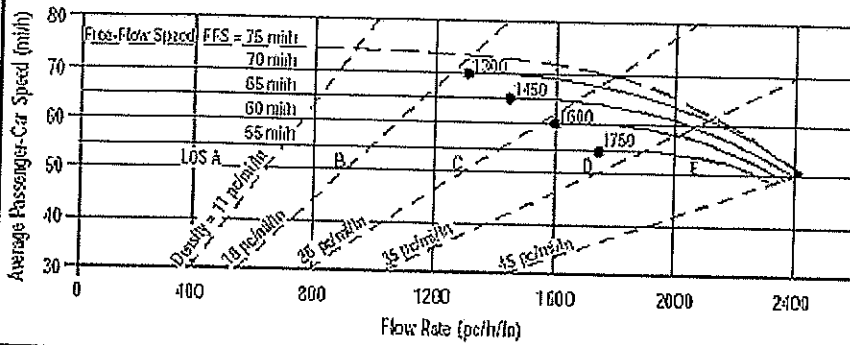


2022 Build - Preferred Alternative with Service Road

BASIC FREEWAY SEGMENTS WORKSHEET



Application	Input	Output
Operational (LOS)	FFS, N, v_p	LOS, S, D
Design (N)	FFS, LOS, v_p	N, S, D
Design (v_p)	FFS, LOS, N	v_p , S, D
Planning (LOS)	FFS, N, AADT	LOS, S, D
Planning (ft)	FFS, LOS, AADT	N, S, D
Planning (v_p)	FFS, LOS, N	v_p , S, D

General Information

Analyst: *KNM*
 Agency or Company: *HNTB*
 Date Performed: *3/25/2008*
 Analysis Time Period: *Peak*

Site Information

Highway/Direction of Travel: *I-4/Eastbound*
 From/To: *Lake Mary Blvd /CR*
 Jurisdiction: *46A/SR417*
 Analysis Year: *2022 Build*

Project Description: *Wekiva Parkway PD&E*

Oper.(LOS)

Des.(N)

Planning Data

Flow Inputs

Volume, V	<i>6000</i>	veh/h	Peak-Hour Factor, PHF	<i>0.95</i>
AADT		veh/day	% Trucks and Buses, P_T	<i>9</i>
Peak-Hr Prop. of AADT, K			% RVs, P_R	<i>0</i>
Peak-Hr Direction Prop, D			General Terrain:	<i>Level</i>
DDHV = AADT x K x D		veh/h	Grade %	<i>mi</i>
Driver type adjustment	<i>1.00</i>		Up/Down %	

Calculate Flow Adjustments

f_p	<i>1.00</i>	E_R	<i>1.2</i>
E_T	<i>1.5</i>	$f_{HV} = 1/[1+P_T(E_T - 1) + P_R(E_R - 1)]$	<i>0.957</i>

Speed Inputs

Calc Speed Adj and FFS

Lane Width	<i>12.0</i>	ft	f_{LW}	<i>0.0</i>	mi/h
Rt-Shoulder Lat. Clearance	<i>6.0</i>	ft	f_{LC}	<i>0.0</i>	mi/h
Interchange Density	<i>0.45</i>	l/mi	f_{ID}	<i>0.0</i>	mi/h
Number of Lanes, N	<i>3</i>		f_N	<i>3.0</i>	mi/h
FFS (measured)		mi/h	FFS	<i>67.0</i>	mi/h
Base free-flow Speed, BFFS	<i>70.0</i>	mi/h			

LOS and Performance Measures

Design (N)

Operational (LOS)			Design (N)	
$v_p = (V \text{ or } DDHV) / (PHF \times N \times f_{HV} \times f_p)$	<i>2200</i>	pc/h/ln	Design LOS	
S	<i>58.3</i>	mi/h	$v_p = (V \text{ or } DDHV) / (PHF \times N \times f_{HV} \times f_p)$	pc/h
$D = v_p / S$	<i>37.8</i>	pc/mi/ln	S	mi/h
LOS	<i>E</i>		$D = v_p / S$	pc/mi/ln
			Required Number of Lanes, N	

Glossary

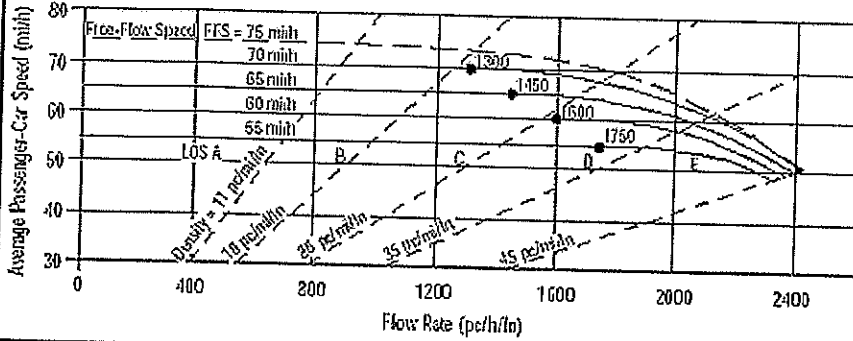
Factor Location

N - Number of lanes
 V - Hourly volume
 v_p - Flow rate
 LOS - Level of service
 DDHV - Directional design hour volume

S - Speed
 D - Density
 FFS - Free-flow speed
 BFFS - Base free-flow speed

E_R - Exhibits 23-8, 23-10
 E_T - Exhibits 23-8, 23-10, 23-11
 f_p - Page 23-12
 LOS, S, FFS, v_p - Exhibits 23-2, 23-3
 f_{LW} - Exhibit 23-4
 f_{LC} - Exhibit 23-5
 f_N - Exhibit 23-6
 f_{ID} - Exhibit 23-7

BASIC FREEWAY SEGMENTS WORKSHEET



Application	Input	Output
Operational (LOS)	FFS, N, v_p	LOS, S, D
Design (N)	FFS, LOS, v_p	N, S, D
Design (v_p)	FFS, LOS, N	v_p , S, D
Planning (LOS)	FFS, N, AADT	LOS, S, D
Planning (N)	FFS, LOS, AADT	N, S, D
Planning (v_p)	FFS, LOS, N	v_p , S, D

General Information

Analyst: *KNM*
 Agency or Company: *HNTB*
 Date Performed: *3/25/2008*
 Analysis Time Period: *Peak*
 Project Description: *Wekiva Parkway PD&E*

Site Information

Highway/Direction of Travel: *I-4/Eastbound*
 From/To: *CR 46A/SR 417/SR 46*
 Jurisdiction:
 Analysis Year: *2022 Build*

Oper. (LOS)

Des. (N)

Planning Data

Flow Inputs

Volume, V: *4520* veh/h
 AADT: veh/day
 Peak-Hr Prop. of AADT, K: % Trucks and Buses, P_T : *9*
 Peak-Hr Direction Prop, D: %RVs, P_R : *0*
 DDHV = AADT x K x D: veh/h
 Driver type adjustment: *1.00*
 General Terrain: *Level*
 Grade % Length: *mi*
 Up/Down %

Calculate Flow Adjustments

f_p : *1.00*
 E_T : *1.5*
 E_R : *1.2*
 $f_{HV} = 1/[1+P_T(E_T - 1) + P_R(E_R - 1)]$: *0.957*

Speed Inputs

Lane Width: *12.0* ft
 Rt-Shoulder Lat. Clearance: *6.0* ft
 Interchange Density: *0.54* l/mi
 Number of Lanes, N: *3*
 FFS (measured): mi/h
 Base free-flow Speed, BFFS: *70.0* mi/h

Calc Speed Adj and FFS

f_{LW} : *0.0* mi/h
 f_{LC} : *0.0* mi/h
 f_{ID} : *0.2* mi/h
 f_N : *3.0* mi/h
 FFS: *66.8* mi/h

LOS and Performance Measures

Operational (LOS)
 $v_p = (V \text{ or } DDHV) / (PHF \times N \times f_{HV} \times f_p)$: *1657* pc/h/ln
 S: *66.3* mi/h
 $D = v_p / S$: *25.0* pc/mi/ln
 LOS: *C*

Design (N)

Design (N)
 Design LOS
 $v_p = (V \text{ or } DDHV) / (PHF \times N \times f_{HV} \times f_p)$: pc/h
 S: mi/h
 $D = v_p / S$: pc/mi/ln
 Required Number of Lanes, N

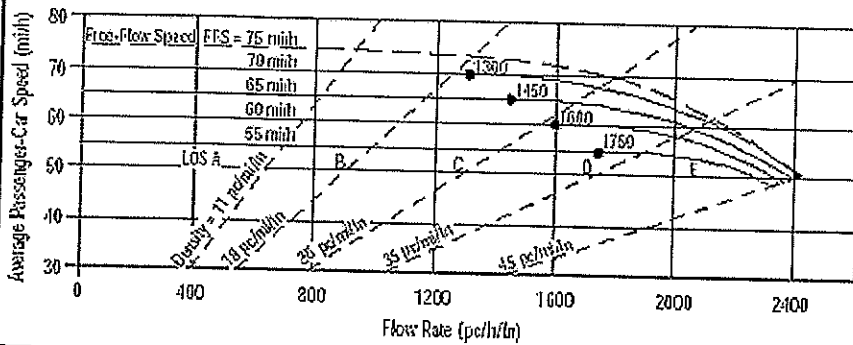
Glossary

N - Number of lanes
 V - Hourly volume
 v_p - Flow rate
 LOS - Level of service
 DDHV - Directional design hour volume
 S - Speed
 D - Density
 FFS - Free-flow speed
 BFFS - Base free-flow speed

Factor Location

E_R - Exhibits 23-8, 23-10
 E_T - Exhibits 23-8, 23-10, 23-11
 f_p - Page 23-12
 LOS, S, FFS, v_p - Exhibits 23-2, 23-3
 f_{LW} - Exhibit 23-4
 f_{LC} - Exhibit 23-5
 f_N - Exhibit 23-6
 f_{ID} - Exhibit 23-7

BASIC FREEWAY SEGMENTS WORKSHEET



Application	Input	Output
Operational (LOS)	FFS, N, v_p	LOS, S, D
Design (N)	FFS, LOS, v_p	N, S, D
Design (v_p)	FFS, LOS, N	v_p , S, D
Planning (LOS)	FFS, N, AADT	LOS, S, D
Planning (N)	FFS, LOS, AADT	N, S, D
Planning (v_p)	FFS, LOS, N	v_p , S, D

General Information		Site Information	
Analyst	KNM	Highway/Direction of Travel	I-4/Eastbound
Agency or Company	HNTB	From/To	SR 46/US17/92
Date Performed	3/25/2008	Jurisdiction	
Analysis Time Period	Peak	Analysis Year	2022 Build
Project Description Wekiva Parkway PD&E			

Oper. (LOS)
 Des. (N)
 Planning Data

Flow Inputs			
Volume, V	6390	veh/h	Peak-Hour Factor, PHF
AAADT		veh/day	0.95
Peak-Hr Prop. of AAADT, K			%Trucks and Buses, P_T
Peak-Hr Direction Prop, D			9
DDHV = AAADT x K x D		veh/h	%RVs, P_R
Driver type adjustment	1.00		0
			General Terrain: Level
			Grade % Length mi
			Up/Down %

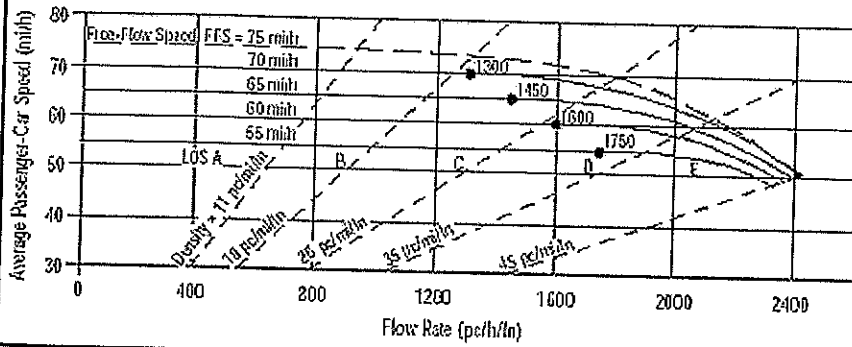
Calculate Flow Adjustments			
f_p	1.00	E_R	1.2
E_T	1.5	$f_{HV} = 1/[1+P_T(E_T-1) + P_R(E_R-1)]$	0.957

Speed Inputs		Calc Speed Adj and FFS	
Lane Width	12.0 ft	f_{LW}	0.0 mi/h
Rt-Shoulder Lat. Clearance	6.0 ft	f_{LC}	0.0 mi/h
Interchange Density	0.67 l/mi	f_{ID}	0.9 mi/h
Number of Lanes, N	4	f_N	1.5 mi/h
FFS (measured)		FFS	67.6 mi/h
Base free-flow Speed, BFFS	70.0 mi/h		

LOS and Performance Measures		Design (N)	
Operational (LOS)		Design (N)	
$v_p = (V \text{ or } DDHV) / (PHF \times N \times f_{HV} \times f_p)$	1757 pc/h/ln	Design LOS	
S	66.4 mi/h	$v_p = (V \text{ or } DDHV) / (PHF \times N \times f_{HV} \times f_p)$	pc/h
$D = v_p / S$	26.5 pc/mi/ln	S	mi/h
LOS	D	$D = v_p / S$	pc/mi/ln
		Required Number of Lanes, N	

Glossary		Factor Location	
N - Number of lanes	S - Speed	E_R - Exhibits 23-8, 23-10	f_{LW} - Exhibit 23-4
V - Hourly volume	D - Density	E_T - Exhibits 23-8, 23-10, 23-11	f_{LC} - Exhibit 23-5
v_p - Flow rate	FFS - Free-flow speed	f_p - Page 23-12	f_N - Exhibit 23-6
LOS - Level of service	BFFS - Base free-flow speed	LOS, S, FFS, v_p - Exhibits 23-2, 23-3	f_{ID} - Exhibit 23-7
DDHV - Directional design hour volume			

BASIC FREEWAY SEGMENTS WORKSHEET



Application	Input	Output
Operational (LOS)	FFS, N, v_p	LOS, S, D
Design (N)	FFS, LOS, v_p	N, S, D
Design (v_p)	FFS, LOS, N	v_p , S, D
Planning (LOS)	FFS, N, AADT	LOS, S, D
Planning (N)	FFS, LOS, AADT	N, S, D
Planning (v_p)	FFS, LOS, N	v_p , S, D

General Information

Analyst: *KNM*
 Agency or Company: *HNTB*
 Date Performed: *3/25/2008*
 Analysis Time Period: *Peak*
 Project Description: *Wekiva Parkway PD&E*

Site Information

Highway/Direction of Travel: *I-4/Eastbound*
 From/To: *Us17/92 to Volusia County Line*
 Jurisdiction:
 Analysis Year: *2022 Build*

Oper. (LOS)

Des. (N)

Planning Data

Flow Inputs

Volume, V: *5950* veh/h
 AADT: veh/day
 Peak-Hr Prop. of AADT, K: % Trucks and Buses, P_T : *9*
 Peak-Hr Direction Prop, D: %RVs, P_R : *0*
 DDHV = AADT x K x D: veh/h
 Driver type adjustment: *1.00*
 General Terrain: *Level*
 Grade % Length: *mi*
 Up/Down %

Calculate Flow Adjustments

f_p : *1.00*
 E_T : *1.5*
 E_R : *1.2*
 $f_{HV} = 1/[1+P_T(E_T - 1) + P_R(E_R - 1)]$: *0.957*

Speed Inputs

Lane Width: *12.0* ft
 Rt-Shoulder Lat. Clearance: *6.0* ft
 Interchange Density: *2.00* l/mi
 Number of Lanes, N: *3*
 FFS (measured): mi/h
 Base free-flow Speed, BFFS: *70.0* mi/h

Calc Speed Adj and FFS

f_{LW} : *0.0* mi/h
 f_{LC} : *0.0* mi/h
 f_{ID} : *7.5* mi/h
 f_N : *3.0* mi/h
 FFS: *59.5* mi/h

LOS and Performance Measures

Operational (LOS)
 $v_p = (V \text{ or } DDHV) / (PHF \times N \times f_{HV} \times f_p)$: *2182* pc/h/ln
 S: *54.2* mi/h
 $D = v_p / S$: *40.3* pc/mi/ln
 LOS: *E*

Design (N)

Design (N)
 Design LOS
 $v_p = (V \text{ or } DDHV) / (PHF \times N \times f_{HV} \times f_p)$: pc/h
 S: mi/h
 $D = v_p / S$: pc/mi/ln
 Required Number of Lanes, N

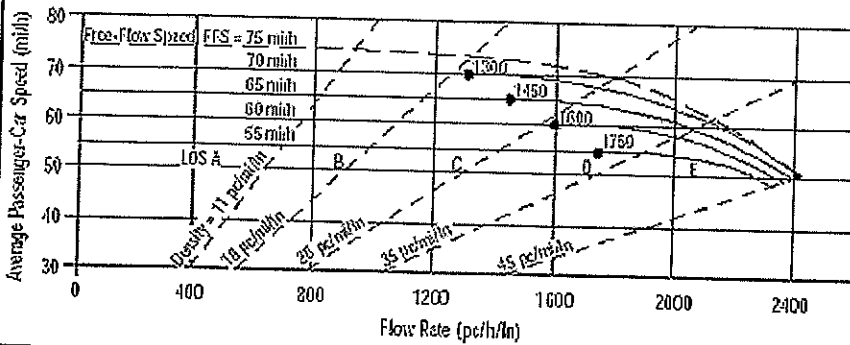
Glossary

N - Number of lanes
 V - Hourly volume
 v_p - Flow rate
 LOS - Level of service
 DDHV - Directional design hour volume
 S - Speed
 D - Density
 FFS - Free-flow speed
 BFFS - Base free-flow speed

Factor Location

E_R - Exhibits 23-8, 23-10
 E_T - Exhibits 23-8, 23-10, 23-11
 f_p - Page 23-12
 LOS, S, FFS, v_p - Exhibits 23-2, 23-3
 f_{LW} - Exhibit 23-4
 f_{LC} - Exhibit 23-5
 f_N - Exhibit 23-6
 f_{ID} - Exhibit 23-7

BASIC FREEWAY SEGMENTS WORKSHEET



Application	Input	Output
Operational (LOS)	FFS, N, v_p	LOS, S, D
Design (N)	FFS, LOS, v_p	N, S, D
Design (v_p)	FFS, LOS, N	v_p , S, D
Planning (LOS)	FFS, N, AADT	LOS, S, D
Planning (N)	FFS, LOS, AADT	N, S, D
Planning (v_p)	FFS, LOS, N	v_p , S, D

General Information		Site Information	
Analyst	KNM	Highway/Direction of Travel	SR 417/Westbound
Agency or Company	HNTB	From/To	North of Rinehart Rd/Rinehart
Date Performed	3/25/2008	Jurisdiction	
Analysis Time Period	Peak	Analysis Year	2022 Build
Project Description Wekiva Parkway PD&E			

<input checked="" type="checkbox"/> Oper.(LOS)	<input type="checkbox"/> Des.(N)	<input type="checkbox"/> Planning Data
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Flow Inputs			
Volume, V	4770	veh/h	Peak-Hour Factor, PHF
AADT		veh/day	%Trucks and Buses, P_T
Peak-Hr Prop. of AADT, K			%RVs, P_R
Peak-Hr Direction Prop, D			General Terrain:
DDHV = AADT x K x D		veh/h	Grade % Length
Driver type adjustment	1.00		Up/Down %

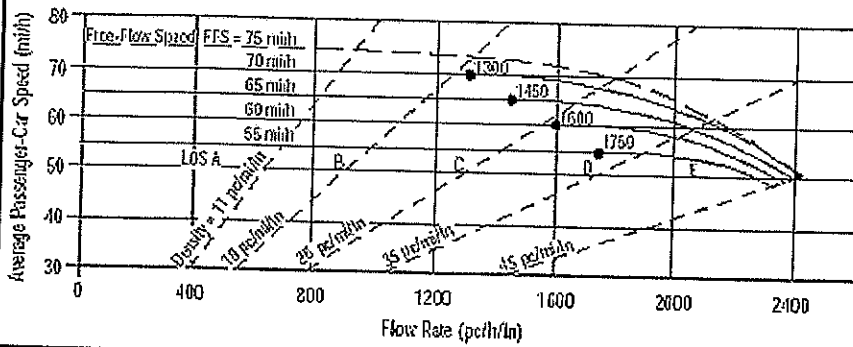
Calculate Flow Adjustments			
f_p	1.00	E_R	1.2
E_T	1.5	$f_{HV} = 1/[1+P_T(E_T - 1) + P_R(E_R - 1)]$	0.952

Speed Inputs		Calc Speed Adj and FFS	
Lane Width	12.0 ft	f_{LW}	0.0 mi/h
Rt-Shoulder Lat. Clearance	6.0 ft	f_{LC}	0.0 mi/h
Interchange Density	0.40 l/mi	f_{ID}	0.0 mi/h
Number of Lanes, N	3	f_N	3.0 mi/h
FFS (measured)		FFS	67.0 mi/h
Base free-flow Speed, BFFS	70.0 mi/h		

LOS and Performance Measures		Design (N)	
Operational (LOS)		Design (N)	
$v_p = (V \text{ or } DDHV) / (PHF \times N \times f_{HV} \times f_p)$	1757 pc/h/ln	Design LOS	
S	65.9 mi/h	$v_p = (V \text{ or } DDHV) / (PHF \times N \times f_{HV} \times f_p)$	pc/h
$D = v_p / S$	26.7 pc/mi/ln	S	mi/h
LOS	D	$D = v_p / S$	pc/mi/ln
		Required Number of Lanes, N	

Glossary		Factor Location	
N - Number of lanes	S - Speed	E_R - Exhibits 23-8, 23-10	f_{LW} - Exhibit 23-4
V - Hourly volume	D - Density	E_T - Exhibits 23-8, 23-10, 23-11	f_{LC} - Exhibit 23-5
v_p - Flow rate	FFS - Free-flow speed	f_p - Page 23-12	f_N - Exhibit 23-6
LOS - Level of service	BFFS - Base free-flow speed	LOS, S, FFS, v_p - Exhibits 23-2, 23-3	f_{ID} - Exhibit 23-7
DDHV - Directional design hour volume			

BASIC FREEWAY SEGMENTS WORKSHEET



Application	Input	Output
Operational (LOS)	FFS, N, v_p	LOS, S, D
Design (N)	FFS, LOS, v_p	N, S, D
Design (v_p)	FFS, LOS, N	v_p , S, D
Planning (LOS)	FFS, N, AADT	LOS, S, D
Planning (N)	FFS, LOS, AADT	N, S, D
Planning (v_p)	FFS, LOS, N	v_p , S, D

General Information

Analyst: *KNM*
 Agency or Company: *HNTB*
 Date Performed: *3/25/2008*
 Analysis Time Period: *Peak*
 Project Description: *Wekiva Parkway PD&E*

Site Information

Highway/Direction of Travel: *SR 417/Westbound*
 From/To: *Rinehart Rd to I-4*
 Jurisdiction:
 Analysis Year: *2022 Build*

Oper. (LOS)

Des. (N)

Planning Data

Flow Inputs

Volume, V	4140	veh/h	Peak-Hour Factor, PHF	0.95
AADT		veh/day	%Trucks and Buses, P_T	10
Peak-Hr Prop. of AADT, K			%RVs, P_R	0
Peak-Hr Direction Prop, D			General Terrain:	Level
DDHV = AADT x K x D		veh/h	Grade %	mi
Driver type adjustment	1.00		Up/Down %	

Calculate Flow Adjustments

f_p	1.00	E_R	1.2
E_T	1.5	$f_{HV} = 1/[1+P_T(E_T-1) + P_R(E_R-1)]$	0.952

Speed Inputs

Lane Width	12.0	ft
Rt-Shoulder Lat. Clearance	6.0	ft
Interchange Density	2.00	l/mi
Number of Lanes, N	3	
FFS (measured)		mi/h
Base free-flow Speed, BFFS	70.0	mi/h

Calc Speed Adj and FFS

f_{LW}	0.0	mi/h
f_{LC}	0.0	mi/h
f_{ID}	7.5	mi/h
f_N	3.0	mi/h
FFS	59.5	mi/h

LOS and Performance Measures

Operational (LOS)
 $v_p = (V \text{ or } DDHV) / (PHF \times N \times f_{HV} \times f_p)$ 1525 pc/h/ln
 S 59.5 mi/h
 $D = v_p / S$ 25.6 pc/mi/ln
 LOS C

Design (N)

Design (N)
 Design LOS
 $v_p = (V \text{ or } DDHV) / (PHF \times N \times f_{HV} \times f_p)$ pc/h
 S mi/h
 $D = v_p / S$ pc/mi/ln
 Required Number of Lanes, N

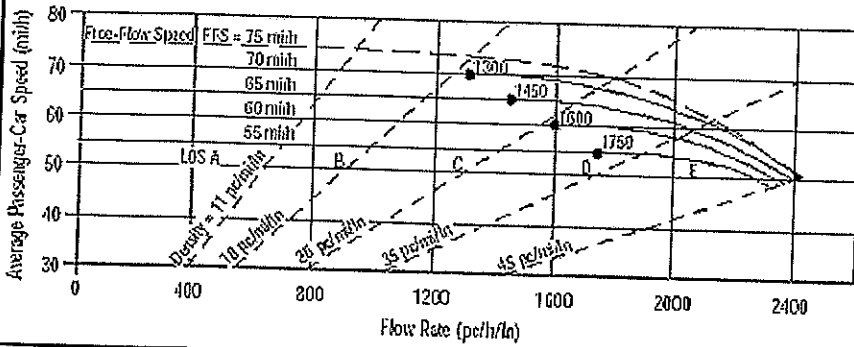
Glossary

N - Number of lanes
 V - Hourly volume
 v_p - Flow rate
 LOS - Level of service
 DDHV - Directional design hour volume
 S - Speed
 D - Density
 FFS - Free-flow speed
 BFFS - Base free-flow speed

Factor Location

E_R - Exhibits 23-8, 23-10
 E_T - Exhibits 23-8, 23-10, 23-11
 f_p - Page 23-12
 LOS, S, FFS, v_p - Exhibits 23-2, 23-3
 f_{LW} - Exhibit 23-4
 f_{LC} - Exhibit 23-5
 f_N - Exhibit 23-6
 f_{ID} - Exhibit 23-7

BASIC FREEWAY SEGMENTS WORKSHEET



Application	Input	Output
Operational (LOS)	FFS, N, v_p	LOS, S, D
Design (N)	FFS, LOS, v_p	N, S, D
Design (v_p)	FFS, LOS, N	v_p , S, D
Planning (LOS)	FFS, N, AADT	LOS, S, D
Planning (N)	FFS, LOS, AADT	N, S, D
Planning (v_p)	FFS, LOS, N	v_p , S, D

General Information		Site Information	
Analyst	KNM	Highway/Direction of Travel	SR 417/Westbound
Agency or Company	HNTB	From/To	I-4 to CD Road
Date Performed	3/25/2008	Jurisdiction	
Analysis Time Period	Peak	Analysis Year	2022 Build
Project Description Wekiva Parkway PD&E			

Oper. (LOS)
 Des. (N)
 Planning Data

Flow Inputs		Calc Speed Adj and FFS	
Volume, V	3260 veh/h	Peak-Hour Factor, PHF	0.95
AADT	veh/day	%Trucks and Buses, P_T	10
Peak-Hr Prop. of AADT, K		%RVs, P_R	0
Peak-Hr Direction Prop, D		General Terrain:	Level
DDHV = AADT x K x D		Grade %	mi
Driver type adjustment	1.00	Length	
		Up/Down %	

Calculate Flow Adjustments		Design (N)	
f_p	1.00	E_R	1.2
E_T	1.5	$f_{HV} = 1/[1+P_T(E_T - 1) + P_R(E_R - 1)]$	0.952

Speed Inputs		Design (N)	
Lane Width	12.0 ft	f_{LW}	0.0 mi/h
Rt-Shoulder Lat. Clearance	6.0 ft	f_{LC}	0.0 mi/h
Interchange Density	2.00 I/mi	f_{ID}	7.5 mi/h
Number of Lanes, N	3	f_N	3.0 mi/h
FFS (measured)	mi/h	FFS	59.5 mi/h
Base free-flow Speed, BFFS	70.0 mi/h		

LOS and Performance Measures		Design (N)	
Operational (LOS)		Design (N)	
$v_p = (V \text{ or } DDHV) / (PHF \times N \times f_{HV} \times f_p)$	1201 pc/h/ln	Design LOS	
S	59.5 mi/h	$v_p = (V \text{ or } DDHV) / (PHF \times N \times f_{HV} \times f_p)$	pc/h
$D = v_p / S$	20.2 pc/mi/ln	S	mi/h
LOS	C	$D = v_p / S$	pc/mi/ln
		Required Number of Lanes, N	

Glossary		Factor Location	
N - Number of lanes	S - Speed	E_R - Exhibits 23-8, 23-10	f_{LW} - Exhibit 23-4
V - Hourly volume	D - Density	E_T - Exhibits 23-8, 23-10, 23-11	f_{LC} - Exhibit 23-5
v_p - Flow rate	FFS - Free-flow speed	f_p - Page 23-12	f_N - Exhibit 23-6
LOS - Level of service	BFFS - Base free-flow speed	LOS, S, FFS, v_p - Exhibits 23-2, 23-3	f_{ID} - Exhibit 23-7
DDHV - Directional design hour volume			

SR 417 WB Off Ramp to I-4 EB & WB.txt
HCS+: Ramps and Ramp Junctions Release 5.4

Phone: _____ Fax: _____
E-mail: _____

-----Diverge Analysis-----

Analyst: CTR
Agency/Co.: HNTB
Date performed: 08/03/2010
Analysis time period: Build Service Road Concept
Freeway/Dir of Travel: SR 417 WB
Junction: Off Ramp to I-4 EB/WB
Jurisdiction: Seminole County
Analysis Year: 2022
Description: Wekiva Parkway Project Development & Environment Study

-----Freeway Data-----

Type of analysis	Diverge	
Number of lanes in freeway	4	
Free-flow speed on freeway	55.0	mph
Volume on freeway	4030	vph

-----Off Ramp Data-----

Side of freeway	Right	
Number of lanes in ramp	2	
Free-Flow speed on ramp	35.0	mph
Volume on ramp	1590	vph
Length of first accel/decel lane	0	ft
Length of second accel/decel lane	1500	ft

-----Adjacent Ramp Data (if one exists)-----

Does adjacent ramp exist?	Yes	
Volume on adjacent ramp	910	vph
Position of adjacent ramp	Downstream	
Type of adjacent ramp	Off	
Distance to adjacent ramp	3765	ft

-----Conversion to pc/h Under Base Conditions-----

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	4030	1590	910	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	1095	432	247	v
Trucks and buses	10	10	10	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00 %	0.00 %	0.00 %	
Length	0.00 mi	0.00 mi	0.00 mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.952	0.952	0.952	
Driver population factor, fP	1.00	1.00	1.00	

SR 417 WB Off Ramp to I-4 EB & WB.txt
 Flow rate, vp 4599 1815 1039 pcph

Estimation of V12 Diverge Areas

$$L = \frac{EQ}{FD} \quad (\text{Equation 25-8 or 25-9})$$

$$P = 0.260 \quad \text{Using Equation 0}$$

$$v_{12} = v_R + (v_F - v_R) P = 2539 \quad \text{pc/h}$$

Capacity Checks

	Actual	Maximum	LOS F?
$v_{Fi} = v_F$	4599	9000	No
$v_{FO} = v_F - v_R$	2784	9000	No
v_R	1815	3800	No
$v_{3 \text{ or } av34}$	1030 pc/h	(Equation 25-15 or 25-16)	
Is $v_{3 \text{ or } av34} > 2700 \text{ pc/h?}$		No	
Is $v_{3 \text{ or } av34} > 1.5 v_{12} / 2$		No	
If yes, $v_{12A} = 2539$		(Equation 25-18)	

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v_{12}	2539	4400	No

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v_{12} - 0.009 L_D = 12.6 \quad \text{pc/mi/ln}$

Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable,	$D = 0.591$	
Space mean speed in ramp influence area,	$S_R = 47.3$	mph
Space mean speed in outer lanes,	$S_0 = 60.2$	mph
space mean speed for all vehicles,	$S = 52.3$	mph

Phone:
E-mail:

Fax:

Diverge Analysis

Analyst: CTR
 Agency/Co.: HNTB
 Date performed: 8/02/2010
 Analysis time period: Build Service Road Concept
 Freeway/Dir of Travel: SR 417 WB
 Junction: Off Ramp to International Pkwy
 Jurisdiction: Seminole County
 Analysis Year: 2022
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Diverge		
Number of lanes in freeway	3		
Free-flow speed on freeway	55.0	mph	
Volume on freeway	2440 ✓	vph	

Off Ramp Data

Side of freeway	Right		
Number of lanes in ramp	1		
Free-Flow speed on ramp	35.0	mph	
Volume on ramp	910 ✓	vph	
Length of first accel/decel lane	0	ft	
Length of second accel/decel lane		ft	

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes ✓		
Volume on adjacent ramp	850 ✓	vph	
Position of adjacent ramp	Downstream		
Type of adjacent ramp	On		
Distance to adjacent ramp	2076	ft	

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	2440 ✓	910	850	vph
Peak-hour factor, PHF	0.92 ✓	0.92	0.92	
Peak 15-min volume, v15	663 ✓	247	231	v
Trucks and buses	11 ✓	11	9	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00 %	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.948	0.948	0.957	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	2798	1044	965	pcph

Estimation of V12 Diverge Areas

L = (Equation 25-8 or 25-9)
EQ
P = 0.642 Using Equation 5
FD
 $v_{12} = v_R + (v_F - v_R) P = 2170$ pc/h
FD

Capacity Checks

	Actual	Maximum	LOS F?
$v_{Fi} = v_F$	2798	6750	No
$v_{FO} = v_F - v_R$	1754	6750	No
v_R	1044	2000	No
$v_{3 \text{ or } av34}$	628 pc/h	(Equation 25-15 or 25-16)	
Is $v_{3 \text{ or } av34} > 2700$ pc/h?		No	
Is $v_{3 \text{ or } av34} > 1.5 v_{12} / 2$		No	
If yes, $v_{12A} = 2170$		(Equation 25-18)	

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v_{12}	2170	4400	No

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v_{12} - 0.009 L_D = 22.9$ pc/mi/ln
Level of service for ramp-freeway junction areas of influence C ✓

Speed Estimation

Intermediate speed variable,	D = 0,522	
Space mean speed in ramp influence area,	S = 48.2	mph
Space mean speed in outer lanes,	S = 60.3	mph
Space mean speed for all vehicles,	S = 50.5	mph

Phone:
E-mail:

Fax:

Diverge Analysis

Analyst: CTR
 Agency/Co.: HNTB
 Date performed: 08/02/2010
 Analysis time period: Build Service Road Concept
 Freeway/Dir of Travel: SR 417 WB
 Junction: Off Ramp to International Pkwy
 Jurisdiction: Seminole County
 Analysis Year: 2022
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	3	
Free-flow speed on freeway	55.0	mph
Volume on freeway	2440 ✓	vph

Off Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	35.0 ✓	mph
Volume on ramp	910 ✓	vph
Length of first accel/decel lane	0	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent ramp	1590 ✓	vph
Position of adjacent ramp	Upstream	
Type of adjacent ramp	Off	
Distance to adjacent ramp	3765	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	2440 ✓	910	1590	vph
Peak-hour factor, PHF	0.92 ✓	0.92	0.92	
Peak 15-min volume, v15	663 ✓	247	432	v
Trucks and buses	11 ✓	11	11	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00 %	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.948	0.948	0.948	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	2798	1044	1823	pcph

Estimation of V12 Diverge Areas

$$L = \frac{EQ}{P} \quad \text{(Equation 25-8 or 25-9)}$$

$$P = 0.642 \quad \text{Using Equation 5}$$

$$v_{12R} = v_{12F} + (v_{12R} - v_{12F}) P_{FD} = 2170 \quad \text{pc/h}$$

Capacity Checks

	Actual	Maximum	LOS F?
v_{12F}	2798	6750	No
v_{12R}	1754	6750	No
v_{12R}	1044	2000	No
v_{12R}	628 pc/h	(Equation 25-15 or 25-16)	
Is $v_{12R} > 2700$ pc/h?		No	
Is $v_{12R} > 1.5 v_{12R} / 2$		No	
If yes, $v_{12A} = 2170$		(Equation 25-18)	

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v_{12}	2170	4400	No

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v_{12} - 0.009 L_D = 22.9$ pc/mi/ln

Level of service for ramp-freeway junction areas of influence C ✓

Speed Estimation

Intermediate speed variable,	$D = 0.522$	
Space mean speed in ramp influence area,	$S_R = 48.2$	mph
Space mean speed in outer lanes,	$S_0 = 60.3$	mph
Space mean speed for all vehicles,	$S = 50.5$	mph

HCS+: Ramps and Ramp Junctions Release 5.4

Phone: Fax:
E-mail:

Merge Analysis

Analyst: CTRR
 Agency/Co.: HNTB
 Date performed: 03/07/2010
 Analysis time period: Build Service Road Concept
 Freeway/Dir of Travel: SR 417 WB
 Junction: On Ramp from I-4 EB & WB
 Jurisdiction: Seminole County
 Analysis Year: 2022
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Merge		
Number of lanes in freeway	3		
Free-flow speed on freeway	55.0	mph	
Volume on freeway	1530	vph	

On Ramp Data

Side of freeway	Right		
Number of lanes in ramp	1		
Free-flow speed on ramp	35.0	mph	
Volume on ramp	850	vph	
Length of first accel/decel lane	1325	ft	
Length of second accel/decel lane		ft	

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes		
Volume on adjacent Ramp	910	vph	
Position of adjacent Ramp	Upstream		
Type of adjacent Ramp	Off		
Distance to adjacent Ramp	2076	ft	

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
		Ramp		
Volume, V (vph)	1530	850	910	vph
Peak-hour factor, PHF	0.90	0.90	0.90	

Peak 15-min volume, v_{15}	425	236	253	v
Trucks and buses	0	0	0	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	1.000	1.000	1.000	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, v_p	1700	944	1011	pcph

Estimation of V12 Merge Areas

$$L = 582.32 \text{ (Equation 25-2 or 25-3)}$$

EQ

$$P = 0.615 \text{ Using Equation 1}$$

FM

$$v = v(P) = 1045 \text{ pc/h}$$

12 F FM

Capacity Checks

v	Actual	Maximum	LOS F?
	2644	6750	No
FO			
v	v	655 pc/h	(Equation 25-4 or 25-5)
3 or av34			
Is v	v	> 2700 pc/h?	No
3 or av34			
Is v	v	> 1.5 v / 2	No
3 or av34	12		
If yes, v	= 1045	(Equation 25-8)	
12A			

Flow Entering Merge Influence Area

v	Actual	Max Desirable	Violation?
	1045	4600	No
R12			

Level of Service Determination (if not F)

$$\text{Density, } D = 5.475 + 0.00734 \frac{v}{R} + 0.0078 \frac{v}{R} - 0.00627 L = 12.2 \text{ pc/mi/ln}$$

Level of service for ramp-freeway junction areas of influence B

Speed Estimation

$$\text{Intermediate speed variable, } M = 0.257$$

S

$$\text{Space mean speed in ramp influence area, } S = 51.7 \text{ mph}$$

R

$$\text{Space mean speed in outer lanes, } S = 54.4 \text{ mph}$$

0

$$\text{Space mean speed for all vehicles, } S = 52.3 \text{ mph}$$

HCS+: Ramps and Ramp Junctions Release 5.4

Phone: Fax:
E-mail:

Merge Analysis

Analyst: CTRR
 Agency/Co.: HNTB
 Date performed: 03/07/2010
 Analysis time period: Build Service Road Concept
 Freeway/Dir of Travel: SR 417 WB
 Junction: On Ramp from I-4 EB & WB
 Jurisdiction: Seminole County
 Analysis Year: 2022
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Merge		
Number of lanes in freeway	3		
Free-flow speed on freeway	55.0	mph	
Volume on freeway	1530	vph	

On Ramp Data

Side of freeway	Right		
Number of lanes in ramp	1		
Free-flow speed on ramp	35.0	mph	
Volume on ramp	850	vph	
Length of first accel/decel lane	1325	ft	
Length of second accel/decel lane		ft	

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes		
Volume on adjacent Ramp	490	vph	
Position of adjacent Ramp	Downstream		
Type of adjacent Ramp	On		
Distance to adjacent Ramp	1325	ft	

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
		Ramp		
Volume, V (vph)	1530	850	490	vph
Peak-hour factor, PHF	0.90	0.90	0.90	

Peak 15-min volume, v_{15}	425	236	136	v
Trucks and buses	0	0	0	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	1.000	1.000	1.000	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, v_p	1700	944	544	pcph

Estimation of V12 Merge Areas

$L =$ (Equation 25-2 or 25-3)

EQ

$P = 0.615$ Using Equation 1

FM

$v = v(P) = 1045$ pc/h

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	2644	6750	No
FO			
v	655 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is $v > 2700$ pc/h?		No	
3 or av34			
Is $v > 1.5 v / 2$		No	
3 or av34 12			
If yes, $v = 1045$		(Equation 25-8)	
12A			

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	1045	4600	No
R12			

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 12.2$ pc/mi/ln

R R 12 A

Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable, $M = 0.257$

S

Space mean speed in ramp influence area, $S = 51.7$ mph

R

Space mean speed in outer lanes, $S = 54.4$ mph

0

Space mean speed for all vehicles, $S = 52.3$ mph

Phone:
E-mail:

Fax:

Merge Analysis

Analyst: CTR
 Agency/Co.: HNTB
 Date performed: 08/03/2010
 Analysis time period: Build Service Road Concept
 Freeway/Dir of Travel: SR 417 WB
 Junction: On Ramp from International Pwy
 Jurisdiction: Seminole County
 Analysis Year: 2022
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	4 ✓	
Free-flow speed on freeway	55.0	mph
Volume on freeway	2380 ✓	vph

On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1 ✓	
Free-flow speed on ramp	35.0	mph
Volume on ramp	490 ✓	vph
Length of first accel/decel lane	900	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes ✓	
Volume on adjacent Ramp	850 ✓	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1325	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	2380 ✓	490	850	vph
Peak-hour factor, PHF	0.92 ✓	0.92	0.92	
Peak 15-min volume, v15	647 ✓	133	231	v
Trucks and buses	11 ✓	9	9	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade		%	%	%
Length		mi	mi	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.948	0.957	0.957	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	2729	557	965	pcph

Estimation of V12 Merge Areas

$$L = \text{(Equation 25-2 or 25-3)}$$

$$EQ$$

$$P = 0.148 \text{ Using Equation 4}$$

$$FM$$

$$v_{12} = v_F (P_{FM}) = 404 \text{ pc/h}$$

Capacity Checks

		Actual	Maximum	LOS F?
v _{FO}		3286	9000	No
v _{3 or av34}	v _{av34}	1162 pc/h	(Equation 25-4 or 25-5)	
Is v _{3 or av34}	v _{av34}	> 2700 pc/h?	No	
Is v _{3 or av34}	v _{av34}	> 1.5 v ₁₂ / 2	Yes	
If yes, v _{12A}		= 1091	(Equation 25-8)	

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v _{12A}	1091	4600	No

Level of Service Determination (if not F)

$$\text{Density, } D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 12.4 \text{ pc/mi/ln}$$

Level of service for ramp-freeway junction areas of influence B ✓

Speed Estimation

Intermediate speed variable,	M = 0.278	
	S	
Space mean speed in ramp influence area,	S = 51.4	mph
	R	
Space mean speed in outer lanes,	S = 53.9	mph
	O	
Space mean speed for all vehicles,	S = 52.6	mph

I-4 WB On Ramp from CR 46A & SR 46.txt
HCS+: Basic Freeway Segments Release 5.4

Phone: Fax:
E-mail:

Operational Analysis

Analyst: CTR
Agency or Company: HNTB
Date Performed: 8/10/2010
Analysis Time Period: Build
Freeway/Direction: I-4 WB
From/To: On Ramp from CR 46A & SR 46
Jurisdiction: Seminole County
Analysis Year: 2022
Description: Wekiva Parkway PD&E

Flow Inputs and Adjustments

Volume, v	6000	veh/h
Peak-hour factor, PHF	0.92	
Peak 15-min volume, v15	1630	v
Trucks and buses	9	%
Recreational vehicles	0	%
Terrain type:	Level	
Grade	0.00	%
Segment length	0.00	mi
Trucks and buses PCE, ET	1.5	
Recreational vehicle PCE, ER	1.2	
Heavy vehicle adjustment, fhv	0.957	
Driver population factor, fp	1.00	
Flow rate, vp	1704	pc/h/ln

Speed Inputs and Adjustments

Lane width	12.0	ft
Right-shoulder lateral clearance	6.0	ft
Interchange density	0.90	interchange/mi
Number of lanes, N	4	
Free-flow speed:	Base	
FFS or BFFS	70.0	mi/h
Lane width adjustment, flw	0.0	mi/h
Lateral clearance adjustment, flc	0.0	mi/h
Interchange density adjustment, fid	2.0	mi/h
Number of lanes adjustment, fn	1.5	mi/h
Free-flow speed, FFS	66.5	mi/h
	Urban Freeway	

LOS and Performance Measures

Flow rate, vp	1704	pc/h/ln
Free-flow speed, FFS	66.5	mi/h
Average passenger-car speed, S	65.8	mi/h
Number of lanes, N	4	
Density, D	25.9	pc/mi/ln
Level of service, LOS	C	

I-4 WB On Ramp from CR 46A & SR 46.txt
overall results are not computed when free-flow speed is less than 55 mph.

SR 417 WB ON Ramp from Rinehart.txt
HCS+: Ramps and Ramp Junctions Release 5.4

Phone: Fax:
E-mail:

Merge Analysis

Analyst: CTR
Agency/Co.: HNTB
Date performed: 8/03/2010
Analysis time period: Build Service Road Concept
Freeway/Dir of Travel: SR 417 WB
Junction: On Ramp from Rinehart Rd
Jurisdiction: Seminole County
Analysis Year: 2022
Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	3	
Free-flow speed on freeway	55.0	mph
Volume on freeway	3600	vph

On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	35.0	mph
Volume on ramp	430	vph
Length of first accel/decel lane	1500	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent Ramp	1170	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	Off	
Distance to adjacent Ramp	1700	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	3600	430	1170	vph
Peak-hour factor, PHF	0.92	0.92	0.90	
Peak 15-min volume, v15	978	117	325	v
Trucks and buses	10	10	0	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade		%	%	%
Length		mi	mi	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.952	0.952	1.000	
Driver population factor, fP	1.00	1.00	1.00	

Flow rate, vp SR 417 WB ON Ramp from Rinehart.txt
 4109 491 1300 pcph

_____ Estimation of V12 Merge Areas _____

L = 1078.60 (Equation 25-2 or 25-3)
 EQ
 P = 0.619 Using Equation 1
 FM
 $v_{12} = v_F (P_{FM}) = 2546$ pc/h

_____ Capacity Checks _____

		Actual	Maximum	LOS F?
	v_{FO}	4600	6750	No
	$v_{3 \text{ or } av34}$	1563 pc/h	(Equation 25-4 or 25-5)	
Is	$v_{3 \text{ or } av34}$	> 2700 pc/h?	No	
Is	$v_{3 \text{ or } av34}$	> $1.5 v_{12} / 2$	No	
If yes,	$v_{12A} = 2546$		(Equation 25-8)	

_____ Flow Entering Merge Influence Area _____

	Actual	Max Desirable	Violation?
v_{R12}	2546	4600	No

_____ Level of Service Determination (if not F) _____

Density, $D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A$ = 19.5 pc/mi/ln
 Level of service for ramp-freeway junction areas of influence B

_____ Speed Estimation _____

Intermediate speed variable,	$M = 0.297$
Space mean speed in ramp influence area,	$S_S = 51.1$ mph
Space mean speed in outer lanes,	$S_R = 51.2$ mph
Space mean speed for all vehicles,	$S_O = 51.1$ mph

Phone:
E-mail:

Fax:

Diverge Analysis

Analyst: CTR
 Agency/Co.: HNTB
 Date performed: 8/02/2010
 Analysis time period: Build Service Road Concept
 Freeway/Dir of Travel: SR 417 WB
 Junction: Off Ramp to Rinehart Rd
 Jurisdiction: Seminole County
 Analysis Year: 2022
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Diverge		
Number of lanes in freeway	3		
Free-flow speed on freeway	55.0	mph	
Volume on freeway	4770 ✓	vph	

Off Ramp Data

Side of freeway	Right		
Number of lanes in ramp	2 ✓		
Free-Flow speed on ramp	35.0	mph	
Volume on ramp	1170 ✓	vph	
Length of first accel/decel lane	500 ✓	ft	
Length of second accel/decel lane	0	ft	

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes ✓		
Volume on adjacent ramp	430 ✓	vph	
Position of adjacent ramp	Downstream		
Type of adjacent ramp	On		
Distance to adjacent ramp	1833	ft	

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	4770 ✓	1170	430	vph
Peak-hour factor, PHF	0.92 ✓	0.92	0.92	
Peak 15-min volume, v15	1296 ✓	318	117	v
Trucks and buses	10	10	10	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00 %	0.00 %	0.00 %	
Length	0.00 mi	0.00 mi	0.00 mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.952	0.952	0.952	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	5444	1335	491	pcph

Estimation of V12 Diverge Areas

$$L = \text{(Equation 25-8 or 25-9)}$$

$$EQ$$

$$P = 0.450 \text{ Using Equation 0}$$

$$FD$$

$$v_{12} = v_R + (v_F - v_R) P = 3184 \text{ pc/h}$$

Capacity Checks

	Actual	Maximum	LOS F?
$v_{Fi} = v_F$	5444	6750	No
$v_{FO} = v_F - v_R$	4109	6750	No
v_R	1335	3800	No
$v_{3 \text{ or } av34}$	2260 pc/h	(Equation 25-15 or 25-16)	
Is $v_{3 \text{ or } av34} > 2700 \text{ pc/h?}$		No	
Is $v_{3 \text{ or } av34} > 1.5 v_{12} / 2$		No	
If yes, $v_{12A} = 3184$		(Equation 25-18)	

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v_{12}	3184	4400	No

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v_{12} - 0.009 L_D = 22.6 \text{ pc/mi/ln}$

Level of service for ramp-freeway junction areas of influence C ✓

Speed Estimation

Intermediate speed variable,	$D = 0.548$	
Space mean speed in ramp influence area,	$S_R = 47.9$	mph
Space mean speed in outer lanes,	$S_O = 55.4$	mph
Space mean speed for all vehicles,	$S = 50.7$	mph

Phone: Fax:
E-mail:

Merge Analysis

Analyst: CTR
 Agency/Co.: HNTB
 Date performed: 8/02/2010
 Analysis time period: Build Service Road Concept
 Freeway/Dir of Travel: SR 417 EB
 Junction: On Ramp from International Pky
 Jurisdiction: Seminole County
 Analysis Year: 2022
 Description: Wekiva Parkway Project Development & Environment Project

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	3	
Free-flow speed on freeway	55.0	mph
Volume on freeway	1530	vph

On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	35.0	mph
Volume on ramp	890	vph
Length of first accel/decel lane	1200	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent Ramp	850	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	Off	
Distance to adjacent Ramp	1964	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	1530	890	850	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	416	242	231	v
Trucks and buses	11	10	10	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade		%	%	%
Length		mi	mi	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fhv	0.948	0.952	0.952	
Driver population factor, fp	1.00	1.00	1.00	

SR 417 EB ON Ramp From Int'l Pkwy_Upstream Analysis.txt
 Flow rate, vp 1755 1016 970 pcph

Estimation of V12 Merge Areas

L = 553.99 (Equation 25-2 or 25-3)
 EQ
 P = 0.611 Using Equation 1
 FM
 $v_{12} = v_F (P_{FM}) = 1072$ pc/h

Capacity Checks

	Actual	Maximum	LOS F?
v_{FO}	2771	6750	No
$v_{3 \text{ or } av34}$	683 pc/h	(Equation 25-4 or 25-5)	
Is $v_{3 \text{ or } av34} > 2700$ pc/h?		No	
Is $v_{3 \text{ or } av34} > 1.5 v_{12} / 2$		No	
If yes, $v_{12A} = 1072$		(Equation 25-8)	

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v_{R12}	1072	4600	No

Level of Service Determination (if not F)

Density, $D_R = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 13.8$ pc/mi/ln
 Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable,	M = 0.268	
Space mean speed in ramp influence area,	S _S = 51.5	mph
Space mean speed in outer lanes,	S _R = 54.3	mph
Space mean speed for all vehicles,	S _O = 52.2	mph

Phone: _____ Fax: _____
 E-mail: _____

_____ Merge Analysis _____

Analyst: CTR
 Agency/Co.: HNTB
 Date performed: 8/02/2010
 Analysis time period: Build Service Road Concept
 Freeway/Dir of Travel: SR 417 EB
 Junction: On Ramp from International Pky
 Jurisdiction: Seminole County
 Analysis Year: 2022
 Description: Wekiva Parkway Project Development & Environment Project

_____ Freeway Data _____

Type of analysis	Merge	
Number of lanes in freeway	3	
Free-flow speed on freeway	55.0	mph
Volume on freeway	1530	vph

_____ On Ramp Data _____

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	35.0	mph
Volume on ramp	890	vph
Length of first accel/decel lane	1200	ft
Length of second accel/decel lane		ft

_____ Adjacent Ramp Data (if one exists) _____

Does adjacent ramp exist?	Yes	
Volume on adjacent Ramp	1660	vph
Position of adjacent Ramp	Downstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	3106	ft

_____ Conversion to pc/h Under Base Conditions _____

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	1530	890	1660	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	416	242	451	v
Trucks and buses	11	10	9	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade		%	%	%
Length		mi	mi	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fhv	0.948	0.952	0.957	
Driver population factor, fp	1.00	1.00	1.00	

Estimation of V12 Merge Areas

$L =$ (Equation 25-2 or 25-3)
 EQ
 $P = 0.611$ Using Equation 1
 FM
 $v_{12} = v_F (P_{FM}) = 1072$ pc/h

Capacity Checks

		Actual	Maximum	LOS F?
		2771	6750	No
	v_{FO}			
	v_3 or v_{av34}	683 pc/h	(Equation 25-4 or 25-5)	
Is	v_3 or $v_{av34} > 2700$ pc/h?		No	
Is	v_3 or $v_{av34} > 1.5 v_{12}/2$		No	
If yes,	$v_{12A} = 1072$		(Equation 25-8)	

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v_{R12}	1072	4600	No

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 13.8$ pc/mi/ln
 Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable,	$M = 0.268$	
Space mean speed in ramp influence area,	$S_S = 51.5$	mph
Space mean speed in outer lanes,	$S_R = 54.3$	mph
Space mean speed for all vehicles,	$S_0 = 52.2$	mph

Phone:
E-mail:

Fax:

Diverge Analysis

Analyst: CTR
 Agency/Co.: HNTB
 Date performed: 8/02/2010
 Analysis time period: Build Service Road Concept
 Freeway/Dir of Travel: SR 417 EB
 Junction: Off Ramp to International Pkwy
 Jurisdiction: Seminole County
 Analysis Year: 2022
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	3	
Free-flow speed on freeway	55.0	mph
Volume on freeway	2870 ✓	vph

Off Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	35.0	mph
Volume on ramp	490 ✓	vph
Length of first accel/decel lane	0	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes ✓	
Volume on adjacent ramp	850 ✓	vph
Position of adjacent ramp	Downstream	
Type of adjacent ramp	Off	
Distance to adjacent ramp	1250	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	2870 ✓	490	850	vph
Peak-hour factor, PHF	0.92 ✓	0.92	0.92	
Peak 15-min volume, v15	780 ✓	133	231	v
Trucks and buses	11 ✓	11	11	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00 %	0.00 %	0.00 %	
Length	0.00 mi	0.00 mi	0.00 mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.948	0.948	0.948	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, v _p	3291	562	975	pcph

Estimation of V12 Diverge Areas

$$L = 1164.95 \text{ (Equation 25-8 or 25-9)}$$

$$EQ$$

$$P = 0.652 \text{ Using Equation 5}$$

$$FD$$

$$v_{12R} = v_{FR} + (v_{FR} - v_{FD}) P = 2341 \text{ pc/h}$$

Capacity Checks

	Actual	Maximum	LOS F?
$v_{Fi} = v_F$	3291	6750	No
$v_{FO} = v_F - v_R$	2729	6750	No
v_R	562	2000	No
$v_{3 \text{ or } av34}$	950 pc/h	(Equation 25-15 or 25-16)	
Is $v_{3 \text{ or } av34} > 2700 \text{ pc/h?}$		No	
Is $v_{3 \text{ or } av34} > 1.5 v_{12} / 2$		No	
If yes, $v_{12A} = 2341$		(Equation 25-18)	

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v_{12}	2341	4400	No

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v_{12} - 0.009 L_D = 24.4 \text{ pc/mi/ln}$

Level of service for ramp-freeway junction areas of influence C ✓

Speed Estimation

Intermediate speed variable,	$D = 0.479$	
Space mean speed in ramp influence area,	$S_R = 48.8$	mph
Space mean speed in outer lanes,	$S_O = 60.3$	mph
Space mean speed for all vehicles,	$S = 51.6$	mph

Phone:
E-mail:

Fax:

Merge Analysis

Analyst: CTR
 Agency/Co.: HNTB
 Date performed: 8/02/2010
 Analysis time period: Build Service Road Concept
 Freeway/Dir of Travel: SR 417 EB
 Junction: On Ramp from I-4 EB & WB
 Jurisdiction: Seminole County
 Analysis Year: 2022
 Description: Wekiva Parkway Project Development & Environment Project

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	3	
Free-flow speed on freeway	55.0	mph
Volume on freeway	2420 ✓	vph

On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	2	
Free-flow speed on ramp	35.0	mph
Volume on ramp	1660 ✓	vph
Length of first accel/decel lane	900	ft
Length of second accel/decel lane	1500	ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes ✓	
Volume on adjacent Ramp	890 ✓	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	3106	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	2420 ✓	1660	890	vph
Peak-hour factor, PHF	0.92 ✓	0.92	0.92	
Peak 15-min volume, v15	658 ✓	451	242	v
Trucks and buses	11 ✓	9	11	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.948	0.957	0.948	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	2775	1886	1021	pcph

Estimation of V12 Merge Areas

$$L = \frac{EQ}{P} \quad (\text{Equation 25-2 or 25-3})$$

$$P = 0.555 \quad \text{Using Equation 0}$$

$$v_{12} = v_F \left(\frac{P}{FM} \right) = 1540 \quad \text{pc/h}$$

Capacity Checks

	Actual	Maximum	LOS F?
v _{FO}	4661	6750	No
v _{3 or av34}	1235 pc/h	(Equation 25-4 or 25-5)	
Is v _{3 or av34} > 2700 pc/h?		No	
Is v _{3 or av34} > 1.5 v ₁₂ / 2		Yes	
If yes, v _{12A} = 1585		(Equation 25-8)	

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v _{12A}	1585	4600	No

Level of Service Determination (if not F)

$$\text{Density, } D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 11.0 \quad \text{pc/mi/ln}$$

Level of service for ramp-freeway junction areas of influence B ✓

Speed Estimation

Intermediate speed variable,	M = 0.215	
Space mean speed in ramp influence area,	S _R = 52.2	mph
Space mean speed in outer lanes,	S ₀ = 52.5	mph
Space mean speed for all vehicles,	S = 52.3	mph

Phone:
E-mail:

Fax:

Diverge Analysis

Analyst: CTR
 Agency/Co.: HNTB
 Date performed: 8/02/2010
 Analysis time period: Build Service Road Concept
 Freeway/Dir of Travel: SR 417 EB
 Junction: Off Ramp to I-4 EB & WB
 Jurisdiction: Seminole County
 Analysis Year: 2022
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	3	
Free-flow speed on freeway	55.0 ✓	mph
Volume on freeway	2380 ✓	vph

Off Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	35.0 ✓	mph
Volume on ramp	850 ✓	vph
Length of first accel/decel lane	0	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes ✓	
Volume on adjacent ramp	890 ✓	vph
Position of adjacent ramp	Downstream	
Type of adjacent ramp	On	
Distance to adjacent ramp	1964	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	2380 ✓	850	890	vph
Peak-hour factor, PHF	0.92 ✓	0.92	0.92	
Peak 15-min volume, v15	647 ✓	231	242	v
Trucks and buses	11 ✓	11	11	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00 %	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.948	0.948	0.948	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	2729	975	1021	pcph

Estimation of V12 Diverge Areas

$$L = \text{(Equation 25-8 or 25-9)}$$

$$EQ$$

$$P = 0.647 \quad \text{Using Equation 5}$$

$$FD$$

$$v_{12R} = v_F + (v_F - v_R) P = 2110 \quad \text{pc/h}$$

Capacity Checks

	Actual	Maximum	LOS F?
$v_{Fi} = v_F$	2729	6750	No
$v_{FO} = v_F - v_R$	1754	6750	No
v_R	975	2000	No
$v_{3 \text{ or } av34}$	619 pc/h	(Equation 25-15 or 25-16)	
Is $v_{3 \text{ or } av34} > 2700$ pc/h?		No	
Is $v_{3 \text{ or } av34} > 1.5 v_{12} / 2$		No	
If yes, $v_{12A} = 2110$		(Equation 25-18)	

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v_{12}	2110	4400	No

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v_{12} - 0.009 L_D = 22.4 \quad \text{pc/mi/ln}$

Level of service for ramp-freeway junction areas of influence C ✓

Speed Estimation

Intermediate speed variable,	$D = 0.516$	
	S	
Space mean speed in ramp influence area,	$S = 48.3$	mph
	R	
Space mean speed in outer lanes,	$S = 60.3$	mph
	O	
Space mean speed for all vehicles,	$S = 50.6$	mph

Phone:
E-mail:

Fax:

Diverge Analysis

Analyst: CTR
 Agency/Co.: HNTB
 Date performed: 8/02/2010
 Analysis time period: Build Service Road Concept
 Freeway/Dir of Travel: SR 417 EB
 Junction: Off Ramp to I-4 EB & WB
 Jurisdiction: Seminole County
 Analysis Year: 2022
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	3	
Free-flow speed on freeway	55.0	mph
Volume on freeway	2380 ✓	vph

Off Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	35.0	mph
Volume on ramp	850 ✓	vph
Length of first accel/decel lane	0	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent ramp	490 ✓	vph
Position of adjacent ramp	Upstream	
Type of adjacent ramp	Off	
Distance to adjacent ramp	1250	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	2380 ✓	850	490	vph
Peak-hour factor, PHF	0.92 ✓	0.92	0.92	
Peak 15-min volume, v15	647 ✓	231	133	v
Trucks and buses	11 ✓	11	11	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00 %	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.948	0.948	0.948	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	2729	975	562	pcph

Estimation of V12 Diverge Areas

$$L = \text{(Equation 25-8 or 25-9)}$$

$$EQ$$

$$P = 0.647 \text{ Using Equation 5}$$

$$FD$$

$$v_{12R} = v_F + (v_R - v_F) P = 2110 \text{ pc/h}$$

Capacity Checks

	Actual	Maximum	LOS F?
$v_{Fi} = v_F$	2729	6750	No
$v_{FO} = v_F - v_R$	1754	6750	No
v_R	975	2000	No
$v_{3 \text{ or } av34}$	619 pc/h	(Equation 25-15 or 25-16)	
Is $v_{3 \text{ or } av34} > 2700 \text{ pc/h?}$		No	
Is $v_{3 \text{ or } av34} > 1.5 v_{12} / 2$		No	
If yes, $v_{12A} = 2110$		(Equation 25-18)	

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v_{12}	2110	4400	No

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v_{12} - 0.009 L_D = 22.4 \text{ pc/mi/ln}$

Level of service for ramp-freeway junction areas of influence C ✓

Speed Estimation

Intermediate speed variable,	$D = 0.516$	
Space mean speed in ramp influence area,	$S_R = 48.3$	mph
Space mean speed in outer lanes,	$S_O = 60.3$	mph
Space mean speed for all vehicles,	$S = 50.6$	mph

HCS+: Ramps and Ramp Junctions Release 5.4

Phone: Fax:
E-mail:

Merge Analysis

Analyst: CTRR
 Agency/Co.: HNTB
 Date performed: 03/07/2010
 Analysis time period: Build Service Road Concept
 Freeway/Dir of Travel: SR 417 EB
 Junction: On Ramp from International
 Jurisdiction: Seminole County
 Analysis Year: 2022
 Description: Wekiva Parkway Project Development & Environment Project

Freeway Data

Type of analysis	Merge		
Number of lanes in freeway	3		
Free-flow speed on freeway	55.0	mph	
Volume on freeway	250	vph	

On Ramp Data

Side of freeway	Right		
Number of lanes in ramp	1		
Free-flow speed on ramp	35.0	mph	
Volume on ramp	890	vph	
Length of first accel/decel lane	750	ft	
Length of second accel/decel lane		ft	

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes		
Volume on adjacent Ramp	850	vph	
Position of adjacent Ramp	Upstream		
Type of adjacent Ramp	Off		
Distance to adjacent Ramp	1964	ft	

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	250	890	850	vph
Peak-hour factor, PHF	0.90	0.90	0.90	

Peak 15-min volume, v ₁₅	69	247	236	v
Trucks and buses	0	0	0	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, f _{HV}	1.000	1.000	1.000	
Driver population factor, f _P	1.00	1.00	1.00	
Flow rate, v _p	278	989	944	pcph

Estimation of V12 Merge Areas

L = 32.34 (Equation 25-2 or 25-3)

EQ

P = 0.599 Using Equation 1

FM

$v = v(P) = 166$ pc/h

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	1267	6750	No
FO			
v v	112 pc/h	(Equation 25-4 or 25-5)	
3 or av ³⁴			
Is v v > 2700 pc/h?		No	
3 or av ³⁴			
Is v v > 1.5 v /2		No	
3 or av ³⁴ 12			
If yes, v = 166		(Equation 25-8)	
12A			

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	166	4600	No
R12			

Level of Service Determination (if not F)

Density, D = $5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 9.3$ pc/mi/ln

R R 12 A

Level of service for ramp-freeway junction areas of influence A

Speed Estimation

Intermediate speed variable, M = 0.281

S

Space mean speed in ramp influence area, S = 51.3 mph

R

Space mean speed in outer lanes, S = 55.0 mph

0

Space mean speed for all vehicles, S = 51.7 mph

HCS+: Ramps and Ramp Junctions Release 5.4

Phone: Fax:
E-mail:

Merge Analysis

Analyst: CTRR
 Agency/Co.: HNTB
 Date performed: 03/07/2010
 Analysis time period: Build Service Road Concept
 Freeway/Dir of Travel: SR 417 EB
 Junction: On Ramp from International
 Jurisdiction: Seminole County
 Analysis Year: 2022
 Description: Wekiva Parkway Project Development & Environment Project

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	3	
Free-flow speed on freeway	55.0	mph
Volume on freeway	250	vph

On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	35.0	mph
Volume on ramp	890	vph
Length of first accel/decel lane	750	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent Ramp	1660	vph
Position of adjacent Ramp	Downstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	3106	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
		Ramp		
Volume, V (vph)	250	890	1660	vph
Peak-hour factor, PHF	0.90	0.90	0.90	

Peak 15-min volume, v_{15}	69	247	461	v
Trucks and buses	0	0	0	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	1.000	1.000	1.000	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, v_p	278	989	1844	pcph

Estimation of V12 Merge Areas

$L =$ (Equation 25-2 or 25-3)

EQ

$P = 0.599$ Using Equation 1

FM

$v = v(P) = 166$ pc/h

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	1267	6750	No
FO			
v v	112 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		No	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34	12		
If yes, $v = 166$		(Equation 25-8)	
12A			

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	166	4600	No
R12			

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 9.3$ pc/mi/ln

R R 12 A

Level of service for ramp-freeway junction areas of influence A

Speed Estimation

Intermediate speed variable, $M = 0.281$

S

Space mean speed in ramp influence area, $S = 51.3$ mph

R

Space mean speed in outer lanes, $S = 55.0$ mph

0

Space mean speed for all vehicles, $S = 51.7$ mph

HCS+: Ramps and Ramp Junctions Release 5.4

Phone: Fax:
E-mail:

Merge Analysis

Analyst: CTRR
 Agency/Co.: HNTB
 Date performed: 03/07/2010
 Analysis time period: Build Service Road Concept
 Freeway/Dir of Travel: SR 417 EB
 Junction: On Ramp from I-4 EB & WB
 Jurisdiction: Seminole County
 Analysis Year: 2022
 Description: Wekiva Parkway Project Development & Environment Project

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	4	
Free-flow speed on freeway	55.0	mph
Volume on freeway	1140	vph

On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	35.0	mph
Volume on ramp	1660	vph
Length of first accel/decel lane	500	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent Ramp	890	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	3106	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
	Ramp			
Volume, V (vph)	1140	1660	890	vph
Peak-hour factor, PHF	0.90	0.90	0.90	

Peak 15-min volume, v ₁₅	317	461	247	v
Trucks and buses	0	0	0	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, f _{HV}	1.000	1.000	1.000	
Driver population factor, f _P	1.00	1.00	1.00	
Flow rate, v _p	1267	1844	989	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)

EQ

P = 0.147 Using Equation 4

FM

v = v (P) = 186 pc/h

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	3111	9000	No
FO			
v v	540 pc/h	(Equation 25-4 or 25-5)	
3 or av ₃₄			
Is v v > 2700 pc/h?		No	
3 or av ₃₄			
Is v v > 1.5 v /2		Yes	
3 or av ₃₄	12		
If yes, v = 506		(Equation 25-8)	
12A			

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	506	4600	No
12A			

Level of Service Determination (if not F)

Density, D = 5.475 + 0.00734 v_R + 0.0078 v_A - 0.00627 L = 19.8 pc/mi/ln

Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable, M = 0.327

S

Space mean speed in ramp influence area, S = 50.8 mph

R

Space mean speed in outer lanes, S = 55.0 mph

0

Space mean speed for all vehicles, S = 51.7 mph

Phone:
E-mail:

Fax:

Diverge Analysis

Analyst: CTR
 Agency/Co.: HNTB
 Date performed: 8/02/2010
 Analysis time period: Build Service Road Concept
 Freeway/Dir of Travel: SR 417 EB
 Junction: Off Ramp to Rinehart Rd
 Jurisdiction: Seminole County
 Analysis Year: 2022
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	4	
Free-flow speed on freeway	55.0	mph
Volume on freeway	4080 ✓	vph

Off Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	35.0 ✓	mph
Volume on ramp	430 ✓	vph
Length of first accel/decel lane	0	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent ramp	1120 ✓	vph
Position of adjacent ramp	Downstream	
Type of adjacent ramp	On	
Distance to adjacent ramp	2402	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	4080 ↓	430	1120	vph
Peak-hour factor, PHF	0.92 ↓	0.92	0.92	
Peak 15-min volume, v15	1109 ↓	117	304	v
Trucks and buses	10 ↓	10	9	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00 %	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.952	0.952	0.957	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	4657	491	1272	pcph

Estimation of V12 Diverge Areas

$$L = \text{EQ} \quad (\text{Equation 25-8 or 25-9})$$

$$P = 0.436 \quad \text{Using Equation 8}$$

$$v_{12R} = v_{FR} + (v_{FR} - v_{RD}) P_{FD} = 2307 \quad \text{pc/h}$$

Capacity Checks

	Actual	Maximum	LOS F?
$v_{Fi} = v_F$	4657	9000	No
$v_{FO} = v_F - v_R$	4166	9000	No
v_R	491	2000	No
$v_{3 \text{ or } av34}$	1175 pc/h	(Equation 25-15 or 25-16)	
Is $v_{3 \text{ or } av34} > 2700 \text{ pc/h?}$		No	
Is $v_{3 \text{ or } av34} > 1.5 v_{12} / 2$		No	
If yes, $v_{12A} = 2307$		(Equation 25-18)	

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v_{12}	2307	4400	No

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v_{12} - 0.009 L_D = 24.1 \text{ pc/mi/ln}$

Level of service for ramp-freeway junction areas of influence C ✓

Speed Estimation

Intermediate speed variable,	$D = 0.472$	
Space mean speed in ramp influence area,	$S_R = 48.9$	mph
Space mean speed in outer lanes,	$S_0 = 59.7$	mph
Space mean speed for all vehicles,	$S = 53.8$	mph

Phone: Fax:
 E-mail:

Diverge Analysis

Analyst: CTR
 Agency/Co.: HNTB
 Date performed: 8/02/2010
 Analysis time period: Build Service Road Concept
 Freeway/Dir of Travel: SR 417 EB
 Junction: Off Ramp to Rinehart Rd
 Jurisdiction: Seminole County
 Analysis Year: 2022
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	4	
Free-flow speed on freeway	55.0 ✓	mph
Volume on freeway	4080 ✓	vph

Off Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	35.0 ✓	mph
Volume on ramp	430 ✓	vph
Length of first accel/decel lane	0	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes ✓	
Volume on adjacent ramp	1660 ✓	vph
Position of adjacent ramp	Upstream	
Type of adjacent ramp	On	
Distance to adjacent ramp	3000	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	4080 ✓	430	1660	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	1109 ✓	117	451	v
Trucks and buses	10 ✓	10	9	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00 %	0.00 %	0.00 %	
Length	0.00 mi	0.00 mi	0.00 mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.952	0.952	0.957	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	4657	491	1886	pcph

Estimation of V12 Diverge Areas

$$L = \text{(Equation 25-8 or 25-9)}$$

$$EQ$$

$$P = 0.436 \text{ Using Equation 8}$$

$$FD$$

$$v_{12R} = v_F + (v_F - v_R) P_{FD} = 2307 \text{ pc/h}$$

Capacity Checks

	Actual	Maximum	LOS F?
$v_{Fi} = v_F$	4657	9000	No
$v_{FO} = v_F - v_R$	4166	9000	No
v_R	491	2000	No
$v_{3 \text{ or } av34}$	1175 pc/h	(Equation 25-15 or 25-16)	
Is $v_{3 \text{ or } av34} > 2700 \text{ pc/h?}$		No	
Is $v_{3 \text{ or } av34} > 1.5 v_{12} / 2$		No	
If yes, $v_{12A} = 2307$		(Equation 25-18)	

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v_{12}	2307	4400	No

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v_{12} - 0.009 L_D = 24.1 \text{ pc/mi/ln}$

Level of service for ramp-freeway junction areas of influence C ✓

Speed Estimation

Intermediate speed variable,	$D = 0.472$	
Space mean speed in ramp influence area,	$S_R = 48.9$	mph
Space mean speed in outer lanes,	$S_O = 59.7$	mph
Space mean speed for all vehicles,	$S = 53.8$	mph

Phone: Fax:
 E-mail:

Merge Analysis

Analyst: CTR
 Agency/Co.: HNTB
 Date performed: 8/02/2010
 Analysis time period: Build Service Road Concept
 Freeway/Dir of Travel: SR 417 EB
 Junction: On Ramp from Rinehart Rd
 Jurisdiction: Seminole County
 Analysis Year: 2022
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	3	
Free-flow speed on freeway	55.0	mph
Volume on freeway	3650 ✓	vph

On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	35.0	mph
Volume on ramp	1120 ✓	vph
Length of first accel/decel lane	1000 ✓	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent Ramp	430 ✓	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	Off	
Distance to adjacent Ramp	2402	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	3650 ✓	1120	430	vph
Peak-hour factor, PHF	0.92 ✓	0.92	0.92	
Peak 15-min volume, v15	992 ✓	304	117	v
Trucks and buses	10	10	10	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.952	0.952	0.952	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	4166	1278	491	pcph

Estimation of V12 Merge Areas

L = 1037.22 (Equation 25-2 or 25-3)
EQ
P = 0.605 Using Equation 1
FM
 $v_{12} = v_{F \text{ FM}} (P) = 2523 \text{ pc/h}$

Capacity Checks

	Actual	Maximum	LOS F?
v _{FO}	5444	6750	No
v _{3 or av34}	1643 pc/h	(Equation 25-4 or 25-5)	
Is v _{3 or av34} > 2700 pc/h?		No	
Is v _{3 or av34} > 1.5 v ₁₂ / 2		No	
If yes, v _{12A} = 2523		(Equation 25-8)	

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v _{R12}	2523	4600	No

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 28.3 \text{ pc/mi/ln}$
Level of service for ramp-freeway junction areas of influence D ✓

Speed Estimation

Intermediate speed variable,	M = 0.426	
Space mean speed in ramp influence area,	S = 49.5	mph
Space mean speed in outer lanes,	S = 50.9	mph
Space mean speed for all vehicles,	S = 49.9	mph

Phone: Fax:
E-mail:

Merge Analysis

Analyst: CTR
 Agency/Co.: HNTB
 Date performed: 08/02/2010
 Analysis time period: Build
 Freeway/Dir of Travel: I-4 EB
 Junction: On Ramp from US 1792
 Jurisdiction: Seminole County
 Analysis Year: 2022
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	3	
Free-flow speed on freeway	70.0	mph
Volume on freeway	5450 ✓	vph

On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	35.0	mph
Volume on ramp	500 ✓	vph
Length of first accel/decel lane	900	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes ↓	
Volume on adjacent Ramp	940 ↓	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	Off	
Distance to adjacent Ramp	1948	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	5450 ✓	500	940	vph
Peak-hour factor, PHF	0.92 ✓	0.92	0.92	
Peak 15-min volume, v15	1481	136	255	v
Trucks and buses	9 ↓	9	9	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade		%	%	%
Length		mi	mi	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.957	0.957	0.957	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	6190	568	1068	pcph

Estimation of V12 Merge Areas

$$L = 1274.01 \text{ (Equation 25-2 or 25-3)}$$

$$EQ$$

$$P = 0.603 \text{ Using Equation 1}$$

$$FM$$

$$v_{12} = v_F (P_{FM}) = 3731 \text{ pc/h}$$

Capacity Checks

	Actual	Maximum	LOS F?
v _{FO}	6758	7200	No
v _{3 or av34}	2459 pc/h	(Equation 25-4 or 25-5)	
Is v _{3 or av34} > 2700 pc/h?		No	
Is v _{3 or av34} > 1.5 v ₁₂ / 2		No	
If yes, v _{12A} = 3731		(Equation 25-8)	

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v _{R12}	3731	4600	No

Level of Service Determination (if not F)

$$\text{Density, } D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 33.1 \text{ pc/mi/ln}$$

Level of service for ramp-freeway junction areas of influence D ✓

Speed Estimation

Intermediate speed variable,	M = 0.545	
Space mean speed in ramp influence area,	S _R = 54.7	mph
Space mean speed in outer lanes,	S ₀ = 62.5	mph
Space mean speed for all vehicles,	S = 57.3	mph

Phone: Fax:
E-mail:

Diverge Analysis

Analyst: CTR
 Agency/Co.: HNTB
 Date performed: 8/02/2010
 Analysis time period: Build
 Freeway/Dir of Travel: I-4 WB
 Junction: Off Ramp to US 1792
 Jurisdiction: Seminole County
 Analysis Year: 2022
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	3	
Free-flow speed on freeway	70.0	mph
Volume on freeway	5950	vph

Off Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	35.0	mph
Volume on ramp	500	vph
Length of first accel/decel lane	500	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent ramp	940	vph
Position of adjacent ramp	Downstream	
Type of adjacent ramp	On	
Distance to adjacent ramp	1948	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	5950	500	940	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	1617	136	255	v
Trucks and buses	9	9	9	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00 %	0.00 %	0.00 %	
Length	0.00 mi	0.00 mi	0.00 mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.957	0.957	0.957	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	6758	568	1068	pcph

Estimation of V12 Diverge Areas

$$L = \text{(Equation 25-8 or 25-9)}$$

$$EQ$$

$$P = 0.565 \text{ Using Equation 5}$$

$$FD$$

$$v_{12R} = v_F + (v_F - v_R) P = 4065 \text{ pc/h}$$

Capacity Checks

	Actual	Maximum	LOS F?
$v_{Fi} = v_F$	6758	7200	No
$v_{FO} = v_F - v_R$	6190	7200	No
v_R	568	2000	No
$v_{3 \text{ or } av34}$	2693 pc/h	(Equation 25-15 or 25-16)	
Is $v_{3 \text{ or } av34} > 2700 \text{ pc/h?}$		No	
Is $v_{3 \text{ or } av34} > 1.5 v_{12} / 2$		No	
If yes, $v_{12A} = 4065$		(Equation 25-18)	

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v_{12}	4065	4400	No

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v_{12} - 0.009 L_D = 34.7 \text{ pc/mi/ln}$

Level of service for ramp-freeway junction areas of influence D

Speed Estimation

Intermediate speed variable,	$D = 0.479$	
	S	
Space mean speed in ramp influence area,	$S = 56.6$	mph
	R	
Space mean speed in outer lanes,	$S = 70.2$	mph
	O	
Space mean speed for all vehicles,	$S = 61.3$	mph

Phone:
E-mail:

Fax:

Diverge Analysis

Analyst: CTR
 Agency/Co.: HNTB
 Date performed: 8/2/2010
 Analysis time period: Build Service Road Concept
 Freeway/Dir of Travel: I-4 EB
 Junction: Off Ramp to CR 46A
 Jurisdiction: Seminole County
 Analysis Year: 2022
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	4 ✓	
Free-flow speed on freeway	55.0	mph
Volume on freeway	6000 ✓	vph

Off Ramp Data

Side of freeway	Right	
Number of lanes in ramp	2 ✓	
Free-Flow speed on ramp	35.0	mph
Volume on ramp	1520 ✓	vph
Length of first accel/decel lane	0	ft
Length of second accel/decel lane	1500 ✓	ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes ✓	
Volume on adjacent ramp	1730 ✓	vph
Position of adjacent ramp	Downstream	
Type of adjacent ramp	Off	
Distance to adjacent ramp	1906	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	6000 ✓	1520	1730	vph
Peak-hour factor, PHF	0.92 ✓	0.92	0.92	
Peak 15-min volume, v15	1630	413	470	v
Trucks and buses	9 ✓	9	9	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00 %	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5*	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.957	0.957	0.957	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	6815	1727	1965	pcph

Estimation of V12 Diverge Areas

$$L = \text{EQ} \quad (\text{Equation 25-8 or 25-9})$$

$$P = 0.260 \quad \text{Using Equation 0}$$

$$v_{12R} = v_{FR} + (v_{FR} - v_{FD}) P = 3050 \text{ pc/h}$$

Capacity Checks

	Actual	Maximum	LOS F?
$v_{Fi} = v_F$	6815	9000	No
$v_{FO} = v_F - v_R$	5088	9000	No
v_R	1727	3800	No
$v_{3 \text{ or } av34}$	1882 pc/h	(Equation 25-15 or 25-16)	
Is $v_{3 \text{ or } av34} > 2700 \text{ pc/h?}$		No	
Is $v_{3 \text{ or } av34} > 1.5 v_{12} / 2$		No	
If yes, $v_{12A} = 3050$		(Equation 25-18)	

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v_{12}	3050	4400	No

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v_{12} - 0.009 L_D = 17.0 \text{ pc/mi/ln}$

Level of service for ramp-freeway junction areas of influence B ✓

Speed Estimation

Intermediate speed variable,	$D = 0.583$	
Space mean speed in ramp influence area,	$S_R = 47.4$	mph
Space mean speed in outer lanes,	$S_0 = 56.9$	mph
Space mean speed for all vehicles,	$S = 52.2$	mph

HCS+: Ramps and Ramp Junctions Release 5.4

Phone: Fax:
E-mail:

Diverge Analysis

Analyst: CTRR
 Agency/Co.: HNTB
 Date performed: 3/05/2010
 Analysis time period: Build Service Road Concept
 Freeway/Dir of Travel: I-4 EB
 Junction: Off Ramp to SR 417 & SR 46
 Jurisdiction: Seminole County
 Analysis Year: 2022
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	3	
Free-flow speed on freeway	55.0	mph
Volume on freeway	4480	vph

Off Ramp Data

Side of freeway	Right	
Number of lanes in ramp	2	
Free-Flow speed on ramp	35.0	mph
Volume on ramp	1730	vph
Length of first accel/decel lane	406	ft
Length of second accel/decel lane	1500	ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent ramp	1520	vph
Position of adjacent ramp	Upstream	
Type of adjacent ramp	Off	
Distance to adjacent ramp	1906	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	4480	1730	1520	vph
Peak-hour factor, PHF	0.90	0.90	0.90	

Peak 15-min volume, v15	1244	481	422	v
Trucks and buses	0	0	0	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00 %	0.00 %	0.00 %	
Length	0.00 mi	0.00 mi	0.00 mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	1.000	1.000	1.000	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	4978	1922	1689	pcph

Estimation of V12 Diverge Areas

L = (Equation 25-8 or 25-9)

EQ

P = 0.450 Using Equation 0

FD

v = v + (v - v) P = 3297 pc/h

12 R F R FD

Capacity Checks

	Actual	Maximum	LOS F?
v = v	4978	6750	No
Fi F			
v = v - v	3056	6750	No
FO F R			
v	1922	3800	No
R			
v v	1681 pc/h	(Equation 25-15 or 25-16)	
3 or av34			
Is v v > 2700 pc/h?		No	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34	12		
If yes, v = 3297		(Equation 25-18)	
12A			

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v	3297	4400	No
12			

Level of Service Determination (if not F)

Density, D = 4.252 + 0.0086 v - 0.009 L = 11.8 pc/mi/ln

R 12 D

Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable, D = 0.601

S

Space mean speed in ramp influence area, S = 47.2 mph

	R	
Space mean speed in outer lanes,		S = 57.7 mph
	0	
Space mean speed for all vehicles,		S = 50.3 mph

HCS+: Ramps and Ramp Junctions Release 5.4

Phone: Fax:
E-mail:

Diverge Analysis

Analyst: CTRR
 Agency/Co.: HNTB
 Date performed: 3/07/2010
 Analysis time period: Build Service Road Concept
 Freeway/Dir of Travel: I-4 EB
 Junction: Off Ramp to SR 417 & SR 46
 Jurisdiction: Seminole County
 Analysis Year: 2022
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	3	
Free-flow speed on freeway	55.0	mph
Volume on freeway	4480	vph

Off Ramp Data

Side of freeway	Right	
Number of lanes in ramp	2	
Free-Flow speed on ramp	35.0	mph
Volume on ramp	1730	vph
Length of first accel/decel lane	406	ft
Length of second accel/decel lane	1500	ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent ramp	610	vph
Position of adjacent ramp	Downstream	
Type of adjacent ramp	On	
Distance to adjacent ramp	1410	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
		Ramp		
Volume, V (vph)	4480	1730	610	vph
Peak-hour factor, PHF	0.90	0.90	0.90	

Peak 15-min volume, v ₁₅	1244	481	169	v
Trucks and buses	0	0	0	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00 %	0.00 %	0.00 %	
Length	0.00 mi	0.00 mi	0.00 mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, f _{HV}	1.000	1.000	1.000	
Driver population factor, f _P	1.00	1.00	1.00	
Flow rate, v _p	4978	1922	678	pcph

Estimation of V12 Diverge Areas

L = (Equation 25-8 or 25-9)

EQ

P = 0.450 Using Equation 0

FD

$v = v + (v - v) P = 3297$ pc/h

12 R F R FD

Capacity Checks

	Actual	Maximum	LOS F?
v = v	4978	6750	No
F _i F			
v = v - v	3056	6750	No
F _O F R			
v	1922	3800	No
R			
v v	1681 pc/h	(Equation 25-15 or 25-16)	
3 or av ₃₄			
Is v v > 2700 pc/h?		No	
3 or av ₃₄			
Is v v > 1.5 v /2		No	
3 or av ₃₄ 12			
If yes, v = 3297		(Equation 25-18)	
12A			

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v	3297	4400	No
12			

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v - 0.009 L = 11.8$ pc/mi/ln

R 12 D

Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable, $D = 0.601$

S

Space mean speed in ramp influence area, $S = 47.2$ mph

Space mean speed in outer lanes,	R	S = 57.7	mph
Space mean speed for all vehicles,	0	S = 50.3	mph

Phone:
E-mail:

Fax:

Merge Analysis

Analyst: CTR
 Agency/Co.: HNTB
 Date performed: 8/2/2010
 Analysis time period: Build Service Road Concept
 Freeway/Dir of Travel: I-4 EB
 Junction: On Ramp from CR 46A
 Jurisdiction: Seminole County
 Analysis Year: 2022
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	3	
Free-flow speed on freeway	55.0	mph
Volume on freeway	2750 ✓	vph

On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	35.0	mph
Volume on ramp	610 ✓	vph
Length of first accel/decel lane	700	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent Ramp	1160 ✓	vph
Position of adjacent Ramp	Downstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	4826	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	2750 ↓	610	1160	vph
Peak-hour factor, PHF	0.92 ↓	0.92	0.92	
Peak 15-min volume, v15	747	166	315	v
Trucks and buses	9 ↓	9	10	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.957	0.957	0.952	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	3124	693	1324	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)
EQ
P = 0.597 Using Equation 1
FM
 $v_{12} = v_F (P_{FM}) = 1865 \text{ pc/h}$

Capacity Checks

	v	Actual	Maximum	LOS F?
	FO	3817	6750	No
	v _{3 or av34}	1259 pc/h	(Equation 25-4 or 25-5)	
Is	v _{3 or av34}	> 2700 pc/h?	No	
Is	v _{3 or av34}	> 1.5 v ₁₂ / 2	No	
If yes,	v _{12A}	= 1865	(Equation 25-8)	

Flow Entering Merge Influence Area

	v	Actual	Max Desirable	Violation?
	R12	1865	4600	No

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 20.7 \text{ pc/mi/ln}$
Level of service for ramp-freeway junction areas of influence C ✓

Speed Estimation

Intermediate speed variable,	M = 0.322	
Space mean speed in ramp influence area,	S _R = 50.8	mph
Space mean speed in outer lanes,	S _O = 52.3	mph
Space mean speed for all vehicles,	S ₀ = 51.3	mph

Phone: Fax:
E-mail:

Merge Analysis

Analyst: CTR
 Agency/Co.: HNTB
 Date performed: 8/2/2010
 Analysis time period: Build Service Road Concept
 Freeway/Dir of Travel: I-4 EB
 Junction: On Ramp from CR 46A
 Jurisdiction: Seminole County
 Analysis Year: 2022
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	3	
Free-flow speed on freeway	55.0	mph
Volume on freeway	2750 ✓	vph

On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	35.0	mph
Volume on ramp	610 ✓	vph
Length of first accel/decel lane	700	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent Ramp	1730 ✓	vph
Position of adjacent Ramp	Upstream ✓	
Type of adjacent Ramp	Off	
Distance to adjacent Ramp	1410	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	2750 ✓	610	1730	vph
Peak-hour factor, PHF	0.92 ✓	0.92	0.92	
Peak 15-min volume, v15	74 ✓	166	470	v
Trucks and buses	9 ✓	9	9	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.957	0.957	0.957	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	3124	693	1965	pcph

Estimation of V12 Merge Areas

$$L = 555.84 \text{ (Equation 25-2 or 25-3)}$$

$$EQ$$

$$P = 0.597 \text{ Using Equation 1}$$

$$FM$$

$$v_{12} = v_F (P_{FM}) = 1865 \text{ pc/h}$$

Capacity Checks

	Actual	Maximum	LOS F?
v _{FO}	3817	6750	No
v _{3 or av34}	1259 pc/h	(Equation 25-4 or 25-5)	
Is v _{3 or av34} > 2700 pc/h?		No	
Is v _{3 or av34} > 1.5 v ₁₂ / 2		No	
If yes, v _{12A} = 1865		(Equation 25-8)	

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v _{R12}	1865	4600	No

Level of Service Determination (if not F)

$$Density, D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 20.7 \text{ pc/mi/ln}$$

Level of service for ramp-freeway junction areas of influence C ✓

Speed Estimation

Intermediate speed variable,	M = 0.322	
Space mean speed in ramp influence area,	S = 50.8	mph
Space mean speed in outer lanes,	S = 52.3	mph
Space mean speed for all vehicles,	S = 51.3	mph

Phone: Fax:
 E-mail:

Merge Analysis

Analyst: CTR
 Agency/Co.: HNTB
 Date performed: 8/2/2010
 Analysis time period: Build Service Road Concept
 Freeway/Dir of Travel: I-4 EB
 Junction: On Ramp from SR 417 WB
 Jurisdiction: Seminole County
 Analysis Year: 2022
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	3	
Free-flow speed on freeway	55.0	mph
Volume on freeway	3360 ✓	vph

On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	2	
Free-flow speed on ramp	35.0	mph
Volume on ramp	1160 ✓	vph
Length of first accel/decel lane	900	ft
Length of second accel/decel lane	1500	ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent Ramp	690 ✓	vph
Position of adjacent Ramp	Downstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1976	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	3360 ✓	1160	690	vph
Peak-hour factor, PHF	0.92 ✓	0.92	0.92	
Peak 15-min volume, v15	913 ✓	315	187	v
Trucks and buses	9 ✓	10	9	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.957	0.952	0.957	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	3817	1324	784	pcph

Estimation of V12 Merge Areas

$$L = \text{EQ} \quad (\text{Equation 25-2 or 25-3})$$

$$P = 0.555 \quad \text{Using Equation 0}$$

$$v_{12} = v_{F \text{ FM}} (P) = 2118 \quad \text{pc/h}$$

Capacity Checks

v _{FO}	Actual	Maximum	LOS F?
	5141	6750	No
v _{3 or av34}	1699 pc/h	(Equation 25-4 or 25-5)	
Is v _{3 or av34} > 2700 pc/h?		No	
Is v _{3 or av34} > 1.5 v ₁₂ / 2		Yes	
If yes, v _{12A} = 2181		(Equation 25-8)	

Flow Entering Merge Influence Area

v _{12A}	Actual	Max Desirable	Violation?
	2181	4600	No

Level of Service Determination (if not F)

$$\text{Density, } D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 11.5 \quad \text{pc/mi/ln}$$

Level of service for ramp-freeway junction areas of influence B ✓

Speed Estimation

Intermediate speed variable,	M = 0.220	
Space mean speed in ramp influence area,	S = 52.1	mph
Space mean speed in outer lanes,	S = 50.9	mph
Space mean speed for all vehicles,	S = 51.7	mph

Phone: Fax:
E-mail:

Merge Analysis

Analyst: CTR
 Agency/Co.: HNTB
 Date performed: 8/2/2010
 Analysis time period: Build Service Road Concept
 Freeway/Dir of Travel: I-4 EB
 Junction: On Ramp from SR 417 WB
 Jurisdiction: Seminole County
 Analysis Year: 2022
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	3	
Free-flow speed on freeway	55.0 ✓	mph
Volume on freeway	3360 ✓	vph

On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	2	
Free-flow speed on ramp	35.0	mph
Volume on ramp	1160 ✓	vph
Length of first accel/decel lane	900	ft
Length of second accel/decel lane	1500	ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent Ramp	610 ✓	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	4826	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	3360 ✓	1160	610	vph
Peak-hour factor, PHF	0.92 ✓	0.92	0.92	
Peak 15-min volume, v15	913 ✓	315	166	v
Trucks and buses	9 ✓	10	9	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade		%	%	%
Length		mi	mi	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.957	0.952	0.957	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	3817	1324	693	pcph

Estimation of V12 Merge Areas

$$L = \text{(Equation 25-2 or 25-3)}$$

$$EQ$$

$$P = 0.555 \text{ Using Equation 0}$$

$$FM$$

$$v_{12} = v_F (P_{FM}) = 2118 \text{ pc/h}$$

Capacity Checks

	Actual	Maximum	LOS F?
v _{FO}	5141	6750	No
v _{3 or av34}	1699 pc/h	(Equation 25-4 or 25-5)	
Is v _{3 or av34} > 2700 pc/h?		No	
Is v _{3 or av34} > 1.5 v ₁₂ / 2		Yes	
If yes, v _{12A} = 2181		(Equation 25-8)	

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v _{12A}	2181	4600	No

Level of Service Determination (if not F)

$$\text{Density, } D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 11.5 \text{ pc/mi/ln}$$

Level of service for ramp-freeway junction areas of influence B ✓

Speed Estimation

Intermediate speed variable,	M = 0.220	
Space mean speed in ramp influence area,	S = 52.1	mph
Space mean speed in outer lanes,	S = 50.9	mph
Space mean speed for all vehicles,	S = 51.7	mph

Phone: _____ Fax: _____
 E-mail: _____

Merge Analysis

Analyst: CTR
 Agency/Co.: HNTB
 Date performed: 8/2/2010
 Analysis time period: Build Service Road Concept
 Freeway/Dir of Travel: I-4 EB
 Junction: On Ramp from SR 417 EB
 Jurisdiction: Seminole County
 Analysis Year: 2022
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	3	
Free-flow speed on freeway	55.0	mph
Volume on freeway	4520 ✓	vph

On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	35.0	mph
Volume on ramp	690 ✓	vph
Length of first accel/decel lane	900	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent Ramp	1180 ✓	vph
Position of adjacent Ramp	Downstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	3172	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	4520 ✓	690	1180	vph
Peak-hour factor, PHF	0.92 ✓	0.92	0.92	
Peak 15-min volume, v15	1228	187	321	v
Trucks and buses	9 ✓	9	9	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.957	0.957	0.957	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	5134	784	1340	pcph

Estimation of V12 Merge Areas

$$L = \text{(Equation 25-2 or 25-3)}$$

$$EQ$$

$$P = 0.603 \text{ Using Equation 1}$$

$$FM$$

$$v_{12} = v_{F \text{ FM}} (P) = 3094 \text{ pc/h}$$

Capacity Checks

	Actual	Maximum	LOS F?
v _{FO}	5918	6750	No
v _{3 or av34}	2040 pc/h	(Equation 25-4 or 25-5)	
Is v _{3 or av34} > 2700 pc/h?		No	
Is v _{3 or av34} > 1.5 v ₁₂ / 2		No	
If yes, v _{12A} = 3094		(Equation 25-8)	

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v _{R12}	3094	4600	No

Level of Service Determination (if not F)

$$\text{Density, } D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 29.7 \text{ pc/mi/ln}$$

Level of service for ramp-freeway junction areas of influence D ✓

Speed Estimation

Intermediate speed variable,	M = 0.446	
Space mean speed in ramp influence area,	S _R = 49.2	mph
Space mean speed in outer lanes,	S ₀ = 49.5	mph
Space mean speed for all vehicles,	S = 49.3	mph

Phone: _____ Fax: _____
 E-mail: _____

Merge Analysis

Analyst: CTR
 Agency/Co.: HNTB
 Date performed: 8/2/2010
 Analysis time period: Build Service Road Concept
 Freeway/Dir of Travel: I-4 EB
 Junction: On Ramp from SR 417 EB
 Jurisdiction: Seminole County
 Analysis Year: 2022
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	3	
Free-flow speed on freeway	55.0	mph
Volume on freeway	4520 ✓	vph

On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	35.0	mph
Volume on ramp	690 ✓	vph
Length of first accel/decel lane	900	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent Ramp	1160 ✓	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1976	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	4520 ✓	690	1160	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	1228 ✓	187	315	v
Trucks and buses	9	9	9	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.957	0.957	0.957	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	5134	784	1318	pcph

Estimation of V12 Merge Areas

$$L = \text{EQ} \quad (\text{Equation 25-2 or 25-3})$$

$$P = 0.603 \quad \text{Using Equation 1}$$

$$v_{12} = v_F (P_{FM}) = 3094 \quad \text{pc/h}$$

Capacity Checks

	Actual	Maximum	LOS F?
v _{FO}	5918	6750	No
v _{3 or av34}	2040 pc/h	(Equation 25-4 or 25-5)	
Is v _{3 or av34} > 2700 pc/h?		No	
Is v _{3 or av34} > 1.5 v ₁₂ / 2		No	
If yes, v _{12A} = 3094		(Equation 25-8)	

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v _{R12}	3094	4600	No

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 29.7$ pc/mi/ln

Level of service for ramp-freeway junction areas of influence $D \checkmark$

Speed Estimation

Intermediate speed variable,	M = 0.446	
Space mean speed in ramp influence area,	S = 49.2	mph
Space mean speed in outer lanes,	S = 49.5	mph
Space mean speed for all vehicles,	S = 49.3	mph

HCS+: Ramps and Ramp Junctions Release 5.4

Phone: Fax:
E-mail:

Merge Analysis

Analyst: CTRR
Agency/Co.: HNTB
Date performed: 3/07/2010
Analysis time period: Build Service Road Concept
Freeway/Dir of Travel: I-4 EB
Junction: On Ramp from SR 46
Jurisdiction: Seminole County
Analysis Year: 2022
Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Merge		
Number of lanes in freeway	4		
Free-flow speed on freeway	55.0	mph	
Volume on freeway	5210	vph	

On Ramp Data

Side of freeway	Right		
Number of lanes in ramp	2		
Free-flow speed on ramp	35.0	mph	
Volume on ramp	1180	vph	
Length of first accel/decel lane	500	ft	
Length of second accel/decel lane	500	ft	

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes		
Volume on adjacent Ramp	690	vph	
Position of adjacent Ramp	Upstream		
Type of adjacent Ramp	On		
Distance to adjacent Ramp	3172	ft	

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
Volume, V (vph)	5210	1180	690	vph
Peak-hour factor, PHF	0.90	0.90	0.90	

Peak 15-min volume, v ₁₅	1447	328	192	v
Trucks and buses	0	0	0	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, f _{HV}	1.000	1.000	1.000	
Driver population factor, f _P	1.00	1.00	1.00	
Flow rate, v _p	5789	1311	767	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)

EQ

P = 0.209 Using Equation 0

FM

v = v (P) = 1210 pc/h

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	7100	9000	No
FO			
v v	2289 pc/h	(Equation 25-4 or 25-5)	
3 or av ₃₄			
Is v v > 2700 pc/h?		No	
3 or av ₃₄			
Is v v > 1.5 v /2		Yes	
3 or av ₃₄ 12			
If yes, v = 2315		(Equation 25-8)	
12A			

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	2315	4600	No
12A			

Level of Service Determination (if not F)

Density, D = 5.475 + 0.00734 v_R + 0.0078 v_A - 0.00627 L = 23.7 pc/mi/ln

Level of service for ramp-freeway junction areas of influence C

Speed Estimation

Intermediate speed variable, M = 0.362

S

Space mean speed in ramp influence area, S = 50.3 mph

R

Space mean speed in outer lanes, S = 50.5 mph

0

Space mean speed for all vehicles, S = 50.4 mph

Phone: _____ Fax: _____
 E-mail: _____

Diverge Analysis

Analyst: CTR
 Agency/Co.: HNTB
 Date performed: 8/2/2010
 Analysis time period: Build Service Road Concept
 Freeway/Dir of Travel: I-4 WB
 Junction: Off Ramp to SR 46
 Jurisdiction: Seminole County
 Analysis Year: 2022
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	4	
Free-flow speed on freeway	55.0	mph
Volume on freeway	6390 ✓	vph

Off Ramp Data

Side of freeway	Right	
Number of lanes in ramp	2	
Free-Flow speed on ramp	35.0	mph
Volume on ramp	1180 ✓	vph
Length of first accel/decel lane	0	ft
Length of second accel/decel lane	500	ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent ramp	2460 ✓	vph
Position of adjacent ramp	Downstream	
Type of adjacent ramp	Off	
Distance to adjacent ramp	6098	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	6390 ✓	1180	2460	vph
Peak-hour factor, PHF	0.92 ✓	0.92	0.92	
Peak 15-min volume, v15	1736 ✓	321	668	v
Trucks and buses	9 ✓	9	9	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00 %	0.00 %	0.00 %	
Length	0.00 mi	0.00 mi	0.00 mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.957	0.957	0.957	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	7258	1340	2794	pcph

Estimation of V12 Diverge Areas

$$L = \text{(Equation 25-8 or 25-9)}$$

$$EQ$$

$$P = 0.260 \quad \text{Using Equation } 0$$

$$FD$$

$$v_{12R} = v_F + (v_R - v_{FD}) P = 2879 \quad \text{pc/h}$$

Capacity Checks

	Actual	Maximum	LOS F?
$v_{Fi} = v_F$	7258	9000	No
$v_{FO} = v_F - v_R$	5918	9000	No
v_R	1340	3800	No
$v_{3 \text{ or } av34}$	2189 pc/h	(Equation 25-15 or 25-16)	
Is $v_{3 \text{ or } av34} > 2700 \text{ pc/h?}$		No	
Is $v_{3 \text{ or } av34} > 1.5 v_{12} / 2$		Yes	
If yes, $v_{12A} = 2903$		(Equation 25-18)	

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v_{12A}	2903	4400	No

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v_{12R} - 0.009 L_D = 24.7 \quad \text{pc/mi/ln}$

Level of service for ramp-freeway junction areas of influence C ✓

Speed Estimation

Intermediate speed variable,	$D = 0.549$	
Space mean speed in ramp influence area,	$S_R = 47.9$	mph
Space mean speed in outer lanes,	$S_0 = 55.7$	mph
Space mean speed for all vehicles,	$S = 52.3$	mph

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Phone: Fax:
E-mail:

Diverge Analysis

Analyst: CTRR
 Agency/Co.: HNTB
 Date performed: 3/07/2010
 Analysis time period: Build Service Road Concept
 Freeway/Dir of Travel: I-4 WB
 Junction: Off Ramp to SR 417 & CR 46A
 Jurisdiction: Seminole County
 Analysis Year: 2022
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	4	
Free-flow speed on freeway	55.0	mph
Volume on freeway	4730	vph

Off Ramp Data

Side of freeway	Right	
Number of lanes in ramp	2	
Free-Flow speed on ramp	35.0	mph
Volume on ramp	2200	vph
Length of first accel/decel lane	500	ft
Length of second accel/decel lane	500	ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent ramp	1180	vph
Position of adjacent ramp	Upstream	
Type of adjacent ramp	Off	
Distance to adjacent ramp	6098	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
		Ramp		
Volume, V (vph)	4730	2200	1180	vph
Peak-hour factor, PHF	0.90	0.90	0.90	

Peak 15-min volume, v ₁₅	1314	611	328	v
Trucks and buses	0	0	0	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00 %	0.00 %	0.00 %	
Length	0.00 mi	0.00 mi	0.00 mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, f _{HV}	1.000	1.000	1.000	
Driver population factor, f _P	1.00	1.00	1.00	
Flow rate, v _p	5256	2444	1311	pcph

Estimation of V12 Diverge Areas

L = (Equation 25-8 or 25-9)

EQ

P = 0.260 Using Equation 0

FD

$v = v + (v - v) P = 3175$ pc/h

12 R F R FD

Capacity Checks

	Actual	Maximum	LOS F?
v = v	5256	9000	No
F _i F			
v = v - v	2812	9000	No
F _O F R			
v	2444	3800	No
R			
v v	1040 pc/h	(Equation 25-15 or 25-16)	
3 or av ₃₄			
Is v v > 2700 pc/h?		No	
3 or av ₃₄			
Is v v > 1.5 v /2		No	
3 or av ₃₄ 12			
If yes, v = 3175		(Equation 25-18)	
12A			

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v	3175	4400	No
12			

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v - 0.009 L = 18.1$ pc/mi/ln

R 12 D

Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable, $D = 0.648$

S

Space mean speed in ramp influence area, $S = 46.6$ mph

Space mean speed in outer lanes,	$\frac{R}{0}$	S = 60.2	mph
Space mean speed for all vehicles,		S = 51.2	mph

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Phone: Fax:
E-mail:

Diverge Analysis

Analyst: CTRR
Agency/Co.: HNTB
Date performed: 3/07/2010
Analysis time period: Build Service Road Concept
Freeway/Dir of Travel: I-4 WB
Junction: Off Ramp to SR 417 & CR 46A
Jurisdiction: Seminole County
Analysis Year: 2022
Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	4	
Free-flow speed on freeway	55.0	mph
Volume on freeway	4730	vph

Off Ramp Data

Side of freeway	Right	
Number of lanes in ramp	2	
Free-Flow speed on ramp	35.0	mph
Volume on ramp	2200	vph
Length of first accel/decel lane	500	ft
Length of second accel/decel lane	500	ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent ramp	590	vph
Position of adjacent ramp	Downstream	
Type of adjacent ramp	On	
Distance to adjacent ramp	3490	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
Volume, V (vph)	4730	2200	590 vph
Peak-hour factor, PHF	0.90	0.90	0.90

Peak 15-min volume, v ₁₅	1314	611	164	v
Trucks and buses	0	0	0	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00 %	0.00 %	0.00 %	
Length	0.00 mi	0.00 mi	0.00 mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, f _{HV}	1.000	1.000	1.000	
Driver population factor, f _P	1.00	1.00	1.00	
Flow rate, v _p	5256	2444	656	pcph

_____ Estimation of V12 Diverge Areas _____

L = (Equation 25-8 or 25-9)
EQ
P = 0.260 Using Equation 0
FD
 $v = v + (v - v) P = 3175 \text{ pc/h}$
12 R F R FD

_____ Capacity Checks _____

	Actual	Maximum	LOS F?
v = v	5256	9000	No
F _i F			
v = v - v	2812	9000	No
F _O F R			
v	2444	3800	No
R			
v v	1040 pc/h	(Equation 25-15 or 25-16)	
3 or av ³⁴			
Is v v > 2700 pc/h?		No	
3 or av ³⁴			
Is v v > 1.5 v / 2		No	
3 or av ³⁴ 12			
If yes, v = 3175		(Equation 25-18)	
12A			

_____ Flow Entering Diverge Influence Area _____

	Actual	Max Desirable	Violation?
v	3175	4400	No
12			

_____ Level of Service Determination (if not F) _____

Density, $D = 4.252 + 0.0086 v - 0.009 L = 18.1 \text{ pc/mi/ln}$
R 12 D
Level of service for ramp-freeway junction areas of influence B

_____ Speed Estimation _____

Intermediate speed variable, $D = 0.648$
S
Space mean speed in ramp influence area, $S = 46.6 \text{ mph}$

Space mean speed in outer lanes,	R	S = 60.2	mph
Space mean speed for all vehicles,	0	S = 51.2	mph

Phone: Fax:
 E-mail:

Merge Analysis

Analyst: CTR
 Agency/Co.: HNTB
 Date performed: 8/2/2010
 Analysis time period: Build Service Road Concept
 Freeway/Dir of Travel: I-4 WB
 Junction: On Ramp from SR 417
 Jurisdiction: Seminole County
 Analysis Year: 2022
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	3	
Free-flow speed on freeway	55.0	mph
Volume on freeway	2750 ✓	vph

On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	2	
Free-flow speed on ramp	35.0	mph
Volume on ramp	590 ✓	vph
Length of first accel/decel lane	600 ✓	ft
Length of second accel/decel lane	1500	ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes ✓	
Volume on adjacent Ramp	2660 ✓	vph
Position of adjacent Ramp	Downstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	3654	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	2750 ✓	590	2660	vph
Peak-hour factor, PHF	0.92 ✓	0.92	0.92	
Peak 15-min volume, v15	74 ✓	160	723	v
Trucks and buses	9 ✓	9	9	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade		%	%	%
Length		mi	mi	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.957	0.957	0.957	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	3124	670	3021	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)
EQ
P = 0.555 Using Equation 0
FM
 $v_{12} = v_F (P_{FM}) = 1734 \text{ pc/h}$

Capacity Checks

v _{FO}	Actual	Maximum	LOS F?
	3794	6750	No
v _{3 or av34}	1390 pc/h	(Equation 25-4 or 25-5)	
Is v _{3 or av34} > 2700 pc/h?		No	
Is v _{3 or av34} > 1.5 v ₁₂ / 2		Yes	
If yes, v _{12A} = 1785		(Equation 25-8)	

Flow Entering Merge Influence Area

v _{12A}	Actual	Max Desirable	Violation?
	1785	4600	No

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 7.4 \text{ pc/mi/ln}$
Level of service for ramp-freeway junction areas of influence A ✓

Speed Estimation

Intermediate speed variable,	M = 0.177	
Space mean speed in ramp influence area,	S _R = 52.7	mph
Space mean speed in outer lanes,	S ₀ = 52.0	mph
Space mean speed for all vehicles,	S = 52.4	mph

Phone: _____ Fax: _____
 E-mail: _____

Merge Analysis

Analyst: CTR
 Agency/Co.: HNTB
 Date performed: 8/2/2010
 Analysis time period: Build Service Road Concept
 Freeway/Dir of Travel: I-4 WB
 Junction: On Ramp from SR 417
 Jurisdiction: Seminole County
 Analysis Year: 2022
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	3	
Free-flow speed on freeway	55.0	mph
Volume on freeway	2750 ✓	vph

On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	2	
Free-flow speed on ramp	35.0	mph
Volume on ramp	590 ✓	vph
Length of first accel/decel lane	600 ✓	ft
Length of second accel/decel lane	1500	ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent Ramp	2460 ✓	vph
Position of adjacent Ramp	Upstream ✓	
Type of adjacent Ramp	Off	
Distance to adjacent Ramp	3490	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	2750 J	590	2460	vph
Peak-hour factor, PHF	0.92 J	0.92	0.92	
Peak 15-min volume, v15	747 J	160	668	v
Trucks and buses	9	9	9	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	%
Length	mi	mi	mi	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.957	0.957	0.957	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	3124	670	2794	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)
EQ
P = 0.555 Using Equation 0
FM
 $v_{12} = v_F (P_{FM}) = 1734 \text{ pc/h}$

Capacity Checks

	Actual	Maximum	LOS F?
v _{FO}	3794	6750	No
v _{3 or av34}	1390 pc/h	(Equation 25-4 or 25-5)	
Is v _{3 or av34} > 2700 pc/h?		No	
Is v _{3 or av34} > 1.5 v ₁₂ / 2		Yes	
If yes, v _{12A} = 1785		(Equation 25-8)	

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v _{12A}	1785	4600	No

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 7.4 \text{ pc/mi/ln}$
Level of service for ramp-freeway junction areas of influence A ✓

Speed Estimation

Intermediate speed variable,	M = 0.177	
Space mean speed in ramp influence area,	S _R = 52.7	mph
Space mean speed in outer lanes,	S ₀ = 52.0	mph
Space mean speed for all vehicles,	S = 52.4	mph

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Phone: Fax:
E-mail:

Merge Analysis

Analyst: CTRR
 Agency/Co.: HNTB
 Date performed: 3/07/2010
 Analysis time period: Build Service Road Concept
 Freeway/Dir of Travel: I-4 WB
 Junction: On Ramp from SR 46 & CR 46A
 Jurisdiction: Seminole County
 Analysis Year: 2022
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	3	
Free-flow speed on freeway	55.0	mph
Volume on freeway	3120	vph

On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	2	
Free-flow speed on ramp	35.0	mph
Volume on ramp	2400	vph
Length of first accel/decel lane	900	ft
Length of second accel/decel lane	900	ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent Ramp	590	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	3654	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	3120	2400	590	vph
Peak-hour factor, PHF	0.90	0.90	0.90	

Peak 15-min volume, v15	867	667	164	v
Trucks and buses	0	0	0	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	1.000	1.000	1.000	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	3467	2667	656	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)

EQ

P = 0.555 Using Equation 0

FM

v = v (P) = 1924 pc/h

12 F FM

Capacity Checks

v	Actual	Maximum	LOS F?
FO	6134	6750	No
v v	1543 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		No	
3 or av34			
Is v v > 1.5 v /2		Yes	
3 or av34	12		
If yes, v = 1981		(Equation 25-8)	
12A			

Flow Entering Merge Influence Area

v	Actual	Max Desirable	Violation?
12A	1981	4600	Yes

Level of Service Determination (if not F)

Density, D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 23.6 pc/mi/ln

R R 12 A

Level of service for ramp-freeway junction areas of influence C

Speed Estimation

Intermediate speed variable, M = 0.539

S

Space mean speed in ramp influence area, S = 48.0 mph

R

Space mean speed in outer lanes, S = 51.5 mph

0

Space mean speed for all vehicles, S = 48.8 mph

Phone: Fax:
 E-mail:

Diverge Analysis

Analyst: CTR
 Agency/Co.: HNTB
 Date performed: 8/02/2010
 Analysis time period: Build Service Road Concept
 Freeway/Dir of Travel: CD Rd (East of I-4) EB
 Junction: Off Ramp to SR 417 EB
 Jurisdiction: Seminole County
 Analysis Year: 2022
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	2	
Free-flow speed on freeway	55.0	mph
Volume on freeway	1730 ✓	vph

Off Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	35.0 ✓	mph
Volume on ramp	430 ✓	vph
Length of first accel/decel lane	0	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes ✓	
Volume on adjacent ramp	160 ✓	vph
Position of adjacent ramp	Downstream	
Type of adjacent ramp	Off	
Distance to adjacent ramp	1584	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	1730	430	160	vph
Peak-hour factor, PHF	0.92 ✓	0.92	0.92	
Peak 15-min volume, v15	470 ✓	117	43	v
Trucks and buses	9 ✓	9	9	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00 %	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.957	0.957	0.957	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	1965	488	182	pcph

Estimation of V12 Diverge Areas

$$L = \frac{EQ}{P} \quad (\text{Equation 25-8 or 25-9})$$

$$P = 1.000 \quad \text{Using Equation 0}$$

$$v_{12R} = v_{FR} + (v_{FR} - v_{FD}) P = 1965 \quad \text{pc/h}$$

Capacity Checks

	Actual	Maximum	LOS F?
$v_{Fi} = v_F$	1965	4500	No
$v_{FO} = v_F - v_R$	1477	4500	No
v_R	488	2000	No
$v_{3 \text{ or } av34}$	0 pc/h	(Equation 25-15 or 25-16)	
Is $v_{3 \text{ or } av34} > 2700$ pc/h?		No	
Is $v_{3 \text{ or } av34} > 1.5 v_{12} / 2$		No	
If yes, $v_{12A} = 1965$		(Equation 25-18)	

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v_{12}	1965	4400	No

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v_{12} - 0.009 L_D = 21.2$ pc/mi/ln

Level of service for ramp-freeway junction areas of influence C \checkmark

Speed Estimation

Intermediate speed variable,	$D = 0.472$	
Space mean speed in ramp influence area,	$S = 48.9$	mph
Space mean speed in outer lanes,	$S = N/A$	mph
Space mean speed for all vehicles,	$S = 48.9$	mph

Phone: Fax:
 E-mail:

Diverge Analysis

Analyst: CTR
 Agency/Co.: HNTB
 Date performed: 8/02/2010
 Analysis time period: Build Service Road Concept
 Freeway/Dir of Travel: CD Rd (East of I-4) EB
 Junction: Off Ramp to SR 417 WB
 Jurisdiction: Seminole County
 Analysis Year: 2022
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	2 ✓	
Free-flow speed on freeway	55.0	mph
Volume on freeway	1300 ✓	vph

Off Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	35.0 ✓	mph
Volume on ramp	160 ✓	vph
Length of first accel/decel lane	0	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes ✓	
Volume on adjacent ramp	690 ✓	vph
Position of adjacent ramp	Downstream	
Type of adjacent ramp	On	
Distance to adjacent ramp	1478	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	1300 ✓	160	690	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	353 ✓	43	187	v
Trucks and buses	9 ✓	9	9	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00 %	0.00 %	0.00 %	
Length	0.00 mi	0.00 mi	0.00 mi	
Trucks and buses PCE, ET	1.5*	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.957	0.957	0.957	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	1477	182	784	pcph

Estimation of V12 Diverge Areas

$$L = \frac{EQ}{P} \quad (\text{Equation 25-8 or 25-9})$$

$$P = 1.000 \quad \text{Using Equation 0}$$

$$v_{12R} = v_{FR} + (v_{FR} - v_{FD}) P = 1477 \quad \text{pc/h}$$

Capacity Checks

	Actual	Maximum	LOS F?
$v_{Fi} = v_F$	1477	4500	No
$v_{FO} = v_F - v_R$	1295	4500	No
v_R	182	2000	No
$v_{3 \text{ or } av34}$	0 pc/h	(Equation 25-15 or 25-16)	
Is $v_{3 \text{ or } av34} > 2700$ pc/h?		No	
Is $v_{3 \text{ or } av34} > 1.5 v_{12} / 2$		No	
If yes, $v_{12A} = 1477$		(Equation 25-18)	

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v_{12}	1477	4400	No

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v_{12} - 0.009 L_D = 17.0$ pc/mi/ln

Level of service for ramp-freeway junction areas of influence B ✓

Speed Estimation

Intermediate speed variable,	$D = 0.444$	
Space mean speed in ramp influence area,	$S_R = 49.2$	mph
Space mean speed in outer lanes,	$S_0 = N/A$	mph
Space mean speed for all vehicles,	$S = 49.2$	mph

Phone: Fax:
E-mail:

Diverge Analysis

Analyst: CTR
 Agency/Co.: HNTB
 Date performed: 8/02/2010
 Analysis time period: Build Service Road Concept
 Freeway/Dir of Travel: CD Rd (East of I-4) EB
 Junction: Off Ramp to SR 417 WB
 Jurisdiction: Seminole County
 Analysis Year: 2022
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	2	
Free-flow speed on freeway	55.0	mph
Volume on freeway	1300 ✓	vph

Off Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	35.0	mph
Volume on ramp	160 ✓	vph
Length of first accel/decel lane	0	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes ✓	
Volume on adjacent ramp	430 ✓	vph
Position of adjacent ramp	Upstream	
Type of adjacent ramp	Off	
Distance to adjacent ramp	1584	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	1300 ✓	160	430	vph
Peak-hour factor, PHF	0.92 ✓	0.92	0.92	
Peak 15-min volume, v15	353 ✓	43	117	v
Trucks and buses	9 ✓	9	9	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00 %	0.00 %	0.00 %	
Length	0.00 mi	0.00 mi	0.00 mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.957	0.957	0.957	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	1477	182	488	pcph

Estimation of V12 Diverge Areas

$$L = \text{(Equation 25-8 or 25-9)}$$

$$EQ$$

$$P = 1.000 \text{ Using Equation } 0$$

$$FD$$

$$v_{12R} = v_F + (v_F - v_R) P = 1477 \text{ pc/h}$$

Capacity Checks

	Actual	Maximum	LOS F?
$v_{Fi} = v_F$	1477	4500	No
$v_{FO} = v_F - v_R$	1295	4500	No
v_R	182	2000	No
$v_{3 \text{ or } av34}$	0 pc/h	(Equation 25-15 or 25-16)	
Is $v_{3 \text{ or } av34} > 2700 \text{ pc/h?}$		No	
Is $v_{3 \text{ or } av34} > 1.5 v_{12} / 2$		No	
If yes, $v_{12A} = 1477$		(Equation 25-18)	

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v_{12}	1477	4400	No

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v_{12} - 0.009 L_D = 17.0 \text{ pc/mi/ln}$

Level of service for ramp-freeway junction areas of influence B ✓

Speed Estimation

Intermediate speed variable,	$D = 0.444$	
Space mean speed in ramp influence area,	$S_R = 49.2$	mph
Space mean speed in outer lanes,	$S_O = N/A$	mph
Space mean speed for all vehicles,	$S = 49.2$	mph

HCS+: Ramps and Ramp Junctions Release 5.4

Phone: Fax:
E-mail:

Merge Analysis

Analyst: CTRR
 Agency/Co.: HNTB
 Date performed: 3/07/2010
 Analysis time period: Build Service Road Concept
 Freeway/Dir of Travel: CD Rd (East of I-4) EB
 Junction: On Ramp from SR 417 EB
 Jurisdiction: Seminole County
 Analysis Year: 2022
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Merge		
Number of lanes in freeway	2		
Free-flow speed on freeway	55.0	mph	
Volume on freeway	1140	vph	

On Ramp Data

Side of freeway	Left		
Number of lanes in ramp	2		
Free-flow speed on ramp	35.0	mph	
Volume on ramp	690	vph	
Length of first accel/decel lane	500	ft	
Length of second accel/decel lane	500	ft	

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes		
Volume on adjacent Ramp	160	vph	
Position of adjacent Ramp	Upstream		
Type of adjacent Ramp	Off		
Distance to adjacent Ramp	1478	ft	

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
		Ramp		
Volume, V (vph)	1140	690	160	vph
Peak-hour factor, PHF	0.90	0.90	0.90	

Peak 15-min volume, v ₁₅	317	192	44	v
Trucks and buses	0	0	0	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, f _{HV}	1.000	1.000	1.000	
Driver population factor, f _P	1.00	1.00	1.00	
Flow rate, v _p	1267	767	178	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)

EQ

P = 1.000 Using Equation 0

FM

v = v (P) = 1267 pc/h

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	2034	4500	No
FO			
v v	0 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		No	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34	12		
If yes, v = 1267		(Equation 25-8)	
12A			

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	1267	4600	No
R12			

Level of Service Determination (if not F)

Density, D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 11.6 pc/mi/ln

R R 12 A

Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable, M = 0.251

S

Space mean speed in ramp influence area, S = 51.7 mph

R

Space mean speed in outer lanes, S = N/A mph

0

Space mean speed for all vehicles, S = 51.7 mph

HCS+: Ramps and Ramp Junctions Release 5.4

Phone: Fax:
E-mail:

Merge Analysis

Analyst: CTRR
 Agency/Co.: HNTB
 Date performed: 3/07/2010
 Analysis time period: Build Service Road Concept
 Freeway/Dir of Travel: CD Rd (East of I-4) EB
 Junction: On Ramp from SR 417 EB
 Jurisdiction: Seminole County
 Analysis Year: 2022
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Merge		
Number of lanes in freeway	2		
Free-flow speed on freeway	55.0	mph	
Volume on freeway	1140	vph	

On Ramp Data

Side of freeway	Left		
Number of lanes in ramp	2		
Free-flow speed on ramp	35.0	mph	
Volume on ramp	690	vph	
Length of first accel/decel lane	500	ft	
Length of second accel/decel lane	500	ft	

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes		
Volume on adjacent Ramp	690	vph	
Position of adjacent Ramp	Downstream		
Type of adjacent Ramp	Off		
Distance to adjacent Ramp	3010	ft	

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
		Ramp		
Volume, V (vph)	1140	690	690	vph
Peak-hour factor, PHF	0.90	0.90	0.90	

Peak 15-min volume, v ₁₅	317	192	192	v
Trucks and buses	0	0	0	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, f _{HV}	1.000	1.000	1.000	
Driver population factor, f _P	1.00	1.00	1.00	
Flow rate, v _p	1267	767	767	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)

EQ

P = 1.000 Using Equation 0

FM

v = v (P) = 1267 pc/h

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	2034	4500	No
FO			
v v	0 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		No	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34 12			
If yes, v = 1267		(Equation 25-8)	
12A			

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	1267	4600	No
R12			

Level of Service Determination (if not F)

Density, D = 5.475 + 0.00734 v_R + 0.0078 v_A - 0.00627 L = 11.6 pc/mi/ln

Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable, M = 0.251

S

Space mean speed in ramp influence area, S = 51.7 mph

R

Space mean speed in outer lanes, S = N/A mph

0

Space mean speed for all vehicles, S = 51.7 mph

HCS+: Ramps and Ramp Junctions Release 5.4

Phone: Fax:
E-mail:

Diverge Analysis

Analyst: CTRR
 Agency/Co.: HNTB
 Date performed: 3/07/2010
 Analysis time period: Build Service Road Concept
 Freeway/Dir of Travel: CD Rd (East of I-4) EB
 Junction: Off Ramp to I-4 EB
 Jurisdiction: Seminole County
 Analysis Year: 2022
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	3	
Free-flow speed on freeway	55.0	mph
Volume on freeway	1830	vph

Off Ramp Data

Side of freeway	Left	
Number of lanes in ramp	2	
Free-Flow speed on ramp	35.0	mph
Volume on ramp	690	vph
Length of first accel/decel lane	500	ft
Length of second accel/decel lane	500	ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent ramp	690	vph
Position of adjacent ramp	Upstream	
Type of adjacent ramp	On	
Distance to adjacent ramp	3010	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	1830	690	690	vph
Peak-hour factor, PHF	0.90	0.90	0.90	

Peak 15-min volume, v15	508	192	192	v
Trucks and buses	0	0	0	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00 %	0.00 %	0.00 %	
Length	0.00 mi	0.00 mi	0.00 mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	1.000	1.000	1.000	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	2033	767	767	pcph

Estimation of V12 Diverge Areas

$L =$ (Equation 25-8 or 25-9)
 EQ
 $P = 0.450$ Using Equation 0
 FD
 $v = v + (v - v) P = 1337$ pc/h
 $12 R F R FD$

Capacity Checks

	Actual	Maximum	LOS F?
$v = v$	2033	6750	No
$Fi F$			
$v = v - v$	1266	6750	No
$FO F R$			
v	767	3800	No
R			
$v v$	696 pc/h	(Equation 25-15 or 25-16)	
3 or $av34$			
Is $v v > 2700$ pc/h?		No	
3 or $av34$			
Is $v v > 1.5 v / 2$		No	
3 or $av34$	12		
If yes, $v = 1337$		(Equation 25-18)	
$12A$			

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v	1337	4400	No
12			

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v - 0.009 L = 2.8$ pc/mi/ln
 R 12 D
 Level of service for ramp-freeway junction areas of influence A

Speed Estimation

Intermediate speed variable, $D = 0.497$
 S
 Space mean speed in ramp influence area, $S = 48.5$ mph

Space mean speed in outer lanes,	$\frac{R}{0}$	S = 60.3 mph
Space mean speed for all vehicles,		S = 51.7 mph

Phone: Fax:
E-mail:

Merge Analysis

Analyst: CTR
 Agency/Co.: HNTB
 Date performed: 8/02/2010
 Analysis time period: Build Service Road Concept
 Freeway/Dir of Travel: CD Rd (West of I-4) WB
 Junction: On Ramp from SR 46 EB
 Jurisdiction: Seminole County
 Analysis Year: 2022
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	2	
Free-flow speed on freeway	55.0 ✓	mph
Volume on freeway	790 ✓	vph

On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	35.0 ✓	mph
Volume on ramp	500 ✓	vph
Length of first accel/decel lane	700	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes ✓	
Volume on adjacent Ramp	2460 ✓	vph
Position of adjacent Ramp	Downstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1426	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	790 ✓	500	2460	vph
Peak-hour factor, PHF	0.92 ✓	0.92	0.92	
Peak 15-min volume, v15	215 ✓	136	668	v
Trucks and buses	9 ✓	9	9	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade		%	%	%
Length		mi	mi	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.957	0.957	0.957	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	897	568	2794	pcph

Estimation of V12 Merge Areas

$$L = \text{(Equation 25-2 or 25-3)}$$

$$EQ$$

$$P = 1.000 \text{ Using Equation } 0$$

$$FM$$

$$v_{12} = v_F (P_{FM}) = 897 \text{ pc/h}$$

Capacity Checks

		Actual	Maximum	LOS F?
v _{FO}		1465	4500	No
v _{3 or av34}	v _{av34}	0 pc/h	(Equation 25-4 or 25-5)	
Is v _{3 or av34}	v _{av34}	> 2700 pc/h?	No	
Is v _{3 or av34}	v _{av34}	> 1.5 v ₁₂ / 2	No	
If yes, v _{12A}	= 897		(Equation 25-8)	

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v _{R12}	897	4600	No

Level of Service Determination (if not F)

$$\text{Density, } D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 12.3 \text{ pc/mi/ln}$$

Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable,	M = 0.289	
Space mean speed in ramp influence area,	S = 51.2	mph
Space mean speed in outer lanes,	S = N/A	mph
Space mean speed for all vehicles,	S = 51.2	mph

HCS+: Ramps and Ramp Junctions Release 5.4

Phone: Fax:
E-mail:

Merge Analysis

Analyst: CTRR
 Agency/Co.: HNTB
 Date performed: 3/07/2010
 Analysis time period: Build Service Road Concept
 Freeway/Dir of Travel: CD Rd (West of I-4) WB
 Junction: On Ramp from I-4 WB
 Jurisdiction: Seminole County
 Analysis Year: 2022
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	4	
Free-flow speed on freeway	55.0	mph
Volume on freeway	1290	vph

On Ramp Data

Side of freeway	Left	
Number of lanes in ramp	2	
Free-flow speed on ramp	35.0	mph
Volume on ramp	2200	vph
Length of first accel/decel lane	530	ft
Length of second accel/decel lane	530	ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent Ramp	500	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	1426	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
		Ramp		
Volume, V (vph)	1290	2200	500	vph
Peak-hour factor, PHF	0.90	0.90	0.90	

Peak 15-min volume, v ₁₅	358	611	139	v
Trucks and buses	0	0	0	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, f _{HV}	1.000	1.000	1.000	
Driver population factor, f _P	1.00	1.00	1.00	
Flow rate, v _p	1433	2444	556	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)

EQ

P = 0.209 Using Equation 0

FM

v = v (P) = 299 pc/h

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	3877	9000	No
FO			
v v	567 pc/h	(Equation 25-4 or 25-5)	
3 or av ₃₄			
Is v v > 2700 pc/h?		No	
3 or av ₃₄			
Is v v > 1.5 v /2		Yes	
3 or av ₃₄ 12			
If yes, v = 573		(Equation 25-8)	
12A			

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	573	4600	No
12A			

Level of Service Determination (if not F)

Density, D = 5.475 + 0.00734 v_R + 0.0078 v_A - 0.00627 L = 18.8 pc/mi/ln

Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable, M = 0.295

S

Space mean speed in ramp influence area, S = 51.2 mph

R

Space mean speed in outer lanes, S = 55.0 mph

0

Space mean speed for all vehicles, S = 51.9 mph

HCS+: Ramps and Ramp Junctions Release 5.4

Phone: Fax:
E-mail:

Merge Analysis

Analyst: CTRR
Agency/Co.: HNTB
Date performed: 3/07/2010
Analysis time period: Build Service Road Concept
Freeway/Dir of Travel: CD Rd (West of I-4) WB
Junction: On Ramp from I-4 WB
Jurisdiction: Seminole County
Analysis Year: 2022
Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	4	
Free-flow speed on freeway	55.0	mph
Volume on freeway	1290	vph

On Ramp Data

Side of freeway	Left	
Number of lanes in ramp	2	
Free-flow speed on ramp	35.0	mph
Volume on ramp	2200	vph
Length of first accel/decel lane	530	ft
Length of second accel/decel lane	530	ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent Ramp	690	vph
Position of adjacent Ramp	Downstream	
Type of adjacent Ramp	Off	
Distance to adjacent Ramp	2851	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
		Ramp		
Volume, V (vph)	1290	2200	690	vph
Peak-hour factor, PHF	0.90	0.90	0.90	

Peak 15-min volume, v_{15}	358	611	192	v
Trucks and buses	0	0	0	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	1.000	1.000	1.000	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	1433	2444	767	pcph

Estimation of V12 Merge Areas

$L =$ (Equation 25-2 or 25-3)

EQ

$P = 0.209$ Using Equation 0

FM

$v = v(P) = 299$ pc/h

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	3877	9000	No
FO			
v v	567 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		No	
3 or av34			
Is v v > 1.5 v /2		Yes	
3 or av34 12			
If yes, v = 573		(Equation 25-8)	
12A			

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	573	4600	No
12A			

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 18.8$ pc/mi/ln

R R 12 A

Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable, $M = 0.295$

S

Space mean speed in ramp influence area, $S = 51.2$ mph

R

Space mean speed in outer lanes, $S = 55.0$ mph

0

Space mean speed for all vehicles, $S = 51.9$ mph

HCS+: Ramps and Ramp Junctions Release 5.4

Phone: Fax:
E-mail:

Diverge Analysis

Analyst: CTRR
 Agency/Co.: HNTB
 Date performed: 3/07/2010
 Analysis time period: Build Service Road Concept
 Freeway/Dir of Travel: CD Rd (West of I-4) WB
 Junction: Off Ramp to SR 417 WB
 Jurisdiction: Seminole County
 Analysis Year: 2022
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Diverge		
Number of lanes in freeway	3		
Free-flow speed on freeway	55.0	mph	
Volume on freeway	3490	vph	

Off Ramp Data

Side of freeway	Right		
Number of lanes in ramp	2		
Free-Flow speed on ramp	35.0	mph	
Volume on ramp	690	vph	
Length of first accel/decel lane	730	ft	
Length of second accel/decel lane	730	ft	

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes		
Volume on adjacent ramp	2200	vph	
Position of adjacent ramp	Upstream		
Type of adjacent ramp	On		
Distance to adjacent ramp	2851	ft	

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
		Ramp		
Volume, V (vph)	3490	690	2200	vph
Peak-hour factor, PHF	0.90	0.90	0.90	

Peak 15-min volume, v ₁₅	969	192	611	v
Trucks and buses	0	0	0	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00 %	0.00 %	0.00 %	
Length	0.00 mi	0.00 mi	0.00 mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, f _{HV}	1.000	1.000	1.000	
Driver population factor, f _P	1.00	1.00	1.00	
Flow rate, v _p	3878	767	2444	pcph

Estimation of V₁₂ Diverge Areas

L = (Equation 25-8 or 25-9)

EQ

P = 0.450 Using Equation 0

FD

$v = v + (v - v) P = 2167$ pc/h

$12 \quad R \quad F \quad R \quad FD$

Capacity Checks

	Actual	Maximum	LOS F?
v = v	3878	6750	No
F _i F			
v = v - v	3111	6750	No
F _O F R			
v	767	3800	No
R			
v v	1711 pc/h	(Equation 25-15 or 25-16)	
3 or av ₃₄			
Is v v > 2700 pc/h?		No	
3 or av ₃₄			
Is v v > 1.5 v / 2		Yes	
3 or av ₃₄ 12			
If yes, v = 2216		(Equation 25-18)	
12A			

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v	2216	4400	No
12A			

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v - 0.009 L = 3.6$ pc/mi/ln

R 12 D

Level of service for ramp-freeway junction areas of influence A

Speed Estimation

Intermediate speed variable, $D = 0.497$

S

Space mean speed in ramp influence area, $S = 48.5$ mph

Space mean speed in outer lanes,	R	S = 57.8	mph
Space mean speed for all vehicles,	0	S = 52.1	mph

Phone: Fax:
E-mail:

Diverge Analysis

Analyst: CTR
 Agency/Co.: HNTB
 Date performed: 8/2/2010
 Analysis time period: Build Service Road Concept
 Freeway/Dir of Travel: CD Rd (West of I-4) WB
 Junction: Off Ramp to SR 417 WB
 Jurisdiction: Seminole County
 Analysis Year: 2022
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	3	
Free-flow speed on freeway	55.0	mph
Volume on freeway	3750	vph

Off Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	35.0	mph
Volume on ramp	690	vph
Length of first accel/decel lane	500	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent ramp	1230	vph
Position of adjacent ramp	Downstream	
Type of adjacent ramp	off	
Distance to adjacent ramp	1531	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	3750	690	1230	vph
Peak-hour factor, PHF	0.92	0.92	0.92	
Peak 15-min volume, v15	1019	187	334	v
Trucks and buses	9	9	9	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00	%	0.00	%
Length	0.00	mi	0.00	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.957	0.957	0.957	
Driver population factor, fP	1.00	1.00	1.00	

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Phone: Fax:
E-mail:

Diverge Analysis

Analyst: CTRR
 Agency/Co.: HNTB
 Date performed: 3/07/2010
 Analysis time period: Build Service Road Concept
 Freeway/Dir of Travel: CD Rd (West of I-4) WB
 Junction: Off Ramp to SR 417 EB
 Jurisdiction: Seminole County
 Analysis Year: 2022
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	2	
Free-flow speed on freeway	55.0	mph
Volume on freeway	2800	vph

Off Ramp Data

Side of freeway	Right	
Number of lanes in ramp	2	
Free-Flow speed on ramp	35.0	mph
Volume on ramp	1230	vph
Length of first accel/decel lane	500	ft
Length of second accel/decel lane	500	ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent ramp	690	vph
Position of adjacent ramp	Upstream	
Type of adjacent ramp	Off	
Distance to adjacent ramp	1531	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
		Ramp	
Volume, V (vph)	2800	1230	690 vph
Peak-hour factor, PHF	0.90	0.90	0.90

Peak 15-min volume, v_{15}	778	342	192	v
Trucks and buses	0	0	0	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00 %	0.00 %	0.00 %	
Length	0.00 mi	0.00 mi	0.00 mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	1.000	1.000	1.000	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, v_p	3111	1367	767	pcph

Estimation of V12 Diverge Areas

$L =$ (Equation 25-8 or 25-9)
 EQ
 $P = 1.000$ Using Equation 0
 FD
 $v = v + (v - v) P = 3111$ pc/h
 12 R F R FD

Capacity Checks

	Actual	Maximum	LOS F?
$v = v$	3111	4500	No
Fi F			
$v = v - v$	1744	4500	No
FO F R			
v	1367	3800	No
R			
$v v$	0 pc/h	(Equation 25-15 or 25-16)	
3 or av34			
Is $v v > 2700$ pc/h?		No	
3 or av34			
Is $v v > 1.5 v / 2$		No	
3 or av34 12			
If yes, $v = 3111$		(Equation 25-18)	
12A			

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v	3111	4400	No
12			

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v - 0.009 L = 17.5$ pc/mi/ln
 R 12 D
 Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable, $D = 0.551$
 S
 Space mean speed in ramp influence area, $S = 47.8$ mph

Space mean speed in outer lanes,	R	S = N/A	mph
Space mean speed for all vehicles,	0	S = 47.8	mph

HCS+: Ramps and Ramp Junctions Release 5.4

Phone: Fax:
E-mail:

Diverge Analysis

Analyst: CTRR
 Agency/Co.: HNTB
 Date performed: 3/07/2010
 Analysis time period: Build Service Road Concept
 Freeway/Dir of Travel: CD Rd (West of I-4) WB
 Junction: Off Ramp to SR 417 EB
 Jurisdiction: Seminole County
 Analysis Year: 2022
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Diverge		
Number of lanes in freeway	2		
Free-flow speed on freeway	55.0	mph	
Volume on freeway	2800	vph	

Off Ramp Data

Side of freeway	Right		
Number of lanes in ramp	2		
Free-Flow speed on ramp	35.0	mph	
Volume on ramp	1230	vph	
Length of first accel/decel lane	500	ft	
Length of second accel/decel lane	500	ft	

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes		
Volume on adjacent ramp	690	vph	
Position of adjacent ramp	Downstream		
Type of adjacent ramp	Off		
Distance to adjacent ramp	4594	ft	

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
		Ramp		
Volume, V (vph)	2800	1230	690	vph
Peak-hour factor, PHF	0.90	0.90	0.90	

Peak 15-min volume, v ₁₅	778	342	192	v
Trucks and buses	0	0	0	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00 %	0.00 %	0.00 %	
Length	0.00 mi	0.00 mi	0.00 mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, f _{HV}	1.000	1.000	1.000	
Driver population factor, f _P	1.00	1.00	1.00	
Flow rate, v _p	3111	1367	767	pcph

Estimation of V12 Diverge Areas

L = (Equation 25-8 or 25-9)
EQ
P = 1.000 Using Equation 0
FD
 $v = v + (v - v) P = 3111 \text{ pc/h}$
12 R F R FD

Capacity Checks

	Actual	Maximum	LOS F?
v = v	3111	4500	No
Fi F			
v = v - v	1744	4500	No
FO F R			
v	1367	3800	No
R			
v v	0 pc/h	(Equation 25-15 or 25-16)	
3 or av34			
Is v v > 2700 pc/h?		No	
3 or av34			
Is v v > 1.5 v /2		No	
3 or av34 12			
If yes, v = 3111		(Equation 25-18)	
12A			

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v	3111	4400	No
12			

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v - 0.009 L = 17.5 \text{ pc/mi/ln}$
R 12 D
Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable, $D = 0.551$
S
Space mean speed in ramp influence area, $S = 47.8 \text{ mph}$

Space mean speed in outer lanes,	R	S = N/A	mph
Space mean speed for all vehicles,	0	S = 47.8	mph

Phone:
E-mail:

Fax:

Diverge Analysis

Analyst: CTR
 Agency/Co.: HNTB
 Date performed: 8/2/2010
 Analysis time period: Build Service Road Concept
 Freeway/Dir of Travel: CD Rd (West of I-4) WB
 Junction: Off Ramp to CR 46A
 Jurisdiction: Seminole County
 Analysis Year: 2022
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	2	
Free-flow speed on freeway	55.0 ✓	mph
Volume on freeway	1830 ✓	vph

Off Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	35.0	mph
Volume on ramp	690 ✓	vph
Length of first accel/decel lane	0	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes ✓	
Volume on adjacent ramp	1520 ✓	vph
Position of adjacent ramp	Downstream	
Type of adjacent ramp	On	
Distance to adjacent ramp	1320	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	1830 ✓	690	1520	vph
Peak-hour factor, PHF	0.92 ✓	0.92	0.92	
Peak 15-min volume, v15	497 ✓	187	413	v
Trucks and buses	9 ✓	9	9	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00 %	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.957	0.957	0.957	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	2079	784	1727	pcph

Estimation of V12 Diverge Areas

$$L = \text{(Equation 25-8 or 25-9)}$$

$$EQ$$

$$P = 1.000 \text{ Using Equation } 0$$

$$FD$$

$$v_{12R} = v_F + (v_R - v_{FD}) P = 2079 \text{ pc/h}$$

Capacity Checks

	Actual	Maximum	LOS F?
$v_{Fi} = v_F$	2079	4500	No
$v_{FO} = v_F - v_R$	1295	4500	No
v_R	784	2000	No
$v_3 \text{ or } v_{av34}$	0 pc/h	(Equation 25-15 or 25-16)	
Is $v_3 \text{ or } v_{av34} > 2700 \text{ pc/h?}$		No	
Is $v_3 \text{ or } v_{av34} > 1.5 v_{12} / 2$		No	
If yes, $v_{12A} = 2079$		(Equation 25-18)	

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v_{12}	2079	4400	No

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v_{12} - 0.009 L_D = 22.1 \text{ pc/mi/ln}$

Level of service for ramp-freeway junction areas of influence C ✓

Speed Estimation

Intermediate speed variable,	$D = 0.499$	
Space mean speed in ramp influence area,	$S = 48.5$	mph
Space mean speed in outer lanes,	$S = N/A$	mph
Space mean speed for all vehicles,	$S = 48.5$	mph

Phone: _____ Fax: _____
 E-mail: _____

Diverge Analysis

Analyst: CTR
 Agency/Co.: HNTB
 Date performed: 8/2/2010
 Analysis time period: Build Service Road Concept
 Freeway/Dir of Travel: CD Rd (West of I-4) WB
 Junction: Off Ramp to CR 46A
 Jurisdiction: Seminole County
 Analysis Year: 2022
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	2	
Free-flow speed on freeway	55.0	mph
Volume on freeway	1830 ✓	vph

Off Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	35.0	mph
Volume on ramp	690 ✓	vph
Length of first accel/decel lane	0	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes ✓	
Volume on adjacent ramp	1230 ✓	vph
Position of adjacent ramp	Upstream	
Type of adjacent ramp	Off	
Distance to adjacent ramp	4594	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	1830 ✓	690	1230	vph
Peak-hour factor, PHF	0.92 ✓	0.92	0.92	
Peak 15-min volume, v15	49 ✓	187	334	v
Trucks and buses	9 ✓	9	9	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00 %	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.957	0.957	0.957	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	2079	784	1397	pcph

Estimation of V12 Diverge Areas

$$L = \text{EQ} \quad (\text{Equation 25-8 or 25-9})$$

$$P = 1.000 \quad \text{Using Equation 0}$$

$$v_{12R} = v_{FR} + (v_{FR} - v_{FD}) P = 2079 \quad \text{pc/h}$$

Capacity Checks

	Actual	Maximum	LOS F?
$v_{Fi} = v_F$	2079	4500	No
$v_{FO} = v_F - v_R$	1295	4500	No
v_R	784	2000	No
$v_{3 \text{ or } av34}$	0 pc/h	(Equation 25-15 or 25-16)	
Is $v_{3 \text{ or } av34} > 2700$ pc/h?		No	
Is $v_{3 \text{ or } av34} > 1.5 v_{12} / 2$		No	
If yes, $v_{12A} = 2079$		(Equation 25-18)	

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v_{12}	2079	4400	No

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v_{12} - 0.009 L_D = 22.1$ pc/mi/ln
 Level of service for ramp-freeway junction areas of influence C ✓

Speed Estimation

Intermediate speed variable,	$D = 0.499$	
Space mean speed in ramp influence area,	$S = 48.5$	mph
Space mean speed in outer lanes,	$S = \text{N/A}$	mph
Space mean speed for all vehicles,	$S = 48.5$	mph

HCS+: Ramps and Ramp Junctions Release 5.4

Phone: Fax:
E-mail:

Merge Analysis

Analyst: CTRR
Agency/Co.: HNTB
Date performed: 3/07/2010
Analysis time period: Build Service Road Concept
Freeway/Dir of Travel: CD Rd (West of I-4) WB
Junction: On Ramp from SR 46 EB
Jurisdiction: Seminole County
Analysis Year: 2022
Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Merge		
Number of lanes in freeway	2		
Free-flow speed on freeway	55.0	mph	
Volume on freeway	880	vph	

On Ramp Data

Side of freeway	Right		
Number of lanes in ramp	1		
Free-flow speed on ramp	35.0	mph	
Volume on ramp	1520	vph	
Length of first accel/decel lane	500	ft	
Length of second accel/decel lane		ft	

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes		
Volume on adjacent Ramp	690	vph	
Position of adjacent Ramp	Upstream		
Type of adjacent Ramp	Off		
Distance to adjacent Ramp	1320	ft	

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
		Ramp		
Volume, V (vph)	880	1520	690	vph
Peak-hour factor, PHF	0.90	0.90	0.90	

Peak 15-min volume, v ₁₅	244	422	192	v
Trucks and buses	0	0	0	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, f _{HV}	1.000	1.000	1.000	
Driver population factor, f _P	1.00	1.00	1.00	
Flow rate, v _p	978	1689	767	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)

EQ

P = 1.000 Using Equation 0

FM

v = v_p (P) = 978 pc/h

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	2667	4500	No
FO			
v	0 pc/h	(Equation 25-4 or 25-5)	
3 or av ₃₄			
Is v > 2700 pc/h?		No	
3 or av ₃₄			
Is v > 1.5 v / 2		No	
3 or av ₃₄ 12			
If yes, v = 978		(Equation 25-8)	
12A			

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	978	4600	No
R12			

Level of Service Determination (if not F)

Density, D = 5.475 + 0.00734 v_R + 0.0078 v_A - 0.00627 L = 22.4 pc/mi/ln

Level of service for ramp-freeway junction areas of influence C

Speed Estimation

Intermediate speed variable, M = 0.342

S

Space mean speed in ramp influence area, S = 50.6 mph

R

Space mean speed in outer lanes, S = N/A mph

0

Space mean speed for all vehicles, S = 50.6 mph

Phone: Fax:
 E-mail:

Merge Analysis

Analyst: CTR
 Agency/Co.: HNTB
 Date performed: 8/2/2010
 Analysis time period: Build Service Road Concept
 Freeway/Dir of Travel: CD Rd (West of I-4) WB
 Junction: On Ramp from CR 46A to CD Rd
 Jurisdiction: Seminole County
 Analysis Year: 2022
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	2	
Free-flow speed on freeway	55.0	mph
Volume on freeway	1140 ✓	vph

On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	35.0 ✓	mph
Volume on ramp	1520 ✓	vph
Length of first accel/decel lane	600	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes ✓	
Volume on adjacent Ramp	690 ✓	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	Off	
Distance to adjacent Ramp	1320	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	1140 ✓	1520	690	vph
Peak-hour factor, PHF	0.92 ✓	0.92	0.92	
Peak 15-min volume, v15	310 ✓	413	187	v
Trucks and buses	9 ✓	9	9	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade		%	%	%
Length		mi	mi	mi
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	

Heavy vehicle adjustment, fHV	0.957	0.957	0.957	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	1295	1727	784	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)
EQ
P = 1.000 Using Equation 0
FM
 $v_{12} = v_F (P_{FM}) = 1295 \text{ pc/h}$

Capacity Checks

		Actual	Maximum	LOS F?
v		3022	4500	No
FO				
v	v	0 pc/h	(Equation 25-4 or 25-5)	
3 or av34				
Is v	v	> 2700 pc/h?	No	
3 or av34				
Is v	v	> 1.5 v / 2	No	
3 or av34		12		
If yes, v		= 1295	(Equation 25-8)	
12A				

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	1295	4600	No
R12			

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 24.5 \text{ pc/mi/ln}$
Level of service for ramp-freeway junction areas of influence C ✓

Speed Estimation

Intermediate speed variable,	M = 0.359	
	S	
Space mean speed in ramp influence area,	S = 50.3	mph
	R	
Space mean speed in outer lanes,	S = N/A	mph
	0	
Space mean speed for all vehicles,	S = 50.3	mph

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Phone: Fax:
E-mail:

Merge Analysis

Analyst: CTRR
 Agency/Co.: HNTB
 Date performed: 3/07/2010
 Analysis time period: Build Service Road Concept
 Freeway/Dir of Travel: Wekiva Pkwy. WB
 Junction: On Ramp from SR 46
 Jurisdiction: Seminole County
 Analysis Year: 2022
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Merge	
Number of lanes in freeway	3	
Free-flow speed on freeway	55.0	mph
Volume on freeway	2870	vph

On Ramp Data

Side of freeway	Right	
Number of lanes in ramp	2	
Free-flow speed on ramp	35.0	mph
Volume on ramp	570	vph
Length of first accel/decel lane	800	ft
Length of second accel/decel lane	640	ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent Ramp	490	vph
Position of adjacent Ramp	Upstream	
Type of adjacent Ramp	On	
Distance to adjacent Ramp	6336	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
		Ramp		
Volume, V (vph)	2870	570	490	vph
Peak-hour factor, PHF	0.90	0.90	0.90	

Peak 15-min volume, v ₁₅	797	158	136	v
Trucks and buses	0	0	0	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, f _{HV}	1.000	1.000	1.000	
Driver population factor, f _P	1.00	1.00	1.00	
Flow rate, v _p	3189	633	544	pcph

Estimation of V12 Merge Areas

$L =$ (Equation 25-2 or 25-3)
 EQ
 $P = 0.555$ Using Equation 0
 FM
 $v = v(P) = 1770$ pc/h
 $12 F FM$

Capacity Checks

v	Actual	Maximum	LOS F?
	3822	6750	No
FO			
v v	1419 pc/h	(Equation 25-4 or 25-5)	
3 or av34			
Is v v > 2700 pc/h?		No	
3 or av34			
Is v v > 1.5 v /2		Yes	
3 or av34 12			
If yes, v = 1822		(Equation 25-8)	
12A			

Flow Entering Merge Influence Area

v	Actual	Max Desirable	Violation?
	1822	4600	No
12A			

Level of Service Determination (if not F)

Density, $D = 5.475 + 0.00734 v_R + 0.0078 v_A - 0.00627 L = 10.3$ pc/mi/ln
 Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable, $M = 0.210$
 S
 Space mean speed in ramp influence area, $S = 52.3$ mph
 R
 Space mean speed in outer lanes, $S = 51.9$ mph
 0
 Space mean speed for all vehicles, $S = 52.1$ mph

HCS+: Ramps and Ramp Junctions Release 5.4

Phone: Fax:
E-mail:

Merge Analysis

Analyst: CTRR
Agency/Co.: HNTB
Date performed: 3/07/2010
Analysis time period: Build Service Road Concept
Freeway/Dir of Travel: Wekiva Pkwy. WB
Junction: On Ramp from SR 46
Jurisdiction: Seminole County
Analysis Year: 2022
Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Merge		
Number of lanes in freeway	3		
Free-flow speed on freeway	55.0	mph	
Volume on freeway	2870	vph	

On Ramp Data

Side of freeway	Right		
Number of lanes in ramp	2		
Free-flow speed on ramp	35.0	mph	
Volume on ramp	570	vph	
Length of first accel/decel lane	800	ft	
Length of second accel/decel lane	640	ft	

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes		
Volume on adjacent Ramp	260	vph	
Position of adjacent Ramp	Downstream		
Type of adjacent Ramp	Off		
Distance to adjacent Ramp	6684	ft	

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
		Ramp		
Volume, V (vph)	2870	570	260	vph
Peak-hour factor, PHF	0.90	0.90	0.90	

Peak 15-min volume, v ₁₅	797	158	72	v
Trucks and buses	0	0	0	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, f _{HV}	1.000	1.000	1.000	
Driver population factor, f _P	1.00	1.00	1.00	
Flow rate, v _p	3189	633	289	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)

EQ

P = 0.555 Using Equation 0

FM

v = v(P) = 1770 pc/h

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	3822	6750	No
FO			
v v	1419 pc/h	(Equation 25-4 or 25-5)	
3 or av ₃₄			
Is v v > 2700 pc/h?		No	
3 or av ₃₄			
Is v v > 1.5 v /2		Yes	
3 or av ₃₄ 12			
If yes, v = 1822		(Equation 25-8)	
12A			

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	1822	4600	No
12A			

Level of Service Determination (if not F)

Density, D = 5.475 + 0.00734 v_R + 0.0078 v_A - 0.00627 L = 10.3 pc/mi/ln

Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable, M = 0.210

S

Space mean speed in ramp influence area, S = 52.3 mph

R

Space mean speed in outer lanes, S = 51.9 mph

0

Space mean speed for all vehicles, S = 52.1 mph

HCS+: Ramps and Ramp Junctions Release 5.4

Phone: Fax:
E-mail:

Diverge Analysis

Analyst: CTRR
 Agency/Co.: HNTB
 Date performed: 3/07/2010
 Analysis time period: Build Service Road Concept
 Freeway/Dir of Travel: Wekiva Pkwy. WB
 Junction: Off Ramp to WB CD
 Jurisdiction: Seminole County
 Analysis Year: 2022
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Diverge		
Number of lanes in freeway	3		
Free-flow speed on freeway	55.0	mph	
Volume on freeway	3440	vph	

Off Ramp Data

Side of freeway	Right		
Number of lanes in ramp	1		
Free-Flow speed on ramp	35.0	mph	
Volume on ramp	260	vph	
Length of first accel/decel lane	1340	ft	
Length of second accel/decel lane		ft	

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes		
Volume on adjacent ramp	570	vph	
Position of adjacent ramp	Upstream		
Type of adjacent ramp	On		
Distance to adjacent ramp	6684	ft	

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
		Ramp		
Volume, V (vph)	3440	260	570	vph
Peak-hour factor, PHF	0.90	0.90	0.90	

Peak 15-min volume, v ₁₅	956	72	158	v
Trucks and buses	0	0	0	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00 %	0.00 %	0.00 %	
Length	0.00 mi	0.00 mi	0.00 mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, f _{HV}	1.000	1.000	1.000	
Driver population factor, f _P	1.00	1.00	1.00	
Flow rate, v _p	3822	289	633	pcph

Estimation of V₁₂ Diverge Areas

L = 4622.39 (Equation 25-8 or 25-9)

EQ

P = 0.651 Using Equation 5

FD

$v = v + (v - v) P = 2590$ pc/h

12 R F R FD

Capacity Checks

	Actual	Maximum	LOS F?
v = v	3822	6750	No
F _i F			
v = v - v	3533	6750	No
F _O F R			
v	289	2000	No
R			
v v	1232 pc/h	(Equation 25-15 or 25-16)	
3 or av ₃₄			
Is v v > 2700 pc/h?		No	
3 or av ₃₄			
Is v v > 1.5 v / 2		No	
3 or av ₃₄ 12			
If yes, v = 2590		(Equation 25-18)	
12A			

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v	2590	4400	No
12			

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v - 0.009 L = 14.5$ pc/mi/ln
R 12 D
Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable, $D = 0.454$
S
Space mean speed in ramp influence area, $S = 49.1$ mph

Space mean speed in outer lanes,	R	S = 59.4	mph
Space mean speed for all vehicles,	0	S = 52.0	mph

HCS+: Ramps and Ramp Junctions Release 5.4

Phone: Fax:
E-mail:

Diverge Analysis

Analyst: CTRR
Agency/Co.: HNTB
Date performed: 3/07/2010
Analysis time period: Build Service Road Concept
Freeway/Dir of Travel: Wekiva Pkwy. WB
Junction: Off Ramp to WB CD
Jurisdiction: Seminole County
Analysis Year: 2022
Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Diverge		
Number of lanes in freeway	3		
Free-flow speed on freeway	55.0	mph	
Volume on freeway	3440	vph	

Off Ramp Data

Side of freeway	Right		
Number of lanes in ramp	1		
Free-Flow speed on ramp	35.0	mph	
Volume on ramp	260	vph	
Length of first accel/decel lane	1340	ft	
Length of second accel/decel lane		ft	

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes		
Volume on adjacent ramp	40	vph	
Position of adjacent ramp	Downstream		
Type of adjacent ramp	On		
Distance to adjacent ramp	6336	ft	

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
		Ramp		
Volume, V (vph)	3440	260	40	vph
Peak-hour factor, PHF	0.90	0.90	0.90	

Peak 15-min volume, v ₁₅	956	72	11	v
Trucks and buses	0	0	0	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00 %	0.00 %	0.00 %	
Length	0.00 mi	0.00 mi	0.00 mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, f _{HV}	1.000	1.000	1.000	
Driver population factor, f _P	1.00	1.00	1.00	
Flow rate, v _p	3822	289	44	pcph

Estimation of V₁₂ Diverge Areas

L = (Equation 25-8 or 25-9)
EQ
P = 0.651 Using Equation 5
FD
 $v = v + (v - v) P = 2590$ pc/h
12 R F R FD

Capacity Checks

	Actual	Maximum	LOS F?
v = v	3822	6750	No
F _i F			
v = v - v	3533	6750	No
F _O F R			
v	289	2000	No
R			
v v	1232 pc/h	(Equation 25-15 or 25-16)	
3 or av ₃₄			
Is v v > 2700 pc/h?		No	
3 or av ₃₄			
Is v v > 1.5 v /2		No	
3 or av ₃₄ 12			
If yes, v = 2590		(Equation 25-18)	
12A			

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v	2590	4400	No
12			

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v - 0.009 L = 14.5$ pc/mi/ln
R 12 D

Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable, $D = 0.454$
S

Space mean speed in ramp influence area, $S = 49.1$ mph

	R	
Space mean speed in outer lanes,		S = 59.4 mph
	0	
Space mean speed for all vehicles,		S = 52.0 mph

HCS+: Ramps and Ramp Junctions Release 5.4

Phone: Fax:
E-mail:

Merge Analysis

Analyst: CTRR
 Agency/Co.: HNTB
 Date performed: 3/07/2010
 Analysis time period: Build Service Road Concept
 Freeway/Dir of Travel: Wekiva Pkwy. WB
 Junction: On Ramp from WB CD
 Jurisdiction: Seminole County
 Analysis Year: 2022
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Merge		
Number of lanes in freeway	4		
Free-flow speed on freeway	55.0	mph	
Volume on freeway	3180	vph	

On Ramp Data

Side of freeway	Right		
Number of lanes in ramp	1		
Free-flow speed on ramp	35.0	mph	
Volume on ramp	40	vph	
Length of first accel/decel lane	500	ft	
Length of second accel/decel lane		ft	

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes		
Volume on adjacent Ramp	260	vph	
Position of adjacent Ramp	Upstream		
Type of adjacent Ramp	Off		
Distance to adjacent Ramp	6336	ft	

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
		Ramp		
Volume, V (vph)	3180	40	260	vph
Peak-hour factor, PHF	0.90	0.90	0.90	

Peak 15-min volume, v ₁₅	883	11	72	v
Trucks and buses	0	0	0	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, f _{HV}	1.000	1.000	1.000	
Driver population factor, f _P	1.00	1.00	1.00	
Flow rate, v _p	3533	44	289	pcph

Estimation of V12 Merge Areas

L = (Equation 25-2 or 25-3)

EQ

P = 0.212 Using Equation 4

FM

v = v (P) = 750 pc/h

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	3577	9000	No
FO			
v v	1391 pc/h	(Equation 25-4 or 25-5)	
3 or av ₃₄			
Is v v > 2700 pc/h?		No	
3 or av ₃₄			
Is v v > 1.5 v /2		Yes	
3 or av ₃₄ 12			
If yes, v = 1413		(Equation 25-8)	
12A			

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	1413	4600	No
12A			

Level of Service Determination (if not F)

Density, D = 5.475 + 0.00734 v_R + 0.0078 v_A - 0.00627 L = 13.7 pc/mi/ln

Level of service for ramp-freeway junction areas of influence B

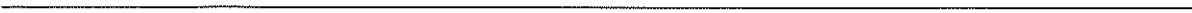
Speed Estimation

Intermediate speed variable, M = 0.303

Space mean speed in ramp influence area, S = 51.1 mph

Space mean speed in outer lanes, S = 53.0 mph

Space mean speed for all vehicles, S = 52.2 mph



HCS+: Ramps and Ramp Junctions Release 5.4

Phone: Fax:
E-mail:

Diverge Analysis

Analyst: CTRR
 Agency/Co.: HNTB
 Date performed: 3/07/2010
 Analysis time period: Build Service Road Concept
 Freeway/Dir of Travel: Wekiva Pkwy. EB
 Junction: Off Ramp to EB CD
 Jurisdiction: Seminole County
 Analysis Year: 2022
 Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Diverge	
Number of lanes in freeway	3	
Free-flow speed on freeway	55.0	mph
Volume on freeway	1880	vph

Off Ramp Data

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	35.0	mph
Volume on ramp	30	vph
Length of first accel/decel lane	500	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes	
Volume on adjacent ramp	210	vph
Position of adjacent ramp	Downstream	
Type of adjacent ramp	On	
Distance to adjacent ramp	6336	ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
		Ramp		
Volume, V (vph)	1880	30	210	vph
Peak-hour factor, PHF	0.90	0.90	0.90	

Peak 15-min volume, v ₁₅	522	8	58	v
Trucks and buses	0	0	0	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	0.00 %	0.00 %	0.00 %	
Length	0.00 mi	0.00 mi	0.00 mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, f _{HV}	1.000	1.000	1.000	
Driver population factor, f _P	1.00	1.00	1.00	
Flow rate, v _p	2089	33	233	pcph

Estimation of V₁₂ Diverge Areas

L = (Equation 25-8 or 25-9)

EQ

P = 0.706 Using Equation 5

FD

$v = v + (v - v) P = 1485$ pc/h

12 R F R FD

Capacity Checks

	Actual	Maximum	LOS F?
v = v	2089	6750	No
Fi F			
v = v - v	2056	6750	No
FO F R			
v	33	2000	No
R			
v v	604 pc/h	(Equation 25-15 or 25-16)	
3 or av ³⁴			
Is v v > 2700 pc/h?		No	
3 or av ³⁴			
Is v v > 1.5 v /2		No	
3 or av ³⁴ 12			
If yes, v = 1485		(Equation 25-18)	
12A			

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v	1485	4400	No
12			

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v - 0.009 L = 12.5$ pc/mi/ln

R 12 D

Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable, $D = 0.431$

S

Space mean speed in ramp influence area, $S = 49.4$ mph

	R	
Space mean speed in outer lanes,		S = 60.3 mph
	0	
Space mean speed for all vehicles,		S = 52.1 mph

HCS+: Ramps and Ramp Junctions Release 5.4

Phone: Fax:
E-mail:

Merge Analysis

Analyst: CTRR
Agency/Co.: HNTB
Date performed: 3/07/2010
Analysis time period: Build Service Road Concept
Freeway/Dir of Travel: Wekiva Pkwy. EB
Junction: On Ramp from EB CD
Jurisdiction: Seminole County
Analysis Year: 2022
Description: Wekiva Parkway Project Development & Environment Study

Freeway Data

Type of analysis	Merge		
Number of lanes in freeway	3		
Free-flow speed on freeway	55.0	mph	
Volume on freeway	1850	vph	

On Ramp Data

Side of freeway	Right		
Number of lanes in ramp	1		
Free-flow speed on ramp	35.0	mph	
Volume on ramp	210	vph	
Length of first accel/decel lane	500	ft	
Length of second accel/decel lane		ft	

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist?	Yes		
Volume on adjacent Ramp	30	vph	
Position of adjacent Ramp	Upstream		
Type of adjacent Ramp	Off		
Distance to adjacent Ramp	6336	ft	

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent	
		Ramp		
Volume, V (vph)	1850	210	30	vph
Peak-hour factor, PHF	0.90	0.90	0.90	

Peak 15-min volume, v ₁₅	514	58	8	v
Trucks and buses	0	0	0	%
Recreational vehicles	0	0	0	%
Terrain type:	Level	Level	Level	
Grade	%	%	%	
Length	mi	mi	mi	
Trucks and buses PCE, ET	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	
Heavy vehicle adjustment, f _{HV}	1.000	1.000	1.000	
Driver population factor, f _P	1.00	1.00	1.00	
Flow rate, v _p	2056	233	33	pcph

Estimation of V12 Merge Areas

L = 140.05 (Equation 25-2 or 25-3)

EQ

P = 0.591 Using Equation 1

FM

v = v (P) = 1216 pc/h

12 F FM

Capacity Checks

	Actual	Maximum	LOS F?
v	2289	6750	No
FO			
v v	840 pc/h	(Equation 25-4 or 25-5)	
3 or av ₃₄			
Is v v > 2700 pc/h?		No	
3 or av ₃₄			
Is v v > 1.5 v /2		No	
3 or av ₃₄ 12			
If yes, v = 1216		(Equation 25-8)	
12A			

Flow Entering Merge Influence Area

	Actual	Max Desirable	Violation?
v	1216	4600	No
R12			

Level of Service Determination (if not F)

Density, D = 5.475 + 0.00734 v_R + 0.0078 v₁₂ - 0.00627 L_A = 13.5 pc/mi/ln

Level of service for ramp-freeway junction areas of influence B

Speed Estimation

Intermediate speed variable, M = 0.303

S

Space mean speed in ramp influence area, S = 51.1 mph

R

Space mean speed in outer lanes, S = 53.8 mph

0

Space mean speed for all vehicles, S = 52.0 mph

Heavy vehicle adjustment, fHV	0.957	0.957	0.957	
Driver population factor, fP	1.00	1.00	1.00	
Flow rate, vp	7258	1340	2794	pcph

Estimation of V12 Diverge Areas

$$L = \text{(Equation 25-8 or 25-9)}$$

$$EQ$$

$$P = 0.260 \quad \text{Using Equation } 0$$

$$FD$$

$$v_{12R} = v_F + (v_R - v_{FD}) P = 2879 \quad \text{pc/h}$$

Capacity Checks

	Actual	Maximum	LOS F?
$v_{Fi} = v_F$	7258	9000	No
$v_{FO} = v_F - v_R$	5918	9000	No
v_R	1340	3800	No
$v_{3 \text{ or } av34}$	2189 pc/h	(Equation 25-15 or 25-16)	
Is $v_{3 \text{ or } av34} > 2700 \text{ pc/h?}$		No	
Is $v_{3 \text{ or } av34} > 1.5 v_{12} / 2$		Yes	
If yes, $v_{12A} = 2903$		(Equation 25-18)	

Flow Entering Diverge Influence Area

	Actual	Max Desirable	Violation?
v_{12A}	2903	4400	No

Level of Service Determination (if not F)

Density, $D = 4.252 + 0.0086 v_{12R} - 0.009 L_D = 24.7 \quad \text{pc/mi/ln}$

Level of service for ramp-freeway junction areas of influence C ✓

Speed Estimation

Intermediate speed variable,	$D = 0.549$	
Space mean speed in ramp influence area,	$S_R = 47.9$	mph
Space mean speed in outer lanes,	$S_0 = 55.7$	mph
Space mean speed for all vehicles,	$S = 52.3$	mph