/Dir of Travel: I-4 EB  
 Junction: On Ramp from SR 46  
 Jurisdiction: Seminole County  
 Analysis Year: 2012  
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	4	
Free-flow speed on freeway	55.0	mph
Volume on freeway	4650	vph

On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	2	
Free-flow speed on ramp	35.0	mph
Volume on ramp	1000	vph
Length of first accel/decel lane	500	ft
Length of second accel/decel lane	500	ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent Ramp	430	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	3172	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	4650	1000	430	vph
Peak-hour factor, PHF	0.90	0.90	0.90	

Peak 15-min volume, v <sub>15</sub>	1292	278	119	v
Trucks and buses	0	0	0	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, f <sub>HV</sub>	1.000	1.000	1.000	
Driver population factor, f <sub>P</sub>	1.00	1.00	1.00	
Flow rate, v <sub>p</sub>	5167	1111	478	pcph

---

Estimation of V12 Merge Areas

---

L = (Equation 25-2 or 25-3)

EQ

P = 0.209 Using Equation 0

FM

v = v(P) = 1080 pc/h

12 F FM

---

Capacity Checks

---

	Actual	Maximum	LOS F?
v	6278	9000	No
FO			
v v	2043 pc/h	(Equation 25-4 or 25-5)	
3 or av <sub>34</sub>			
Is v v > 2700 pc/h?		No	
3 or av <sub>34</sub>			
Is v v > 1.5 v /2		Yes	
3 or av <sub>34</sub> 12			
If yes, v = 2066		(Equation 25-8)	
12A			

---

Flow Entering Merge Influence Area

---

	Actual	Max Desirable	Violation?
v	2066	4600	No
12A			

---

Level of Service Determination (if not F)

---

Density, D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 20.3 pc/mi/ln

R            R            12            A

Level of service for ramp-freeway junction areas of influence C

---

Speed Estimation

---

Intermediate speed variable, M = 0.310

S

Space mean speed in ramp influence area, S = 51.0 mph

R

Space mean speed in outer lanes, S = 51.2 mph

0

Space mean speed for all vehicles, S = 51.1 mph

Phone:  
E-mail:

Fax:

Diverge Analysis

Analyst: CTR  
 Agency/Co.: HNTB  
 Date performed: 8/2/2010  
 Analysis time period: Build Service Road Concept  
 Freeway/Dir of Travel: I-4 WB  
 Junction: Off Ramp to SR 46  
 Jurisdiction: Seminole County  
 Analysis Year: 2012  
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	4	
Free-flow speed on freeway	55.0 ✓	mph
Volume on freeway	5650 ✓	vph

Off Ramp Data

Side of freeway	Right	
Number of lanes in ramp	2	
Free-Flow speed on ramp	35.0 ✓	mph
Volume on ramp	1000 ✓	vph
Length of first accel/decel lane	0	ft
Length of second accel/decel lane	500	ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent ramp	1700 ✓	vph
Position of adjacent ramp	Downstream	
Type of adjacent ramp	Off	
Distance to adjacent ramp	6098	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	5650 ✓	1000	1700	vph
Peak-hour factor, PHF	0.92 ✓	0.92	0.92	
Peak 15-min volume, v15	1535 ✓	272	462	v
Trucks and buses	9 ✓	9	9	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00 %	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	



Heavy vehicle adjustment, fHV	0.957	0.957	0.957	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	6418	1136	1931	pcph

Estimation of V12 Diverge Areas

---

$$L = \text{(Equation 25-8 or 25-9)}$$

$$EQ$$

$$P = 0.260 \text{ Using Equation 0}$$

$$FD$$

$$v_{12R} = v_R + (v_F - v_R) P = 2509 \text{ pc/h}$$

Capacity Checks

---

	Actual	Maximum	LOS F?
$v_{Fi} = v_F$	6418	9000	No
$v_{FO} = v_F - v_R$	5282	9000	No
$v_R$	1136	3800	No
$v_{3 \text{ or } av34}$	1954 pc/h	(Equation 25-15 or 25-16)	
Is $v_{3 \text{ or } av34} > 2700 \text{ pc/h?}$		No	
Is $v_{3 \text{ or } av34} > 1.5 v_{12} / 2$		Yes	
If yes, $v_{12A} = 2567$		(Equation 25-18)	

Flow Entering Diverge Influence Area

---

	Actual	Max Desirable	Violation?
$v_{12A}$	2567	4400	No

Level of Service Determination (if not F)

---

Density,  $D = 4.252 + 0.0086 v_{12} - 0.009 L_D = 21.8 \text{ pc/mi/ln}$

Level of service for ramp-freeway junction areas of influence C ✓

Speed Estimation

---

Intermediate speed variable,	$D = 0.530$	
Space mean speed in ramp influence area,	$S = 48.1$	mph
Space mean speed in outer lanes,	$S = 56.7$	mph
Space mean speed for all vehicles,	$S = 52.9$	mph

---

HCS+: Ramps and Ramp Junctions Release 5.4

Phone: Fax:  
E-mail:

Diverge Analysis

Analyst: CTRR  
 Agency/Co.: HNTB  
 Date performed: 3/07/2010  
 Analysis time period: Build Service Road Concept  
 Freeway/Dir of Travel: I-4 WB  
 Junction: Off Ramp to SR 417 & CR 46A  
 Jurisdiction: Seminole County  
 Analysis Year: 2012  
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Diverge		
Number of lanes in freeway	4		
Free-flow speed on freeway	55.0	mph	
Volume on freeway	4220	vph	

Off Ramp Data

Side of freeway	Right		
Number of lanes in ramp	2		
Free-Flow speed on ramp	35.0	mph	
Volume on ramp	1690	vph	
Length of first accel/decel lane	500	ft	
Length of second accel/decel lane	500	ft	

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes		
Volume on adjacent ramp	1000	vph	
Position of adjacent ramp	Upstream		
Type of adjacent ramp	Off		
Distance to adjacent ramp	6098	ft	

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
		Ramp		
Volume, V (vph)	4220	1690	1000	vph
Peak-hour factor, PHF	0.90	0.90	0.90	

Peak 15-min volume, v <sub>15</sub>	1172	469	278	v
Trucks and buses	0	0	0	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00 %	0.00 %	0.00 %	
Length	0.00 mi	0.00 mi	0.00 mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, f <sub>HV</sub>	1.000	1.000	1.000	
Driver population factor, f <sub>P</sub>	1.00	1.00	1.00	
Flow rate, v <sub>p</sub>	4689	1878	1111	pcph

---

Estimation of V12 Diverge Areas

---

L = (Equation 25-8 or 25-9)

EQ

P = 0.260 Using Equation 0

FD

$v = v + (v - v) P = 2609$  pc/h

12 R F R FD

---

Capacity Checks

---

	Actual	Maximum	LOS F?
$v = v$	4689	9000	No
F <sub>i</sub> F			
$v = v - v$	2811	9000	No
F <sub>O</sub> F R			
v	1878	3800	No
R			
$v v$	1040 pc/h	(Equation 25-15 or 25-16)	
3 or av34			
Is $v v > 2700$ pc/h?		No	
3 or av34			
Is $v v > 1.5 v / 2$		No	
3 or av34 12			
If yes, $v = 2609$		(Equation 25-18)	
12A			

---

Flow Entering Diverge Influence Area

---

	Actual	Max Desirable	Violation?
v	2609	4400	No
12			

---

Level of Service Determination (if not F)

---

Density,  $D = 4.252 + 0.0086 v - 0.009 L = 13.2$  pc/mi/ln

R 12 D

Level of service for ramp-freeway junction areas of influence B

---

Speed Estimation

---

Intermediate speed variable,  $D = 0.597$

S

Space mean speed in ramp influence area,  $S = 47.2$  mph

	R	
Space mean speed in outer lanes,		S = 60.2 mph
	0	
Space mean speed for all vehicles,		S = 52.2 mph

---

HCS+: Ramps and Ramp Junctions Release 5.4

Phone: Fax:  
E-mail:

Diverge Analysis

Analyst: CTRR  
Agency/Co.: HNTB  
Date performed: 3/07/2010  
Analysis time period: Build Service Road Concept  
Freeway/Dir of Travel: I-4 WB  
Junction: Off Ramp to SR 417 & CR 46A  
Jurisdiction: Seminole County  
Analysis Year: 2012  
Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	4	
Free-flow speed on freeway	55.0	mph
Volume on freeway	4220	vph

Off Ramp Data

Side of freeway	Right	
Number of lanes in ramp	2	
Free-Flow speed on ramp	35.0	mph
Volume on ramp	1690	vph
Length of first accel/decel lane	500	ft
Length of second accel/decel lane	500	ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent ramp	440	vph
Position of adjacent ramp	Downstream	
Type of adjacent ramp	On	
Distance to adjacent ramp	3490	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
		Ramp		
Volume, V (vph)	4220	1690	440	vph
Peak-hour factor, PHF	0.90	0.90	0.90	

Peak 15-min volume, v <sub>15</sub>	1172	469	122	v
Trucks and buses	0	0	0	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00 %	0.00 %	0.00 %	
Length	0.00 mi	0.00 mi	0.00 mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, f <sub>HV</sub>	1.000	1.000	1.000	
Driver population factor, f <sub>P</sub>	1.00	1.00	1.00	
Flow rate, v <sub>p</sub>	4689	1878	489	pcph

Estimation of V12 Diverge Areas

---

L = (Equation 25-8 or 25-9)

EQ

P = 0.260 Using Equation 0

FD

$v = v + (v - v) P = 2609$  pc/h

12 R F R FD

Capacity Checks

---

	Actual	Maximum	LOS F?
v = v	4689	9000	No
F <sub>i</sub> F			
v = v - v	2811	9000	No
F <sub>O</sub> F R			
v	1878	3800	No
R			
v v	1040 pc/h	(Equation 25-15 or 25-16)	
3 or av <sub>34</sub>			
Is v v > 2700 pc/h?		No	
3 or av <sub>34</sub>			
Is v v > 1.5 v /2		No	
3 or av <sub>34</sub> 12			
If yes, v = 2609		(Equation 25-18)	
12A			

Flow Entering Diverge Influence Area

---

	Actual	Max Desirable	Violation?
v	2609	4400	No
12			

Level of Service Determination (if not F)

---

Density,  $D = 4.252 + 0.0086 v - 0.009 L = 13.2$  pc/mi/ln

R 12 D

Level of service for ramp-freeway junction areas of influence B

Speed Estimation

---

Intermediate speed variable,  $D = 0.597$

S

Space mean speed in ramp influence area,  $S = 47.2$  mph

Space mean speed in outer lanes,	$\frac{R}{0}$	S = 60.2 mph
Space mean speed for all vehicles,		S = 52.2 mph

---

Phone: Fax:  
E-mail:

Merge Analysis

Analyst: CTR  
 Agency/Co.: HNTB  
 Date performed: 8/2/2010  
 Analysis time period: Build Service Road Concept  
 Freeway/Dir of Travel: I-4 WB  
 Junction: On Ramp from SR 417  
 Jurisdiction: Seminole County  
 Analysis Year: 2012  
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	3	
Free-flow speed on freeway	55.0	mph
Volume on freeway	2950 ✓	vph

On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	2	
Free-flow speed on ramp	35.0	mph
Volume on ramp	440 ↓	vph
Length of first accel/decel lane	600 ↓	ft
Length of second accel/decel lane	1500 ↓	ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes ↓	
Volume on adjacent Ramp	2140 ↓	vph
Position of adjacent Ramp	Downstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	3654	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	2950 ↓	440	2140	vph
Peak-hour factor, PHF	0.92 ↓	0.92	0.92	
Peak 15-min volume, v15	802 ↓	120	582	v
Trucks and buses	9 ↓	9	9	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	



Heavy vehicle adjustment, fHV	0.957	0.957	0.957	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	3351	500	2431	pcph

Estimation of V12 Merge Areas

$$L = \text{(Equation 25-2 or 25-3)}$$

$$EQ$$

$$P = 0.555 \text{ Using Equation 0}$$

$$FM$$

$$v_{12} = v_F (P_{FM}) = 1860 \text{ pc/h}$$

Capacity Checks

	Actual	Maximum	LOS F?
v <sub>FO</sub>	3851	6750	No
v <sub>3 or av34</sub>	1491 pc/h	(Equation 25-4 or 25-5)	
Is v <sub>3 or av34</sub> > 2700 pc/h?		No	
Is v <sub>3 or av34</sub> > 1.5 v <sub>12</sub> / 2		Yes	
If yes, v <sub>12A</sub> = 1914		(Equation 25-8)	

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v <sub>12A</sub>	1914	4600	No

Level of Service Determination (if not F)

$$\text{Density, } D_R = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 7.1 \text{ pc/mi/ln}$$

Level of service for ramp-freeway junction areas of influence A ✓

Speed Estimation

Intermediate speed variable,	M = 0.176	
Space mean speed in ramp influence area,	S <sub>R</sub> = 52.7	mph
Space mean speed in outer lanes,	S <sub>0</sub> = 51.6	mph
Space mean speed for all vehicles,	S = 52.3	mph

Phone: Fax:  
 E-mail:

Merge Analysis

Analyst: CTR  
 Agency/Co.: HNTB  
 Date performed: 8/2/2010  
 Analysis time period: Build Service Road Concept  
 Freeway/Dir of Travel: I-4 WB  
 Junction: On Ramp from SR 417  
 Jurisdiction: Seminole County  
 Analysis Year: 2012  
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	3	
Free-flow speed on freeway	55.0	mph
Volume on freeway	2950 ✓	vph

On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	2	
Free-flow speed on ramp	35.0 ✓	mph
Volume on ramp	440 ✓	vph
Length of first accel/decel lane	600 ✓	ft
Length of second accel/decel lane	1500 ✓	ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes ✓	
Volume on adjacent Ramp	1700 ✓	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	Off	
Distance to adjacent Ramp	3490	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	2950	440	1700	vph
Peak-hour factor, PHF	0.92 ✓	0.92	0.92	
Peak 15-min volume, v15	802 ✓	120	462	v
Trucks and buses	9 ✓	9	9	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade		%	%	%
Length		mi	mi	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.957	0.957	0.957	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	3351	500	1931	pcph

Estimation of V12 Merge Areas

$$L = \text{EQ} \quad (\text{Equation 25-2 or 25-3})$$

$$P = 0.555 \quad \text{Using Equation 0}$$

$$v_{12} = v_{F \text{ FM}} (P) = 1860 \quad \text{pc/h}$$

Capacity Checks

	Actual	Maximum	LOS F?
v <sub>FO</sub>	3851	6750	No
v <sub>3 or av34</sub>	1491 pc/h	(Equation 25-4 or 25-5)	
Is v <sub>3 or av34</sub> > 2700 pc/h?		No	
Is v <sub>3 or av34</sub> > 1.5 v <sub>12</sub> / 2		Yes	
If yes, v <sub>12A</sub> = 1914		(Equation 25-8)	

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v <sub>12A</sub>	1914	4600	No

Level of Service Determination (if not F)

$$\text{Density, } D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 7.1 \quad \text{pc/mi/ln}$$

Level of service for ramp-freeway junction areas of influence A ✓

Speed Estimation

Intermediate speed variable,	M = 0.176	
Space mean speed in ramp influence area,	S = 52.7	mph
Space mean speed in outer lanes,	S = 51.6	mph
Space mean speed for all vehicles,	S = 52.3	mph

HCS+: Ramps and Ramp Junctions Release 5.4

Phone: Fax:  
E-mail:

Merge Analysis

Analyst: CTRR  
 Agency/Co.: HNTB  
 Date performed: 3/07/2010  
 Analysis time period: Build Service Road Concept  
 Freeway/Dir of Travel: I-4 WB  
 Junction: On Ramp from SR 46 & CR 46A  
 Jurisdiction: Seminole County  
 Analysis Year: 2012  
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	3	
Free-flow speed on freeway	55.0	mph
Volume on freeway	2970	vph

On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	2	
Free-flow speed on ramp	35.0	mph
Volume on ramp	2130	vph
Length of first accel/decel lane	900	ft
Length of second accel/decel lane	900	ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent Ramp	440	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	3654	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	2970	2130	440	vph
Peak-hour factor, PHF	0.90	0.90	0.90	

Peak 15-min volume, v <sub>15</sub>	825	592	122	v
Trucks and buses	0	0	0	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, f <sub>HV</sub>	1.000	1.000	1.000	
Driver population factor, f <sub>P</sub>	1.00	1.00	1.00	
Flow rate, v <sub>p</sub>	3300	2367	489	pcph

Estimation of V12 Merge Areas

---

L = (Equation 25-2 or 25-3)

EQ

P = 0.555 Using Equation 0

FM

v = v (P ) = 1832 pc/h

12 F FM

Capacity Checks

---

	Actual	Maximum	LOS F?
v	5667	6750	No
FO			
v v	1468 pc/h	(Equation 25-4 or 25-5)	
3 or av <sub>34</sub>			
Is v v > 2700 pc/h?		No	
3 or av <sub>34</sub>			
Is v v > 1.5 v /2		Yes	
3 or av <sub>34</sub> 12			
If yes, v = 1885		(Equation 25-8)	
12A			

Flow Entering Merge Influence Area

---

	Actual	Max Desirable	Violation?
v	1885	4600	No
12A			

Level of Service Determination (if not F)

---

Density, D = 5.475 + 0.00734 v<sub>R</sub> + 0.0078 v<sub>12</sub> - 0.00627 L<sub>A</sub> = 20.6 pc/mi/ln

Level of service for ramp-freeway junction areas of influence C

Speed Estimation

---

Intermediate speed variable, M = 0.406

S

Space mean speed in ramp influence area, S = 49.7 mph

R

Space mean speed in outer lanes, S = 51.7 mph

0

Space mean speed for all vehicles, S = 50.2 mph

Phone:  
E-mail:

Fax:

Merge Analysis

Analyst: CTR  
 Agency/Co.: HNTB  
 Date performed: 8/2/2010  
 Analysis time period: Build Service Road Concept  
 Freeway/Dir of Travel: CD Rd (West of I-4) WB  
 Junction: On Ramp from CR 46A to CD Rd  
 Jurisdiction: Seminole County  
 Analysis Year: 2012  
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	2	
Free-flow speed on freeway	55.0	mph
Volume on freeway	950 ✓	vph

On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	35.0	mph
Volume on ramp	1190 ✓	vph
Length of first accel/decel lane	600	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent Ramp	470 ✓	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	Off	
Distance to adjacent Ramp	1320	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	950 ✓	1190	470	vph
Peak-hour factor, PHF	0.92 ✓	0.92	0.92	
Peak 15-min volume, v15	258	323	128	v
Trucks and buses	9 ✓	9	9	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.957	0.957	0.957	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	1079	1352	534	pcph

---

Estimation of V12 Merge Areas

---

L = (Equation 25-2 or 25-3)  
EQ  
P = 1.000 Using Equation 0  
FM  
 $v_{12} = v_F (P_{FM}) = 1079 \text{ pc/h}$

---

Capacity Checks

---

	Actual	Maximum	LOS F?
v <sub>FO</sub>	2431	4500	No
v <sub>3 or av34</sub>	0 pc/h	(Equation 25-4 or 25-5)	
Is v <sub>3 or av34</sub> > 2700 pc/h?		No	
Is v <sub>3 or av34</sub> > 1.5 v <sub>12</sub> / 2		No	
If yes, v <sub>12A</sub> = 1079		(Equation 25-8)	

---

Flow Entering Merge Influence Area

---

	Actual	Max Desirable	Violation?
v <sub>R12</sub>	1079	4600	No

---

Level of Service Determination (if not F)

---

Density,  $D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 20.1 \text{ pc/mi/ln}$   
Level of service for ramp-freeway junction areas of influence C ✓

---

Speed Estimation

---

Intermediate speed variable,	M = 0.323	
Space mean speed in ramp influence area,	S = 50.8	mph
Space mean speed in outer lanes,	S = N/A	mph
Space mean speed for all vehicles,	S = 50.8	mph

---

Phone: Fax:  
 E-mail:

Diverge Analysis

Analyst: CTR  
 Agency/Co.: HNTB  
 Date performed: 8/02/2010  
 Analysis time period: Build Service Road Concept  
 Freeway/Dir of Travel: CD Rd (East of I-4) EB  
 Junction: Off Ramp to SR 417 EB  
 Jurisdiction: Seminole County  
 Analysis Year: 2012  
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	2	
Free-flow speed on freeway	55.0	mph
Volume on freeway	1390 ✓	vph

Off Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	35.0	mph
Volume on ramp	350 ✓	vph
Length of first accel/decel lane	0	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes ✓	
Volume on adjacent ramp	90 ✓	vph
Position of adjacent ramp	Downstream	
Type of adjacent ramp	Off	
Distance to adjacent ramp	1584	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	1390 ↓	350	90	vph
Peak-hour factor, PHF	0.92 ↓	0.92	0.92	
Peak 15-min volume, v15	378 ↓	95	24	v
Trucks and buses	9 ✓	9	9	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00 %	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	



Heavy vehicle adjustment, fHV	0.957	0.957	0.957	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	1579	398	102	pcph

---

Estimation of V12 Diverge Areas

---

$$L = \text{EQ} \quad (\text{Equation 25-8 or 25-9})$$

$$P = 1.000 \quad \text{Using Equation 0}$$

$$v_{12R} = v_{FR} + (v_{FR} - v_{FD}) P = 1579 \quad \text{pc/h}$$

---

Capacity Checks

---

	Actual	Maximum	LOS F?
$v_{Fi} = v_F$	1579	4500	No
$v_{FO} = v_F - v_R$	1181	4500	No
$v_R$	398	2000	No
$v_{3 \text{ or } 34}$	0 pc/h	(Equation 25-15 or 25-16)	
Is $v_{3 \text{ or } 34} > 2700$ pc/h?		No	
Is $v_{3 \text{ or } 34} > 1.5 v_{12} / 2$		No	
If yes, $v_{12A} = 1579$		(Equation 25-18)	

---

Flow Entering Diverge Influence Area

---

	Actual	Max Desirable	Violation?
$v_{12}$	1579	4400	No

---

Level of Service Determination (if not F)

---

Density,  $D = 4.252 + 0.0086 v_{12} - 0.009 L_D = 17.8$  pc/mi/ln

Level of service for ramp-freeway junction areas of influence B ✓

---

Speed Estimation

---

Intermediate speed variable,	$D = 0.464$	
Space mean speed in ramp influence area,	$S = 49.0$	mph
Space mean speed in outer lanes,	$S = \text{N/A}$	mph
Space mean speed for all vehicles,	$S = 49.0$	mph

---

Phone: Fax:  
E-mail:

Diverge Analysis

Analyst: CTR  
 Agency/Co.: HNTB  
 Date performed: 8/02/2010  
 Analysis time period: Build Service Road Concept  
 Freeway/Dir of Travel: CD Rd (East of I-4) EB  
 Junction: Off Ramp to SR 417 WB  
 Jurisdiction: Seminole County  
 Analysis Year: 2012  
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	2	
Free-flow speed on freeway	55.0	mph
Volume on freeway	1040 ✓	vph

Off Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	35.0 ✓	mph
Volume on ramp	90 ✓	vph
Length of first accel/decel lane	0	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent ramp	350 ✓	vph
Position of adjacent ramp	Upstream	
Type of adjacent ramp	Off	
Distance to adjacent ramp	1584	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	1040 ✓	90	350	vph
Peak-hour factor, PHF	0.92 ✓	0.92	0.92	
Peak 15-min volume, v15	283 ✓	24	95	v
Trucks and buses	9 ✓	9	9	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00 %	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.957	0.957	0.957	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	1181	102	398	pcph

Estimation of V12 Diverge Areas

$$L = \frac{EQ}{P} \quad (\text{Equation 25-8 or 25-9})$$

$$P = 1.000 \quad \text{Using Equation 0}$$

$$v_{12} = v_R + (v_F - v_R) P = 1181 \quad \text{pc/h}$$

Capacity Checks

	Actual	Maximum	LOS F?
$v_{Fi} = v_F$	1181	4500	No
$v_{FO} = v_F - v_R$	1079	4500	No
$v_R$	102	2000	No
$v_{3 \text{ or } av34}$	0 pc/h	(Equation 25-15 or 25-16)	
Is $v_{3 \text{ or } av34} > 2700$ pc/h?		No	
Is $v_{3 \text{ or } av34} > 1.5 v_{12} / 2$		No	
If yes, $v_{12A} = 1181$		(Equation 25-18)	

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
$v_{12}$	1181	4400	No

Level of Service Determination (if not F)

Density,  $D = 4.252 + 0.0086 v_{12} - 0.009 L_D = 14.4$  pc/mi/ln

Level of service for ramp-freeway junction areas of influence B ✓

Speed Estimation

Intermediate speed variable,	$D = 0.437$	
Space mean speed in ramp influence area,	$S = 49.3$	mph
Space mean speed in outer lanes,	$S = N/A$	mph
Space mean speed for all vehicles,	$S = 49.3$	mph

Phone: \_\_\_\_\_ Fax: \_\_\_\_\_  
 E-mail: \_\_\_\_\_

Diverge Analysis

Analyst: CTR  
 Agency/Co.: HNTB  
 Date performed: 8/02/2010  
 Analysis time period: Build Service Road Concept  
 Freeway/Dir of Travel: CD Rd (East of I-4) EB  
 Junction: Off Ramp to SR 417 WB  
 Jurisdiction: Seminole County  
 Analysis Year: 2012  
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Diverge		
Number of lanes in freeway	2		
Free-flow speed on freeway	55.0		mph
Volume on freeway	1040	✓	vph

Off Ramp Data

Side of freeway	Right		
Number of lanes in ramp	1		
Free-Flow speed on ramp	35.0		mph
Volume on ramp	90	✓	vph
Length of first accel/decel lane	0		ft
Length of second accel/decel lane			ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes		
Volume on adjacent ramp	430	✓	vph
Position of adjacent ramp	Downstream		
Type of adjacent ramp	On		
Distance to adjacent ramp	1478		ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	1040	90	430	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	283	24	117	v
Trucks and buses	9	9	9	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00 %	0.00 %	0.00 %	
Length	0.00 mi	0.00 mi	0.00 mi	
Trucks and buses PCE, ET	1.5*	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.957	0.957	0.957	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	1181	102	488	pcph

Estimation of V12 Diverge Areas

$$L = \text{EQ} \quad (\text{Equation 25-8 or 25-9})$$

$$P = 1.000 \quad \text{Using Equation 0}$$

$$v_{12R} = v_{FR} + (v_{FR} - v_{FD}) P = 1181 \quad \text{pc/h}$$

Capacity Checks

	Actual	Maximum	LOS F?
$v_{Fi} = v_F$	1181	4500	No
$v_{FO} = v_F - v_R$	1079	4500	No
$v_R$	102	2000	No
$v_3 \text{ or } v_{av34}$	0 pc/h	(Equation 25-15 or 25-16)	
Is $v_3 \text{ or } v_{av34} > 2700 \text{ pc/h?}$		No	
Is $v_3 \text{ or } v_{av34} > 1.5 v_{12} / 2$		No	
If yes, $v_{12A} = 1181$		(Equation 25-18)	

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
$v_{12}$	1181	4400	No

Level of Service Determination (if not F)

Density,  $D = 4.252 + 0.0086 v_{12} - 0.009 L_D = 14.4 \text{ pc/mi/ln}$

Level of service for ramp-freeway junction areas of influence B ✓

Speed Estimation

Intermediate speed variable,	$D = 0.437$	
Space mean speed in ramp influence area,	$S_R = 49.3$	mph
Space mean speed in outer lanes,	$S_0 = \text{N/A}$	mph
Space mean speed for all vehicles,	$S = 49.3$	mph

HCS+: Ramps and Ramp Junctions Release 5.4

Phone: Fax:  
E-mail:

Merge Analysis

Analyst: CTRR  
Agency/Co.: HNTB  
Date performed: 3/07/2010  
Analysis time period: Build Service Road Concept  
Freeway/Dir of Travel: CD Rd (East of I-4) EB  
Junction: On Ramp from SR 417 EB  
Jurisdiction: Seminole County  
Analysis Year: 2012  
Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Merge		
Number of lanes in freeway	2		
Free-flow speed on freeway	55.0	mph	
Volume on freeway	950	vph	

On Ramp Data

Side of freeway	Left		
Number of lanes in ramp	2		
Free-flow speed on ramp	35.0	mph	
Volume on ramp	430	vph	
Length of first accel/decel lane	500	ft	
Length of second accel/decel lane	500	ft	

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes		
Volume on adjacent Ramp	90	vph	
Position of adjacent Ramp	Upstream		
Type of adjacent Ramp	Off		
Distance to adjacent Ramp	1478	ft	

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
Volume, V (vph)	950	430	90	vph
Peak-hour factor, PHF	0.90	0.90	0.90	

Peak 15-min volume, v <sub>15</sub>	264	119	25	v
Trucks and buses	0	0	0	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, f <sub>HV</sub>	1.000	1.000	1.000	
Driver population factor, f <sub>P</sub>	1.00	1.00	1.00	
Flow rate, v <sub>p</sub>	1056	478	100	pcph

---

Estimation of V12 Merge Areas

---

L = (Equation 25-2 or 25-3)

EQ

P = 1.000 Using Equation 0

FM

v = v (P ) = 1056 pc/h

12 F FM

---

Capacity Checks

---

	Actual	Maximum	LOS F?
v	1534	4500	No
FO			
v v	0 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		No	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34	12		
If yes, v = 1056		(Equation 25-8)	
12A			

---

Flow Entering Merge Influence Area

---

	Actual	Max Desirable	Violation?
v	1056	4600	No
R12			

---

Level of Service Determination (if not F)

---

Density, D = 5.475 + 0.00734 v<sub>R</sub> + 0.0078 v<sub>12</sub> - 0.00627 L<sub>A</sub> = 7.8 pc/mi/ln

Level of service for ramp-freeway junction areas of influence A

---

Speed Estimation

---

Intermediate speed variable, M = 0.237

S

Space mean speed in ramp influence area, S = 51.9 mph

R

Space mean speed in outer lanes, S = N/A mph

0

Space mean speed for all vehicles, S = 51.9 mph

HCS+: Ramps and Ramp Junctions Release 5.4

Phone: Fax:  
E-mail:

Merge Analysis

Analyst: CTRR  
 Agency/Co.: HNTB  
 Date performed: 3/07/2010  
 Analysis time period: Build Service Road Concept  
 Freeway/Dir of Travel: CD Rd (East of I-4) EB  
 Junction: On Ramp from SR 417 EB  
 Jurisdiction: Seminole County  
 Analysis Year: 2012  
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Merge		
Number of lanes in freeway	2		
Free-flow speed on freeway	55.0	mph	
Volume on freeway	950	vph	

On Ramp Data

Side of freeway	Left		
Number of lanes in ramp	2		
Free-flow speed on ramp	35.0	mph	
Volume on ramp	430	vph	
Length of first accel/decel lane	500	ft	
Length of second accel/decel lane	500	ft	

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes		
Volume on adjacent Ramp	430	vph	
Position of adjacent Ramp	Downstream		
Type of adjacent Ramp	Off		
Distance to adjacent Ramp	3010	ft	

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
	Ramp			
Volume, V (vph)	950	430	430	vph
Peak-hour factor, PHF	0.90	0.90	0.90	



Peak 15-min volume, v <sub>15</sub>	264	119	119	v
Trucks and buses	0	0	0	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, f <sub>HV</sub>	1.000	1.000	1.000	
Driver population factor, f <sub>P</sub>	1.00	1.00	1.00	
Flow rate, v <sub>p</sub>	1056	478	478	pcph

---

Estimation of V12 Merge Areas

---

L = (Equation 25-2 or 25-3)

EQ

P = 1.000 Using Equation 0

FM

v = v (P) = 1056 pc/h

12 F FM

---

Capacity Checks

---

	Actual	Maximum	LOS F?
v	1534	4500	No
FO			
v v	0 pc/h	(Equation 25-4 or 25-5)	
3 or av <sub>34</sub>			
Is v v > 2700 pc/h?		No	
3 or av <sub>34</sub>			
Is v v > 1.5 v /2		No	
3 or av <sub>34</sub>	12		
If yes, v = 1056		(Equation 25-8)	
12A			

---

Flow Entering Merge Influence Area

---

	Actual	Max Desirable	Violation?
v	1056	4600	No
R12			

---

Level of Service Determination (if not F)

---

Density, D = 5.475 + 0.00734 v<sub>R</sub> + 0.0078 v<sub>A</sub> - 0.00627 L = 7.8 pc/mi/ln

Level of service for ramp-freeway junction areas of influence A

---

Speed Estimation

---

Intermediate speed variable, M = 0.237

S

Space mean speed in ramp influence area, S = 51.9 mph

R

Space mean speed in outer lanes, S = N/A mph

0

Space mean speed for all vehicles, S = 51.9 mph

HCS+: Ramps and Ramp Junctions Release 5.4

Phone: Fax:  
E-mail:

Diverge Analysis

Analyst: CTRR  
 Agency/Co.: HNTB  
 Date performed: 3/07/2010  
 Analysis time period: Build Service Road Concept  
 Freeway/Dir of Travel: CD Rd (East of I-4) EB  
 Junction: Off Ramp to I-4 EB  
 Jurisdiction: Seminole County  
 Analysis Year: 2012  
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	3	
Free-flow speed on freeway	55.0	mph
Volume on freeway	1380	vph

Off Ramp Data

Side of freeway	Left	
Number of lanes in ramp	2	
Free-Flow speed on ramp	35.0	mph
Volume on ramp	430	vph
Length of first accel/decel lane	500	ft
Length of second accel/decel lane	500	ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent ramp	430	vph
Position of adjacent ramp	Upstream	
Type of adjacent ramp	On	
Distance to adjacent ramp	3010	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	1380	430	430	vph
Peak-hour factor, PHF	0.90	0.90	0.90	

Peak 15-min volume, v <sub>15</sub>	383	119	119	v
Trucks and buses	0	0	0	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00 %	0.00 %	0.00 %	
Length	0.00 mi	0.00 mi	0.00 mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, f <sub>HV</sub>	1.000	1.000	1.000	
Driver population factor, f <sub>P</sub>	1.00	1.00	1.00	
Flow rate, v <sub>p</sub>	1533	478	478	pcph

Estimation of V12 Diverge Areas

---

L = (Equation 25-8 or 25-9)

EQ

P = 0.450 Using Equation 0

FD

v = v + (v - v) P = 953 pc/h

12 R F R FD

Capacity Checks

---

	Actual	Maximum	LOS F?
v = v	1533	6750	No
F <sub>i</sub> F			
v = v - v	1055	6750	No
F <sub>O</sub> F R			
v	478	3800	No
R			
v v	580 pc/h	(Equation 25-15 or 25-16)	
3 or av <sub>34</sub>			
Is v v > 2700 pc/h?		No	
3 or av <sub>34</sub>			
Is v v > 1.5 v /2		No	
3 or av <sub>34</sub> 12			
If yes, v = 953		(Equation 25-18)	
12A			

Flow Entering Diverge Influence Area

---

	Actual	Max Desirable	Violation?
v	953	4400	No
12			

Level of Service Determination (if not F)

---

Density, D = 4.252 + 0.0086 v - 0.009 L = -0.6 pc/mi/ln

R 12 D

Level of service for ramp-freeway junction areas of influence A

Speed Estimation

---

Intermediate speed variable, D = 0.471

S

Space mean speed in ramp influence area, S = 48.9 mph

Space mean speed in outer lanes,	$S = 60.3$	mph
Space mean speed for all vehicles,	$S = 52.3$	mph

---